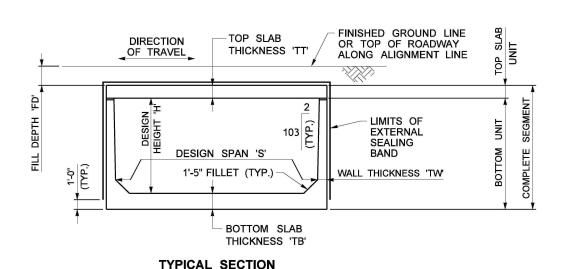
#### **GENERAL NOTES**

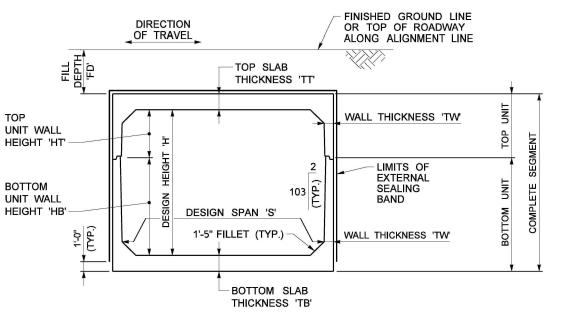
- 1. All materials and workmanship shall be in accordance with the requirements of the current edition of the **Standard Specifications**.
- 2. These Buried Structure Split Boxes have been designed in accordance with the requirements of the AASHTO LRFD Bridge Design Specifications 9th Edition 2020 and the WSDOT Bridge Design Manual (BDM) 2023. The seismic design for Class 2 Split Boxes (top and bottom units) has been designed in accordance with the AASHTO Technical Manual for Design and Construction of Road Tunnels, 2010. The seismic design for Class 2 Split Box ~ Slabs (bottom unit with top slab unit) has been designed in accordance with the AASHTO LRFD Bridge Design Specifications 9th Edition 2020. Two seismic zones corresponding to Site Adjusted Peak Ground Acceleration (A<sub>c</sub>) values of 0.32g and 0.64g have been considered.
- 3. The Contractor shall be responsible for safely lifting, shipping, installing, and backfilling the precast Buried Structure units.
- 4. Precast unit lay widths, 'LW', shall be a minimum of 4 feet, except the shorter side ('LW1') of Skew Option 3 or Skew Option 4 end units, shown on Sheet 2, shall not be less than 3 feet. Lay width of top and bottom units, for a given complete segment, shall be the same width and joints shall align.
- 5. The native subgrade material shall have a minimum factored bearing resistance capable of resisting the Strength Limit State maximum bearing as specified in the Design Tables. Bedding and Leveling material shall be in accordance with **Standard Specifications Section 6-20.3(6)A**.
- 6. The backfill and/or any existing soil within the Zone of Influence shown on Sheet 2 shall be granular soil with a minimum Internal Angle of Friction of 34 degrees and a total unit weight within the range of 120 to 145 PCF.
- 7. Precast concrete shall be Class 7000. Headwalls and cast-in-place concrete shall be Class 4000. 5-inch min. concrete topping slabs shall be Class 4000D.

- 8. Reinforcing steel shall conform to Standard Specifications Section 9-07.2. AASHTO M31 GR 60 (ASTM A615) are permitted and when used shall not be welded. When required, epoxy-coating of steel reinforcing bars shall conform to Standard Specification Section 9-07.3. Galvanized reinforcing bars conforming to ASTM A767 Class 1 or ASTM A1094 may be substituted for epoxy-coated reinforcement. The Contractor may substitute deformed welded wire reinforcement (WWR) conforming to Standard Specifications Section 9-07.7 provided any equivalent bar area reduction is limited to 15% of the original area and spacing is no greater than 2/3 of the dimension provided in the Design Tables. The specified minimum yield strength of the WWR shall be limited to a maximum of 75 ksi. Alternative proposed substitutions shall be submitted in accordance with Standard Specifications Section 6-20.3(1)A2.
- 9. Unless otherwise noted, concrete cover to reinforcing shall be 2 inches.
- 10. All steel plates and shapes shall be ASTM A36 or ASTM A572 GR 50. All bolts, nuts, and washers, unless noted otherwise, shall conform to Standard Specification Section 9-06.5(3). Resin Bonded Anchors Systems shall conform to Standard Specifications Section 9-06.4. All steel plates and shapes shall be galvanized in accordance with AASHTO M111 after fabrication, unless noted otherwise. Galvanizing shall be removed at any field welded zones. Bolts and hardware shall be galvanized in accordance with AASHTO M232 or ASTM F2329, as applicable. All galvanizing repairs shall be in accordance with ASTM A780. Galvanizing Repair Paint shall conform to Standard Specifications Section 9-08.1(2)B
- Resin Bonded Anchors shall conform to Standard Specifications Section 6-02.3(18)A and shall be installed in roto-drilled holes. The Contractor shall identify rebar free zones on the structure prior to drilling.
- 12. Unless otherwise noted all joints shall be sealed with Butyl Rubber Sealant and wrapped with External Sealing Bands in accordance with **Standard Specifications Section 6-20,3(8)A**.

- 13. Alternative Joint Types between precast units, other than those detailed herein, shall not be used unless submitted for acceptance by the Engineer in a Type 2 Working Drawing.
- Worker, Pedestrian and Bicycle Fall Protection shall be provided in accordance with Standard Specifications Section 6-20.3(1)F.
- 15. These plans may be used in combination with the following Standard Plans, unless otherwise specified in the contract:
- a. A-40.50 Bridge Approach Slab
- C-20.40 Beam Guardrail Type 31 Placement 12'-6", 18'-9", or 25'-0" Span
- c. C-20.41 Box Culvert Embedded Anchor Guardrail Steel Post Type 31
- d. C-20.43 Box Culvert Bolt-Thru Anchor Guardrail Steel Post Type 31
- e. C-81.10 42"Single Slope Barrier on Structure (TL-4)
- f. D-3.09 Permanent Geosynthetic (wingwall and headwalls)
- g. D-20.10 Precast Reinforced Concrete Retaining Wall (wingwalls)
- n. L-5.10 Bridge Railing Type Chain Link Pipe Rail (worker fall protection)
- . L-5.15 Cable Fence (worker fall protection)
- Provide surface treatment(s) as shown in this Plan unless noted otherwise in the Contract Documents. Alternate surface treatment(s) requires approval by the State Bridge and Structures Architect.
- All Split Box ~ Slab structures with less than 2 feet of Fill Depth shall have seismic lateral force restrainers per one of the options provided on sheets 14, 16, or 17.
- 18. Elastomeric Bearing Pad shall conform to **Standard Specifications Section 6-02.3(19) and 9-31.8(1).**
- For Structure Free Zone, freeboard, and maintenance clearance, see Contract Documents.



SPLIT BOX ~ SLAB



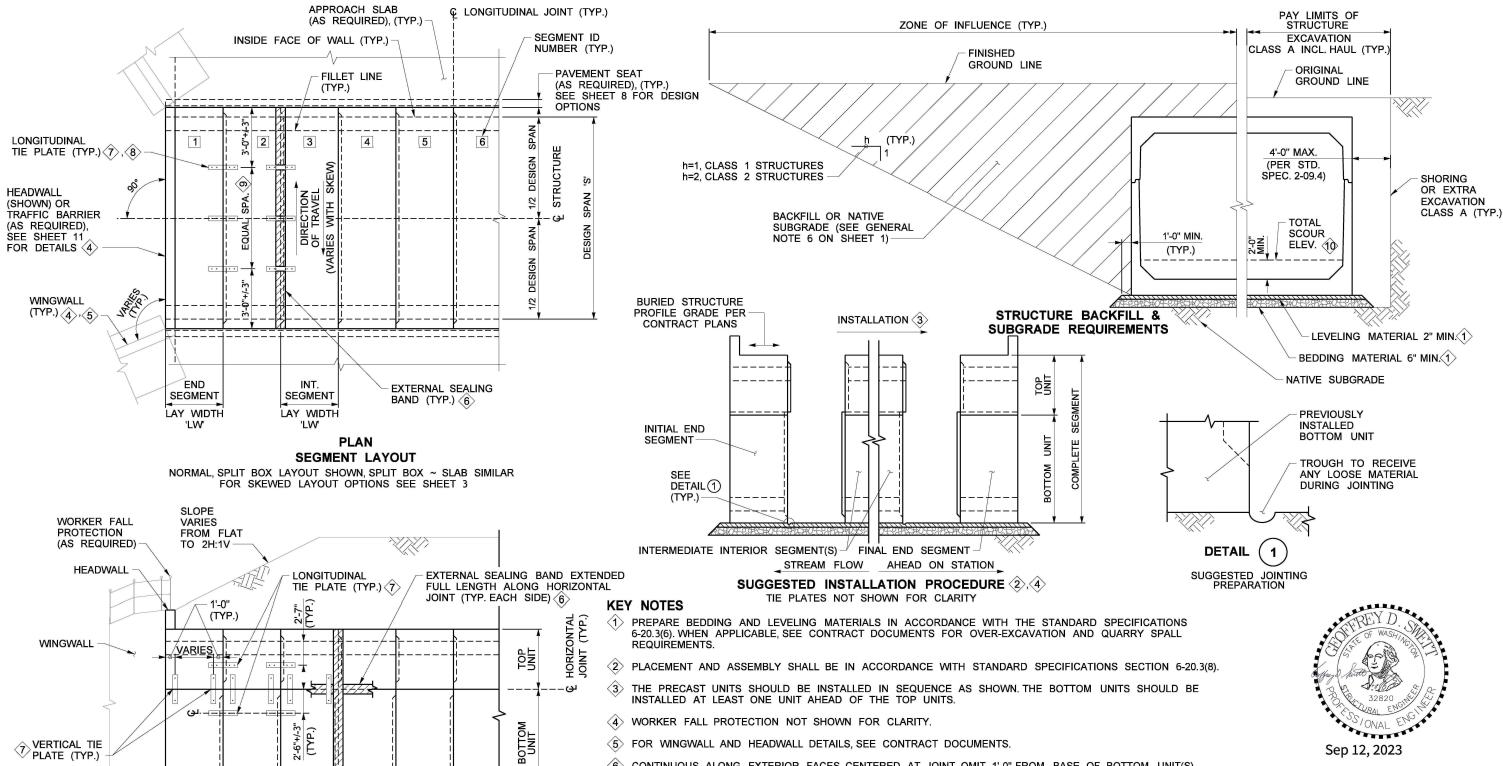
TYPICAL SECTION
SPLIT BOX



# BURIED STRUCTURE SPLIT BOX STANDARD PLAN E-20.10-00

SHEET 1 OF 17 SHEETS





# SPLIT BOX STANDARD PLAN E-20.10-00

SHEET 2 OF 17 SHEETS



**BURIED STRUCTURE** 

## **ELEVATION SEGMENT LAYOUT**

NORMAL, SPLIT BOX SHOWN, SPLIT BOX ~ SLAB SIMILAR FOR SKEWED LAYOUT OPTIONS SEE SHEET 3

EXTERNAL SEALING

EXTERIOR JOINTS)

VERTICAL JOINT

6 BAND (TYP. FOR ALL

- CONTINUOUS ALONG EXTERIOR FACES, CENTERED AT JOINT, OMIT 1'-0" FROM BASE OF BOTTOM UNIT(S) EXTERNAL SEALING BANDS SHALL PASS UNDER TIE PLATES (TYP.) SEE TYPICAL SECTION(S) ON SHEET 1 FOR GRAPHIC OF LIMITS.
- TIE PLATES ARE REQUIRED FOR THE FIRST THREE SEGMENTS AT EACH END, OR FOR A MINIMUM OF 12.0 FT. INBOARD FROM EACH END WHICHEVER IS GREATER. OMIT UPPER LONGITUDINAL AND VERTICAL TIE PLATES ON WALL FOR SPLIT BOX ~ SLABS. SEE SHEET 12 FOR TIE PLATE DETAILS. ADJUST HORIZONTAL LOCATION AS NECESSARY WHEN RESTRAINERS ARE REQUIRED.
- (8) AS APPLICABLE, DEPENDING ON SITE ROADWAY SCENARIO, SEE SHEET 8.
- TRANSVERSE SPACING BETWEEN TIE PLATES SHALL BE LIMITED TO 4'-0" WHEN BOTH, THE SITE PEAK GROUND ACCELERATION (A ) VALUE EXCEEDS 0.55g AND THE BACKFILL SLOPE RETAINED BY THE HEADWALL IS STEEPER THAN 2.5H:1V. FOR ALL OTHER CONDITIONS SPACING SHALL BE LIMITED TO 6'-0".
- DETERMINED BY THE TOTAL SCOUR RESULTING FROM THE WORST-CASE CONDITION BETWEEN THE SCOUR DESIGN FLOOD AND THE SCOUR CHECK FLOOD, IN ACCORDANCE WITH THE WSDOT HYDRAULICS MANUAL.

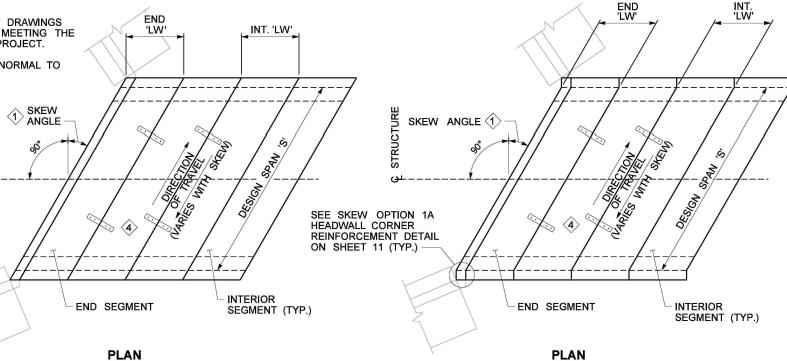
- 1) SKEW ANGLE SHALL NOT EXCEED 30 DEGREES.
- 2 FOR DETAILS NOT SHOWN SEE SEGMENT LAYOUT ON SHEET 2.

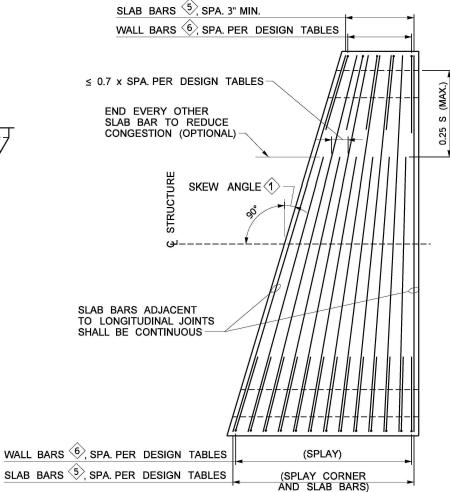
3 CONTRACTORS MAY SUBMIT SHOP DRAWINGS FOR ANY SKEW OPTION SHOWN MEETING THE DESIGN REQUIREMENTS OF THE PROJECT.

4 TIE PLATES, SHALL BE ORIENTED NORMAL TO JOINT (TYP).

5 SLAB BARS: B1 , B2 , T1 AND T2 (SEE SHEET 4)

6 WALL BARS: B3 , B4 , B5 T3 , T4 AND T5 (SEE SHEET 4)



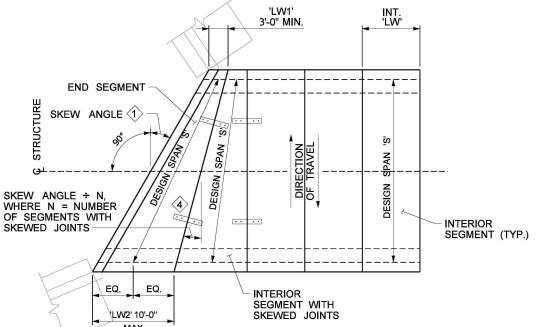


INT. 'LW' 3'-0" MIN. SKEW ANGLE (1) ... 4 ..... END SEGMENT INTERIOR SEGMENT (TYP.) EQ. EQ. 'LW2' 10'-0"

**SKEW OPTION 1** 

UNIFORM SKEW SEGMENT LAYOUT 2,3

PLAN **SKEW OPTION 3** TRAPEZOIDAL END SEGMENT LAYOUT 2,3



SKEW OPTION 2

SKEWED SLAB, NORMAL WALL SEGMENT LAYOUT 2,3

**PLAN SKEW OPTION 4** TRANSITIONING END SEGMENT LAYOUT ②,③

#### PLAN SKEW OPTION 3/4 REINFORCEMENT SPLAY DETAILS

ONLY SELECTED REINFORCING SHOWN FOR CLARITY

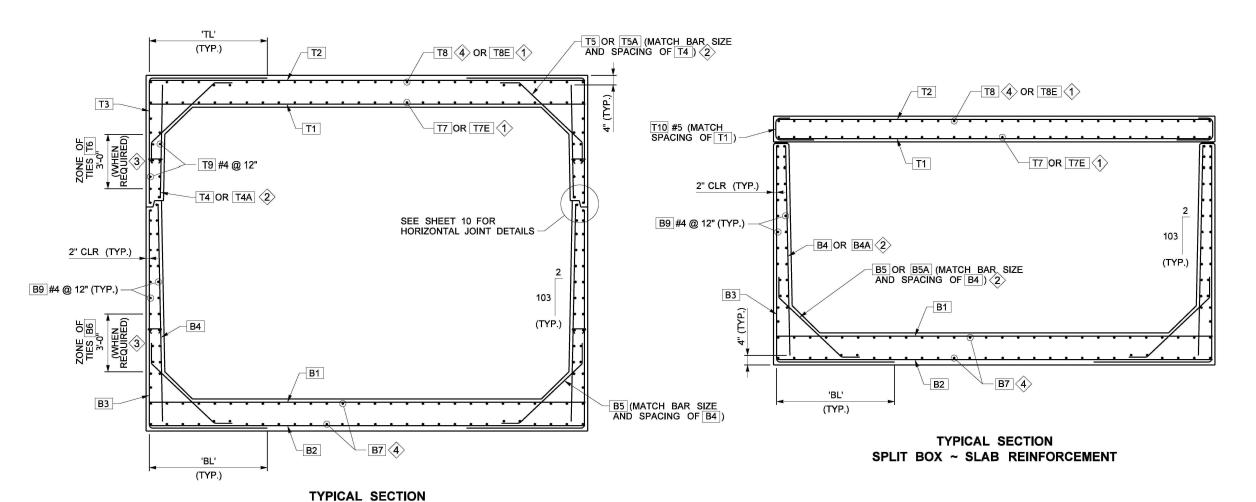


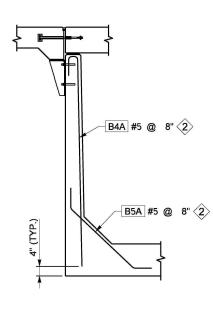
Sep 12, 2023

## **BURIED STRUCTURE** SPLIT BOX STANDARD PLAN E-20.10-00

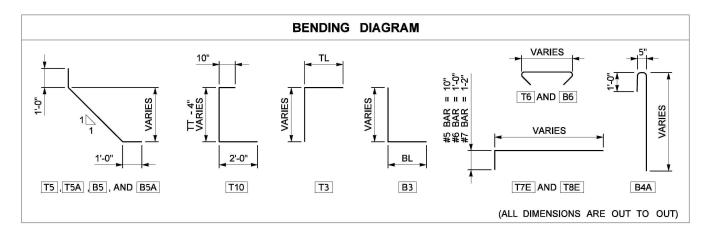
SHEET 3 OF 17 SHEETS







SPLIT BOX ~ SLAB ADDITIONAL REINFORCEMENT WHEN APPROACH SLAB SEAT IS PRESENT



SPLIT BOX REINFORCEMENT

#### **KEY NOTES**

DARS TE AND THE SHALL BE INSTALLED IN THE END SEGMENTS, ROTATE 90° HOOK AS NECESSARY TO PROVIDE THE SPECIFIED CLEARANCE.

SIZE AND SPACING OF BARS T7E SHALL MATCH T7

BARS T8E SHALL BE AS FOLLOWS:

END SEGMENT WITHOUT HEADWALL OR TRAFFIC BARRIER: #5 @ 12" END SEGMENT WITH UP TO 2'-0" HEADWALL: #6 @ 12" END SEGMENT WITH UP TO 4'-0" HEADWALL: #6 @ 6"

END SEGMENT WITH TRAFFIC BARRIER: #7 @ 6"
FOR ADDITIONAL END SEGMENT DETAILS SEE SHEET 11.

② WHEN AN APPROACH SLAB SEAT IS PRESENT, ADDITIONAL REINFORCEMENT IS REQUIRED AS FOLLOWS:

FOR SPLIT BOX ~ SLAB STRUCTURES: ADD  $\boxed{B4A}$  #5  $\bigcirc$  8" AND  $\boxed{B5A}$  #5  $\bigcirc$  8". BUNDLE WITH TYPICAL  $\boxed{B4}$  AND  $\boxed{B5}$  RESPECTIVELY.

FOR SPLIT BOX STRUCTURES: ADD  $\overline{14A}$  #5 @ 8" AND  $\overline{15A}$  #5 @ 8". BUNDLE WITH TYPICAL  $\overline{14}$  AND  $\overline{15}$  RESPECTIVELY.

- 3 TIES TO #4 AND B6 #4 SHALL BE SPACED AT 6" MAX. VERTICALLY AND AT 2'-0" MAX. HORIZONTALLY. EACH TIE SHALL BE HOOKED AROUND LONGITUDINAL BARS TO OR B9. ADDITIONAL BARS TO AND B9 MAY BE ADDED TO FACILITATE PLACEMENT OF TIES AS REQUIRED.
- (4) BARS B7 AND T8 SHALL BE #4 @ 12" WHEN CORRESPONDING SLAB THICKNESS ≤ 20 INCHES, AND #4 @ 9" FOR THICKER SLABS.



# SPLIT BOX STANDARD PLAN E-20.10-00

**BURIED STRUCTURE** 

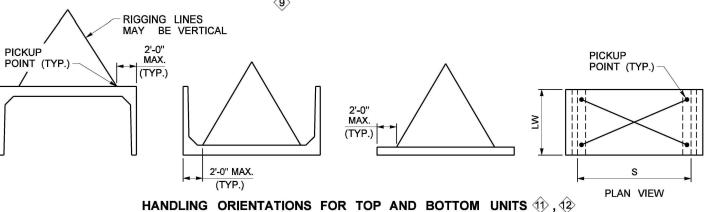
SHEET 4 OF 17 SHEETS

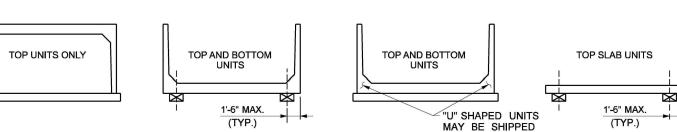


DESIGN TABLE ~ CLASS I STRUCTURES ~ SPANS 12', 15' AND 18'

DEGIGIN IA	SIGN TABLE ~ CLASS I STRUCTURES ~ SPANS 12", 15' AND 18'  DESIGN HEIGHT MEMBER THICKNES					KNESS	TOP UNIT REINFORCEMENT 3											BOTTOM UNIT REINFORCEMENT (3)										l N	IATERIAL						
DESIGN SPAN S	TOTAL H	TOP UNIT WALL	BOT. UNIT WALL	FILL DEPTH FD	WALLS TW	TOP SLAB	BOT. SLAB	(SL	AB RIOR)		Γ2 AB		T3 (CORNI	]	(w	T4 ALL RIOR)	T6 (TIES)	(BC	Г <u>7</u> ОТ.)		AB RIOR)		32 _AB		B3 DRNER)		(WAI	] .L	B6 (TIES)	тор	UNIT		OM UNIT	STR. I BEARING DEMAND	JOINT
<u>(1)</u>		HT	НВ	<b>(2</b> )		TT	ТВ	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	TL	SIZE	SPA.	⟨₹)	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	BL	SIZE	SPA.	⟨Ŷ⟩	CONC. (CY/FT)	STEEL (LBS/FT)	CONC. (CY/FT)	STEEL (LBS/FT)	(PSF)	TYPE 6
				0' ≤ FD < 2'	10"	12"	10"	#7	6"	#5	12"		•		#4	8"	*	#5	10"	#6	6"	#7	5"	#7	5"	3'-2"	#4	8"		0.58	106.1	1.06	322.5	2397	2
				2'≤ FD ≤ 4'	10"	12"	10"	#7	6"	#5	12"				#4	8"		#4	12"	#6	6"	#7	5"	#7	5"	3'-2"	#4	8"		0.58	98.4	1.06	322.5	2490	2
12'	10'	NA	10'	4'< FD ≤ 6'	12"	12"	12"	#8	6"	#5	12"		NA		#4	8"	NA	#4	12"	#6	6"	#6	5"	#7	5"	4'-3"	#4	8"	NA	0.59	112.8	1.28	318.9	2901	2
				6'< FD ≤ 8'	12"	12"	12"	#8	6"	#5	12"				#4	8"		#4	12"	#6	6"	#7	5"	#7	5"	3'-4"	#4	8"		0.59	112.8	1.28	327.7	3255	2
				8'< FD ≤ 10'	12"	12"	12"	#9	6"	#5	12"			_	#4	8"		#6	10"	#7	6"	#7	5''	#8	5"	3'-8"	#4	8"		0.59	150.3	1.28	382.2	3631	2
				0' ≤ FD < 2'	10"	12"	10"	#9	6"	#5	12"				#4	8"		#4	12"	#8	6"	#6	5''	#7	5" :	3'-11"	#4	8"		0.69	157.2	1.15	361.0	2269	2
	10'	WA	10'	2'≤ FD ≤ 4'	10"	12"	10"	#8	6"	#5	12"		NA		#4	8"	NA	#4	12"	#8	6"	#6	5"	#7	5"	5'-1"	#4	8"	N/A	0.69	138.3	1.15	378.8	2313	2
				4'< FD ≤ 8'	12"	14"	12"	#9	6"	#5	10"		1071		#4	8"	1071	#4	12"	#9	6"	#6	5"	#7	5"	4'-1"	#4	8"	1071	0.81	163.6	1.39	399.5	3147	2
				8'< FD ≤ 10'	12"	16"	14"	#9	6"	#6	12"				#4	8"		#5	10"	#8	6"	#6	5"	#8	5"	3'-7"	#4	8"		0.91	183.0	1.50	422.7	3625	2
				0' ≤ FD < 2'	10"	12"	10"	#9	6"	#6	7"	#6	7"	3'-0"	#4	8"		#5	9"	#8	6"	#5	10"	#6	5"	4'-4"	#4	8"		0.99	272.1	1.15	301.1	2030	1
				2'≤ FD ≤ 4'	10"	12"	10"	#9	6"	#6	7"	#6	7"	3'-0"	#4	8"		#4	12"	#8	6"	#5	12"	#6	-	4'-9"	#4	8"		0.99	260.1	1.15	282.2	1992	1
				4'< FD ≤ 8'	10"	14"	12"	#9	6"	#5	10"	#5	5"	5'-2"	#4	8"	NA	#4	12"	#9	6"	#5	10"	#6		3'-9"	#4	8"	NA	1.09	244.9	1.26	315.9	2792	1
15'	12'		7'	8'< FD ≤ 12'	12"	16"	14"	#10	6"	#6	12"	#5	6"	3'-10"	#4	8"	,	#4	12"	#9	6"	#6	12"	#7	6"	3'-5"	#4	8"		1.27	273.3	1.50	342.9	3739	1
	ТО	5'	TO	12'< FD ≤ 16'	12"	18"	16"	#10	6"	#6	12"	#6	6"	3'-9"	#4	8"		#4	12"	#9	6"	#6	10"	#7		3'-7"	#4	8"		1.38	292.3	1.60	375.3	4436	1
	15'		10'	16'< FD ≤ 20'	12"	20"	18"	#10	6"	#5	6"	#6	6"	3'-9"	#4	8"		#4	12"	#10	6"	#6	12"	#8		3'-9"	#4	8"		1.48	303.5	1.71	417.2	5372	1
				20'< FD ≤ 24'	12"	22"	22"	#10	6"	#6	10"	#6	5"	3'-11"	#4	8"	Y	#4	12"	#10	6"	#6	9"	#8		3'-11"	#4	8"	Y	1.59	327.1	1.92	443.4	6342	1
				24'< FD ≤ 28'	12"	22"	22"	#11	6"	#6	10"	#7	5"	3'-11"	#4	8"	Υ	#4	12"	#10	6"	#6	10"	#8		3'-11"	#4	8"	Y	1.59	387.9	1.92	473.0	7209	1
				28'< FD ≤ 32'	12"	24"	24"	#11	6"	#7	10"	#7	5"	4'-1"	#4	8"	Y	#4	12"	#11	6"	#7	10"	#8		4'-1"	#4	8"	Y	1.69	402.5	2.02	522.3	8145	1
				32'< FD ≤ 38'	12"	24"	24"	#11	5"	#7	10"	#8	5"	4'-1"	#4	8"	Y	#4	12"	#11	6"	#7	10"	#8	-	4'-1"	#4	8"	Y	1.69	470.2	2.02	522.3	9445	1
				0' ≤ FD < 2'	10"	16"	12"	#8	6"	#6	12"				#4	8"		#4	12"	#9	6"	#6	5"	#7		3'-2"	#4	8"		1.04	164.2	1.37	422.3	2259	2
				2'≤ FD ≤ 4'	10"	14"	12"	#9	6"	#5	10"		.0.0		#4	8"		#4	12"	#9	6"	#6	5"	#7		3'-2"	#4	8"		0.92	188.0	1.37	420.7	2348	2
	10'	NA	10'	4'< FD ≤ 6'	12"	16"	14"	#9	6"	#6	12"		NA		#4	8"	NA	#4	12"	#10	6"	#7	10"	#7		4'-3"	#4	8"	NA	1.06	195.9	1.63	449.0	2731	2
				6'< FD ≤ 8'	12"	18"	16"	#9	6"	#6	12"				#4	8"		#4	12"	#9	6"	#7	10"	#7		3'-10"	#4	8"		1.18	196.3	1.75	412.6	3165	2
				8'< FD ≤ 10'	12"	18"	16"	#10	6"	#6	12"		-		#4	8"		#4	12"	#9	6"	#6	5"	#8	-	3'-6"	#4	8"		1.18	196.3	1.75	483.8	3576	2
				0' ≤ FD < 2'	10"	14"	12"	#9	6"	#6	9"	#6	9"	3'-1"	#4	8"		#5	8"	#8	6"	#6	10"	#6	_	3'-1"	#4	8"		1.22	290.0	1.37	328.2	1877	1
18'				2'≤ FD ≤ 4'	10"	14"	12"	#9	6"	#5	8"	#6	8"	4'-0"	#4	8"		#4	12"	#9	6"	#6	10"	#6		3'-1"	#4	8"		1.22	272.9	1.37	356.4	2001	1
	12'		7'	4'< FD ≤ 6'	10"	16"	14"	#9	6"	#5	9"	#5	6"	3'-8"	#4	8"	NA	#4	12"	#9	6"	#5	10"	#7		3'-6"	#4	8"	NA	1.34	261.4	1.49	381.6	2349	1
	ТО	5'	ТО	6'< FD ≤ 10'	10"	18"	16"	#10	6"	#6	12"	#6	6"	3'-5"	#4	8"		#4	12"	#10	6"	#6	10"	#7		3'-5"	#4	8"		1.47	316.1	1.61	428.4	3198	1
	15'		10'	10'< FD ≤ 14'	12"	20"	18"	#11	6"	#5	6"	#5	6"	3'-9"	#4	8"		#4	12"	#10	6"	#6	10"	#7		3'-9"	#4	8"		1.67	360.8	1.87	444.3	3984	1
				14'< FD ≤ 18'	12"	22"	22"	#11	6"	#5	7"	#6	7"	3'-11"	#4	8"	Y	#4	12"	#10	6"	#6	10"	#7	2011	3'-11"	#4	8"	Y	1.79	380.4	2.12	461.9	4973	1
				18'< FD ≤ 22'	12"	22"	22"	#11	5"	#6	10"	#6	5"	3'-11"	#4	8"	Y	#4	12"	#11	6"	#6	10"	#8		3'-11"	#4	8"	Y	1.79	443.9	2.12	548.0	5841	1
				22'< FD ≤ 30'	12"	24"	24"	#10 (9)	4"	#7	10"	#6	5"	4'-1"	#4	8"	Y	#4	12"	#10 (9)	4"	#7	10"	#8	5"	4'-1"	#4	8"	Υ	1.92	462.7	2.24	610.4	7643	1

- DESIGN SPAN 'S', SHALL BE TAKEN EQUAL TO THE HORIZONTAL DISTANCE BETWEEN INSIDE FACE OF WALLS AT THE CORNER BETWEEN THE FILLET AND THE WALL AS SHOWN ON SHEET 1. FOR SEGMENTS WITH A SKEW SEE GRAPHICAL CLARIFICATION OF DESIGN SPAN ON SHEET 3. IF THE DESIGN SPAN FALLS IN BETWEEN A DESIGN SPAN VALUE IN THE TABLES, USE REINFORCEMENT FOR THE LONGER SPAN. CLASS 1 STRUCTURES WITH SPANS GREATER THAN 18 FEET AND LESS THAN 20 FEET SHALL USE GEOMETRY AND REINFORCEMENT FOR 21 FOOT SPAN CLASS 2 STRUCTURES WITH A PGA = 0.32G.
- THE MAXIMUM FILL DEPTH OVER THE ENTIRE BURIED STRUCTURE SHALL BE USED WHEN SELECTING DESIGN FILL DEPTH.
- (3) FOR REINFORCING DETAILS SEE SHEET 4.
- $\stackrel{\textstyle <}{4}$  MAXIMUM REQUIRED BEARING RESISTANCE FOR STRENGTH LIMIT STATE.
- 5 THESE STRUCTURES HAVE BEEN DESIGNED FOR A TRAVERSE DIFFERENTIAL SETTLEMENT DISPLACEMENT EQUAL TO 2 INCHES PER 100 FEET OF STRUCTURAL SPAN (DESIGN SPAN + 1.0 FOOT)
- 6 FOR HORIZONTAL JOINT TYPE DETAILS SEE SHEET 10.
- 7 WALL TIES T6 AND B6 ARE REQUIRED WHEN INDICATED BY LETTER 'Y'.
- (8) MATERIAL QUANTITIES ARE FOR A TYPICAL INTERIOR SEGMENT. STEEL WEIGHT DOES NOT INCLUDE ADDITIONAL REINFORCING REQUIRED FOR APPROACH SLAB SEAT.
- (9) SINGLE BARS SPACED AT 4 INCHES MAY BE SUBSTITUTED BY TWO-BAR BUNDLES SPACED AT 8".
- 10 QUANTITIES OF BOTTOM UNIT CORRESPOND TO 'HB' = 10'-0".
- REINFORCING OF PRECAST UNITS HAVE BEEN DESIGNED FOR HANDLING AND SHIPPING ORIENTATIONS PRESENTED ON THIS SHEET. ALTERNATE SHIPPING OR HANDLING ORIENTATIONS SHALL BE ANALYZED BY THE CONTRACTOR.





ON THIS FACE SHIPPING ORIENTATIONS (1), (2)



Sep 12, 2023

SHEET 5 OF 17 SHEETS

APPROVED FOR PUBLICATION

Mode a Maines Sep 12, 2023

STATE DESIGN ENGINEER

Washington, State, Department of T

(2) CONCRETE COMPRESSIVE STRENGTH SHALL BE AT LEAST F'CI = 3.5 KSI FOR HANDLING AND F'C = 7.0 KSI FOR SHIPPING.

DESIGN TABLE ~ PGA = 0.32g ~ CLASS II STRUCTURES ~ SPANS 21', 24', 27' AND 30'

	DESIGN HEIGHT				MEMBER THICKNESS TOP UNIT REINFORCEMENT 3										BOTTOM UNIT REINFORCEMENT 3											MATERIAL QUANTITIES				DESIGN VALUES						
DESIGN SPAN S	TOTAL H	TOP UNIT WALL	BOT. UNIT WALL	FILL DEPTH FD	WALLS TW	TOP SLAB TT	BOT. SLAB TB	(SI	T1 LAB RIOR)	(SL EXTE		1	T3 (CORNEI	R)	(W	T4 ALL RIOR)	T6 (TIES)		T7 OT.)	(5	B1 SLAB ERIOR)	(S	B2 LAB ERIOR)		B3 (CORNE	R)	(W	B4 ALL RIOR)	B6 (TIES)		UNIT		OM UNIT	STR. I BEARING DEMAND (PSF)		REQ'D HORIZ. JOINT TYPE
(1)		HT	НВ	<b>(2</b> >		- 11	IB	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	TL	SIZE	SPA.	♦	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	BL	SIZE	SPA.	<b>⟨</b> ĵ⟩	CONC. (CY/FT)	STEEL (LBS/FT)	CONC. (CY/FT)	STEEL (LBS/FT)	4>	<b>(5)</b>	<b>6</b>
				0' ≤ FD < 2'	12"	16"	14"	#9	6"	#6	12"		N/A		N	I/A	N/A	#6	12"	#9	6"	#5	6"	#6	6"	3'-4"	#4	8"	N/A	1.21	250.9	1.76	382.2	2177	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	16"	14"	#9	6"	#6	12"		N/A		N	I/A	N/A	#4	12"	#9	6"	#5	6"	#7	6"	4'-4"	#4	8"	N/A	1.21	230.9	1.76	421.5	2365	N/A	2
				4'< FD ≤ 6'	12"	18"	16"	#10	6"	#6	12"		N/A		N	I/A	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-5"	#4	8"	N/A	1.35	272.2	1.90	488.9	2777	N/A	2
21'				0' ≤ FD < 2'	12"	14"	12"	#10	6"	#6	8"	#5	8"	3'-4"	#4	8"	N/A	#6	9"	#9	6"	#5	6"	#7	6"	3'-7"	#4	8"	N/A	1.43	376.7	1.61	413.5	1762	0.16"	1A
				2' ≤ FD ≤ 4'	12"	16"	14"	#10	6"	#6	8"	#5	8"	3'-5"	#4	8"	N/A	#4	12"	#9	6"	#5	6"	#7	6"	3'-7"	#4	8"	N/A	1.57	347.4	1.76	415.4	2015	0.18"	1A
	12' TO 15'	5'	7' TO 10'	4'< FD ≤ 8'	12"	18"	16"	#11	6"	#5	7"	#5	7"	3'-7"	#4	8"	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.71	388.5	1.90	490.6	2767	0.21"	1
				8'< FD ≤ 12'	12"	20"	20"	#11	6"	#5	6"	#5	6"	3'-9"	#4	8"	N/A	#4	12"	#11	6"	#6	6"	#8	6"	3'-9"	#4	8"	N/A	1.86	403.0	2.18	566.2	3660	0.25"	1
				12'< FD ≤ 16'	12"	22"	22"	#11	5"	#6	10"	#6	5"	3'-11"	#4	8"	N/A	#4	9"	#11	6"	#6	10"	#8	5"	4'-0"	#4	8"	N/A	2.00	492.9	2.32	624.2	4455	0.28"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD ≤ 2'	12"	14"	14"	#9	6"	#6	5"	#8	5"	5'-0"	#4	8"	N/A	#6	12"	#9	6"	#6	5"	#8	5"	4'-9"	#4	8"	N/A	1.76	553.5	1.76	530.3	1830	0.27"	1
				0' ≤ FD < 2'	12"	18"	16"	#9	6"	#6	12"		N/A		N	VA	N/A	#6	12"	#9	6"	#5	6"	#7	6"	3'-5"	#4	8"	N/A	1.52	282.7	2.05	446.5	2137	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	18"	16"	#10	6"	#6	12"		N/A		N	I/A	N/A	#4	12"	#10	6"	#5	6"	#7	6"	3'-5"	#4	8"	N/A	1.52	306.5	2.05	492.9	2350	N/A	2
				4'< FD ≤ 6'	12"	18"	18"	#11	6"	#6	12"		N/A		N	V/A	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.52	358.4	2.21	528.2	2733	N/A	2
				0' ≤ FD < 2'	12"	18"	16"	#10	6"	#6	8"	#5	8"	3'-7"	#4	8"	N/A	#6	10"	#8	6"	#7	10"	#6	5"	3'-7"	#4	8"	N/A	1.88	415.5	2.05	405.6	1773	0.17"	1
24'	12' TO 15'			2' ≤ FD ≤ 4'	12"	18"	16"	#10	6"	#6	8"	#5	8"	3'-7"	#4	8"	N/A	#4	12"	#9	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.88	385.5	2.05	481.4	1984	0.18"	1
		5'	7' TO 10'	4'< FD ≤ 8'	12"	20"	18"	#11	6"	#6	8"	#6	8"	3'-9"	#4	8"	N/A	#4	12"	#10	6"	#5	6"	#8	6"	3'-10"	#4	8"	N/A	2.04	452.8	2.21	536.1	2776	0.22"	1
				8'< FD ≤ 10'	12"	22"	22"	#11	6"	#6	8"	#6	8"	3'-11"	#4	8"	N/A	#4	9"	#11	6"	#7	10"	#7	5"	3'-11"	#4	8"	N/A	2.20	470.8	2.53	637.9	3260	0.24"	1
				10'< FD ≤ 14'	12"	24"	24"	#10<	9> 4"	#6	7"	#6	7"	4'-1"	#4	8"	N/A	#4	9"	#10	9 4"	#7	10"	#7	5"	4'-1"	#4	8"	N/A	2.36	546.5	2.69	747.5	4150	0.27"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD < 2'	12"	16"	16"	#9	6"	#6	5"	#8	5"	3'-10"	#4	8"	N/A	#6	12"	#9	6"	#6	5"	#8	5"	3'-11"	#4	8"	N/A	2.05	578.9	2.05	557.4	1817	0.25"	1
				2' ≤ FD ≤ 4'	12"	16"	16"	#10	6"	#6	5"	#8	5"	4'-3"	#4	8"	N/A	#4	12"	#10	6"	#6	5"	#8	5"	4'-3"	#4	8"	N/A	2.05	608.0	2.05	608.0	2028	0.29"	1
				0' ≤ FD < 2'	12"	18"	16"	#10	6"	#6	12"		N/A			V/A	N/A	#6	10"	#10	6"	#6	6"	#6	6"	3'-9"	#4	8"	N/A	1.68	374.9	2.19	525.6	2107	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	18"	18"	#11	6"	#6	12"		N/A		-	I/A	N/A	#4	12"	#10	_	#6	6"	#7	6"	3'-9"	#4	8"	N/A	1.68	398.8	2.37	558.5	2401	N/A	2
				4'< FD ≤ 6'	12"	22"	20"	#11	6"	#6	10"		N/A			I/A	N/A	#4	9"	#11	6"	#7	6"	#8	6"	3'-9"	#4	8"	N/A	2.04	420.1	2.55	688.8	2855	N/A	2
	Mark Control Marie			0' ≤ FD < 2'	12"	18"	16"	#10	6"	#7	8"	#5	8"	3'-7"	#4	8"	N/A	#6	10"	#9	6"	#8	10"	#7	5"	3'-7"	#4	8"	N/A	2.05	479.4	2.19	534.7	1679	0.17"	1
27'	12' TO 15'	5'	7' TO 10'	2' ≤ FD ≤ 4'	12"	20"	18"	#11	6"	#7	8"	#6	8"	3'-9"	#4	8"	N/A	#4	12"	#10		#8	10"	#7	5"	3'-9"	#4	8"	N/A	2.23	518.7	2.37	590.2	1974	0.19"	1
				4'< FD ≤ 6'	12"	22"	20"	#11	6"	#7	8"	#6	8"	3'-11"	#4	8"	N/A	#4	9"	#10	-	#8	10"	#7	5"	3'-11"	#4	8"	N/A	2.40	538.0	2.55	594.0	2394	0.21"	1
				6'< FD ≤ 10'	12"	24"	24"		9> 4"	#7	9"	#6	9"	4'-1"	#4	8"	N/A	#4	9"	#11	5"	#8	10"	#7	5"	4'-1"	#4	8"	N/A	2.58	590.3	2.91	770.4	3266	0.24"	1
	17' TO 20'	7' 10 10'	10'	0' ≤ FD < 2'	12"	18"	18"	#10	7"	#8	10"	#8	5"	3'-8"	#4	8"	N/A	#6	12"	#9	6"	#8	10"	#8	5"	3'-10"	#4	8"	N/A	2.37	622.1	2.37	582.7	1782	0.28"	1
				2'< FD ≤ 4'	12"	18"	18"	#10	6"	#8	10"	#8	5"	4'-0"	#4	8"	N/A	#4	12"	#10		#8	10"	#8	5"	4'-0"	#4	8"	N/A	2.37	636.6	2.37	636.6	2009	0.3"	1
				0' ≤ FD < 2'	12"	20"	18"	#10	6"	#6	11"		N/A			I/A	N/A	#6	10"	#11		#6	5"	#8	5"	3'-9"	#4	8"	N/A	2.05	416.4	2.54	751.6	2101	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	20"	18"	#11	6"	#6	11"		N/A			I/A	N/A	#4	12"	#11	6"	#7	6"	#7	6"	3'-9"	#4	8"	N/A	2.05	443.8	2.54	696.9	2322	N/A	2
				4'< FD ≤ 6'	12"	22"	20"	#11	5"	#6	10"	115	N/A	01.01		I/A	N/A	#4	9"	#11		#7	6"	#8	6"	4'-0"	#4	8"	N/A	2.25	531.8	2.74	806.9	2737	N/A	2
				0' ≤ FD < 2'	12"	20"	18"	#11	6"	#7	7"	#5	7"	3'-9"	#4	8"	N/A	#6	9"	#10		#6	5"	#7	5"	3'-9"	#4	8"	N/A	2.41	610.8	2.54	642.3	1713	0.15"	1
30'	12' TO 15'	5'	7' TO 10'	2' ≤ FD ≤ 4'	12"	22"	20"	#11	6"	#7	7"	#5	7"	3'-11"	#4	8"	N/A	#4	9"	#10		#6	5"	#7	5"	3'-11"	#4	8"	N/A	2.61	588.7	2.74	646.1	1999	0.19"	1
				4'< FD ≤ 6'	12"	24"	22"	#11	5"	#7	7"	#6	7"	4'-1"	#4	8"	N/A	#4	9"	#11	6"	#6	5"	#8	5"	4'-1"	#4	8"	N/A	2.81	674.5	2.93	817.7	2414	0.21"	1
	471 TO 201	71.70 15:	401	6'< FD ≤ 8'	12"	24"	24"	#10<	7	#7	7"	#6	7"	4'-1"	#4	8"	N/A	#4	9"	#11		#6	5"	#8	5"	4'-1"	#4	8"	N/A	2.81	679.5	3.13	885.4	2821	0.23"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD < 2'	12"	18"	18"	#10	6"	#6	6"	#8	6"	4'-2"	#4	8"	N/A	#6	10"	#10		#6	5"	#8	5"	3'-9"	#4	8"	N/A	2.54	675.6	2.54	685.5	1743	0.26"	1
				2' ≤ FD ≤ 4'	12"	20"	20"	#11	6"	#6	5"	#8	5"	3'-9"	#4	8"	N/A	#4	12"	#11	6"	#6	5"	#8	5"	3'-9"	#4	8"	N/A	2.74	752.1	2.74	752.1	2028	0.3"	1

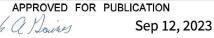
- DESIGN SPAN 'S', SHALL BE TAKEN EQUAL TO THE HORIZONTAL DISTANCE BETWEEN INSIDE FACE OF WALLS AT THE CORNER BETWEEN THE FILLET AND THE WALL AS SHOWN ON SHEET 1. FOR SEGMENTS WITH A SKEW SEE GRAPHICAL CLARIFICATION OF DESIGN SPAN ON SHEET 3. IF THE DESIGN SPAN FALLS INBETWEEN A DESIGN SPAN VALUE IN THE TABLES. USE REINFORCEMENT FOR THE LONGER SPAN.
- 2 THE MAXIMUM FILL DEPTH OVER THE ENTIRE BURIED STRUCTURE SHALL BE USED WHEN SELECTING DESIGN FILL DEPTH.
- (3) FOR REINFORCING DETAILS SEE SHEET 4.
- 4 MAXIMUM REQUIRED BEARING RESISTANCE FOR STRENGTH LIMIT STATE.
- (5) THE STRUCTURE HAS BEEN DESIGNED FOR THE RACKING DISPLACEMENTS PRESENTED HERE. THESE DESIGN VALUES ARE DETERMINED BASED ON THE DESIGN CRITERIA AS OUTLINED IN THE BDM. THESE STRUCTURES HAVE BEEN DESIGNED FOR A TRAVERSE DIFFERENTIAL SETTLEMENT DISPLACEMENT EQUAL TO 2 INCHES PER 100 FEET OF STRUCTURAL SPAN (DESIGN SPAN + 1.0 FOOT).
- 6 FOR HORIZONTAL JOINT TYPE DETAILS SEE SHEET 10.
- (7) WALL TIES T6 AND B6 ARE REQUIRED WHEN INDICATED BY LETTER 'Y'.
- (8) MATERIAL QUANTITIES ARE FOR TYPICAL INTERIOR SEGMENT. STEEL WEIGHT DOES NOT INCLUDE ADDITIONAL REINFORCING REQUIRED FOR APPROACH SLAB SEAT.
- (9) SINGLE BARS SPACED AT 4 INCHES MAY BE SUBSTITUTED BY TWO-BAR BUNDLES SPACED AT 8".
- (1) QUANTITIES OF BOTTOM UNIT CORRESPOND TO 'HB' = 10'-0".
- (1) SEE SHEET 5 FOR HANDLING AND SHIPPING ORIENTATIONS.



# BURIED STRUCTURE SPLIT BOX

#### STANDARD PLAN E-20.10-00

SHEET 6 OF 17 SHEETS







	DESIGN HEIGHT			MEMB	MBER THICKNESS TOP UNIT REINFORCEMENT (3)									BOTTOM UNIT REINFORCEMENT 3											MATERIAL	QUANTITI	DESIGN VALUES									
DESIGN SPAN S	TOTAL H	TOP UNIT WALL	BOT. UNIT WALL	FILL DEPTH FD	WALLS TW	TOP SLAB TT	BOT. SLAB TB	(S	T1 LAB RIOR)	(SLAB EXTERIO			T3 DRNER)		(W)	ALL RIOR)	T6 (TIES)		T7 OT.)	(S	B1] LAB ERIOR)	(SI	B2 LAB ERIOR)	(	B3 (CORNE	R)	(W	B4] ALL RIOR)	B6 (TIES)	TOF	OUNIT	вотто	OM UNIT	STR. I BEARING DEMAND (PSF)		REQ'D HORIZ JOINT TYPE
<b>(1)</b>		HT	НВ	<b>②</b>			"	SIZE	SPA.	SIZE SI	PA.	SIZE	PA.	TL	SIZE	SPA.	♦	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	SIZE	SPA.	BL	SIZE	SPA.	♦	CONC. (CY/FT)	STEEL (LBS/FT)	CONC. (CY/FT)	STEEL (LBS/FT)	4>	<b>(5)</b>	<u>6</u>
				0' ≤ FD < 2'	12"	16"	14"	#9	6"	#6 1	2"	N	I/A		N	/A	N/A	#6	12"	#9	6"	#6	6"	#7	6"	4'-8"	#4	8"	N/A	1.21	250.9	1.76	445.0	2177	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	16"	14"	#9	6"	#6 1	2"	N	I/A		N	/A	N/A	#4	12"	#9	6"	#6	5"	#7	5"	3'-4"	#4	8"	N/A	1.21	230.9	1.76	471.2	2365	N/A	2
				4'< FD ≤ 6'	12"	18"	16"	#10	6"	#6 1	2"	N	I/A		N	/A	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-5"	#4	8"	N/A	1.35	272.2	1.90	488.9	2777	N/A	2
21'				0' ≤ FD < 2'	12"	14"	12"	#10	6"	#6	3"	#5	8" 3	3'-4"	#4	8"	N/A	#6	9"	#9	6"	#5	6"	#7	6"	3'-7"	#4	8"	N/A	1.43	376.7	1.61	413.5	1762	0.5"	1A
				2' ≤ FD ≤ 4'	12"	16"	14"	#10	6"	#6	3"	#5	8" 3	3'-5"	#4	8"	N/A	#4	12"	#9	6"	#5	6"	#7	6"	3'-7"	#4	8"	N/A	1.57	347.4	1.76	415.4	2015	0.56"	1A
	12' TO 15'	5'	7' TO 10'	4'< FD ≤ 8'	12"	18"	16"	#11	6"	#5	7"	#5	7" 3	3'-7"	#4	8"	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.71	388.5	1.90	490.6	2767	0.66"	1
				8'< FD ≤ 12'	12"	20"	20"	#11	6"	#5	6"	#5	6" 3	3'-9"	#5	9"	Y	#4	12"	#11	6"	#6	6"	#8	6"	3'-9"	#5	9"	Υ	1.86	425.9	2.18	593.0	3660	0.77"	1
				12'< FD ≤ 16'	12"	22"	22"	#11	5"	#6 1	0"	#6	5" 3	B'-11"	#5	8"	Y	#4	9"	#11	6"	#6	10"	#8	5"	4'-0"	#5	8"	Y	2.00	520.4	2.32	658.6	4455	0.85"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD ≤ 2'	12"	14"	14"	#9	6"	#6	5"	#8	5" 5	5'-0"	#4	8"	N/A	#6	12"	#9	6"	#6	5"	#8	5"	4'-9"	#4	8"	N/A	1.76	553.5	1.76	530.3	1830	0.84"	1
				0' ≤ FD < 2'	12"	18"	16"	#9	6"	#6 1	2"	N	I/A		N	/A	N/A	#6	12"	#9	6"	#7	10"	#7	6"	3'-9"	#4	8"	N/A	1.52	282.7	2.05	458.6	2137	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	18"	16"	#10	6"	#6 1	2"	N	V/A		N	/A	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-5"	#4	8"	N/A	1.52	306.5	2.05	526.1	2350	N/A	2
				4'< FD ≤ 6'	12"	18"	18"	#11	6"	#6 1	2"	N	V/A		N	/A	N/A	#4	12"	#10	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.52	358.4	2.21	528.2	2733	N/A	2
				0' ≤ FD < 2'	12"	18"	16"	#10	6"	#6	3"	#5	8" 3	3'-7"	#4	8"	N/A	#6	10"	#8	6"	#7	10"	#6	5"	3'-7"	#4	8"	N/A	1.88	415.5	2.05	405.6	1773	0.51"	1A
24'	12' TO 15'			2' ≤ FD ≤ 4'	12"	18"	16"	#10	6"	#6	3"	#5	8" 3	3'-7"	#4	8"	N/A	#4	12"	#9	6"	#7	10"	#7	5"	3'-7"	#4	8"	N/A	1.88	385.5	2.05	481.4	1984	0.57"	1A
		5'	7' TO 10'	4'< FD ≤ 8'	12"	20"	18"	#11	6"	#6	3"	#6	8" 3	3'-9"	#4	8"	N/A	#4	12"	#10	6"	#5	6"	#8	6"	3'-10"	#4	8"	N/A	2.04	452.8	2.21	536.1	2776	0.66"	1
				8'< FD ≤ 10'	12"	22"	22"	#11	6"	#6	3"	#6	8" 3	8'-11"	#5	9"	Y	#4	9"	#11	6"	#7	10"	#7	5"	3'-11"	#5	9"	Υ	2.20	493.9	2.53	666.2	3260	0.73"	1
				10'< FD ≤ 14'	12"	24"	24"	#10<	9> 4"	#6	7"	#7	7" 4	4'-1"	#5	8"	N/A	#4	9"	#10<	9 4"	#7	10"	#8	5"	4'-1"	#5	8"	N/A	2.36	580.9	2.69	767.5	4150	0.84"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD < 2'	12"	16"	16"	#9	6"	#6	5"	#8	5" 3	3'-10"	#4	8"	N/A	#6	12"	#9	6"	#6	5"	#8	5"	3'-11"	#4	8"	N/A	2.05	578.9	2.05	557.4	1817	0.78"	1
				2' ≤ FD ≤ 4'	12"	16"	16"	#10	6"	#6	5"	#8	5" 4	4'-3"	#4	8"	N/A	#4	12"	#10	6"	#8	8"	#8 <	9> 4"	3'-7"	#4	8"	N/A	2.05	608.0	2.05	656.5	2028	0.92"	1
				0' ≤ FD < 2'	12"	18"	16"	#10	6"	#6 1	2"	N	I/A		N	/A	N/A	#6	10"	#10	6"	#6	6"	#7	6"	3'-9"	#4	8"	N/A	1.68	374.9	2.19	558.0	2107	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	18"	18"	#11	6"	#6 1	2"	N	V/A		N	/A	N/A	#4	12"	#10	6"	#6	5"	#7	5"	3'-9"	#4	8"	N/A	1.68	391.9	2.37	600.1	2401	N/A	2
				4'< FD ≤ 6'	12"	22"	20"	#11	6"	#6 1	0"	N	V/A		N	/A	N/A	#4	9"	#11	6"	#7	6"	#8	6"	3'-9"	#4	8"	N/A	2.04	408.1	2.55	688.8	2855	N/A	2
				0' ≤ FD < 2'	12"	18"	16"	#10	6"	#7	3"	#5	8" 3	3'-7"	#4	8"	N/A	#6	10"	#9	6"	#8	10"	#7	5"	3'-7"	#4	8"	N/A	2.05	479.4	2.19	534.7	1679	0.52"	1A
27'	12' TO 15'	5'	7' TO 10'	2' ≤ FD ≤ 4'	12"	20"	18"	#11	6"	#7	3"	#6	8" 3	3'-9"	#4	8"	N/A	#4	12"	#10	6"	#8	10"	#7	5"	3'-9"	#4	8"	N/A	2.23	518.7	2.37	590.2	1974	0.58"	1A
				4'< FD ≤ 6'	12"	22"	20"	#11	6"	#7	3"	#6	8" 3	3'-11"	#4	8"	N/A	#4	9"	#10	6"	#8	10"	#7	5"	3'-11"	#4	8"	N/A	2.40	538.0	2.55	594.0	2394	0.64"	1
				6'< FD ≤ 10'	12"	24"	24"	#10<	9 4"	#7	9"	#6	9" 4	4'-1"	#4	7"	N/A	#4	9"	#11	5"	#8	10"	#8	5"	4'-1"	#4	7"	N/A	2.58	594.0	2.91	822.9	3266	0.75"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD < 2'	12"	18"	18"	#10	7"	#8 1	0"	#8	5" 3	3'-8"	#4	8"	N/A	#6	12"	#9	6"	#8	10"	#8	5"	3'-10"	#4	8"	N/A	2.37	622.1	2.37	582.7	1782	0.87"	1
				2'< FD ≤ 4'	12"	18"	18"	#10	6"	#8 1	0"	#8	5" 4	4'-0"	#4	8"	N/A	#4	12"	#10	6"	#8	10"	#8	5"	4'-0"	#4	8"	N/A	2.37	636.6	2.37	636.6	2009	0.94"	1
				0' ≤ FD < 2'	12"	20"	18"	#10	6"	#6 1	1"	N	I/A		N	/A	N/A	#6	10"	#11	6"	#7	6"	#8	5"	3'-9"	#4	8"	N/A	2.05	416.4	2.54	751.6	2101	N/A	2
	10'	N/A	10'	2' ≤ FD ≤ 4'	12"	20"	18"	#11	6"	#6 1	1"	N	I/A		N	/A	N/A	#4	12"	#11	6"	#6	5"	#7	5"	3'-9"	#4	8"	N/A	2.05	443.8	2.54	706.3	2322	N/A	2
				4'< FD ≤ 6'	12"	22"	20"	#11	5"	#6 1	0"	N	I/A		N	/A	N/A	#4	9"	#11	5"	#6	5"	#8	5"	4'-0"	#4	8"	N/A	2.25	518.5	2.74	824.7	2737	N/A	2
				0' ≤ FD < 2'	12"	20"	18"	#11	6"	#7	7"	#5	7" 3	3'-9"	#4	8"	N/A	#6	9"	#10	6"	#6	5"	#7	5"	3'-9"	#4	8"	N/A	2.41	610.8	2.54	642.3	1713	0.48"	1A
30'	12' TO 15'	5'	7' TO 10'	2' ≤ FD ≤ 4'	12"	22"	20"	#11	6"	#7	7"	#5	7" 3	3'-1 <mark>1</mark> "	#4	8"	N/A	#4	9"	#10	6"	#6	5"	#7	5"	3'-11"	#4	8"	N/A	2.61	588.7	2.74	646.1	1999	0.59"	1
				4'< FD ≤ 6'	12"	24"	22"	#11	5"	#7	7"	#6	7" 4	4'-1"	#4	8"	N/A	#4	9"	#11	6"	#6	5"	#8	5"	4'-1"	#4	8"	N/A	2.81	667.9	2.93	817.7	2414	0.65"	1
				6'< FD ≤ 8'	12"	24"	24"	#10<	9> 4"	#7	7"	#6	7" 4	4'-1"	#4	8"	N/A	#4	9"	#11	5"	#6	5"	#8	5"	4'-1"	#4	8"	N/A	2.81	679.5	3.13	885.4	2821	0.71"	1
	17' TO 20'	7' TO 10'	10'	0' ≤ FD < 2'	12"	18"	18"	#10	6"	#6	5"	#8	6" 4	4'-2"	#4	8"	N/A	#6	10"	#10	6"	#6	5"	#8	5"	3'-9"	#4	8"	N/A	2.54	675.6	2.54	685.5	1743	0.8"	1
				2' ≤ FD ≤ 4'	12"	20"	20"	#11	6"	#6	5"	#8	5" 3	3'-9"	#4	8"	N/A	#4	12"	#11	6"	#6	5"	#8	5"	3'-9"	#4	8"	N/A	2.74	752.1	2.74	752.1	2028	0.95"	1

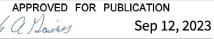
- DESIGN SPAN 'S', SHALL BE TAKEN EQUAL TO THE HORIZONTAL DISTANCE BETWEEN INSIDE FACE OF WALLS AT THE CORNER BETWEEN THE FILLET AND THE WALL AS SHOWN ON SHEET 1. FOR SEGMENTS WITH A SKEW SEE GRAPHICAL CLARIFICATION OF DESIGN SPAN ON SHEET 3. IF THE DESIGN SPAN FALLS INBETWEEN A DESIGN SPAN VALUE IN THE TABLES. USE REINFORCEMENT FOR THE LONGER SPAN.
- 2 THE MAXIMUM FILL DEPTH OVER THE ENTIRE BURIED STRUCTURE SHALL BE USED WHEN SELECTING DESIGN FILL DEPTH.
- (3) FOR REINFORCING DETAILS SEE SHEET 4.
- 4 MAXIMUM REQUIRED BEARING RESISTANCE FOR STRENGTH LIMIT STATE.
- (5) THE STRUCTURE HAS BEEN DESIGNED FOR THE RACKING DISPLACEMENTS PRESENTED HERE. THESE DESIGN VALUES ARE DETERMINED BASED ON THE DESIGN CRITERIA AS OUTLINED IN THE BDM. THESE STRUCTURES HAVE BEEN DESIGNED FOR A TRAVERSE DIFFERENTIAL SETTLEMENT DISPLACEMENT EQUAL TO 2 INCHES PER 100 FEET OF STRUCTURAL SPAN (DESIGN SPAN + 1.0 FOOT).
- 6 FOR HORIZONTAL JOINT TYPE DETAILS SEE SHEET 10.
- (7) WALL TIES T6 AND B6 ARE REQUIRED WHEN INDICATED BY LETTER 'Y'.
- MATERIAL QUANTITIES ARE FOR TYPICAL INTERIOR SEGMENT. STEEL WEIGHT DOES NOT INCLUDE ADDITIONAL REINFORCING REQUIRED FOR APPROACH SLAB SEAT.
- 9 SINGLE BARS SPACED AT 4 INCHES MAY BE SUBSTITUTED BY TWO-BAR BUNDLES SPACED AT 8".
- (1) QUANTITIES OF BOTTOM UNIT CORRESPOND TO 'HB' = 10'-0".
- (1) SEE SHEET 5 FOR HANDLING AND SHIPPING ORIENTATIONS.



## BURIED STRUCTURE SPLIT BOX

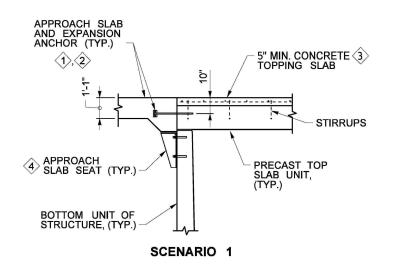
#### STANDARD PLAN E-20.10-00

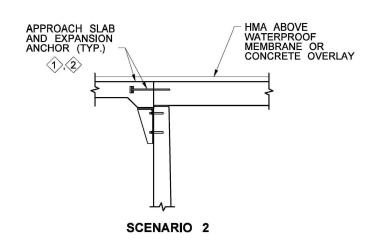
SHEET 7 OF 17 SHEETS

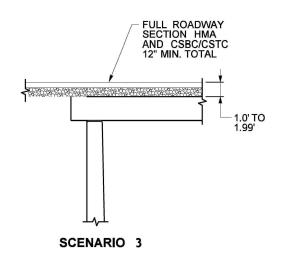


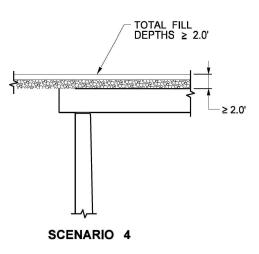
STATE DESIGN ENGINEER



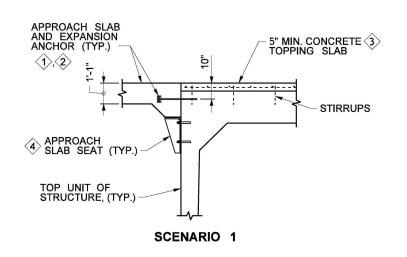


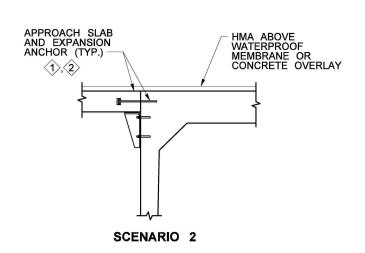


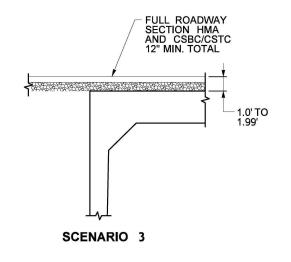


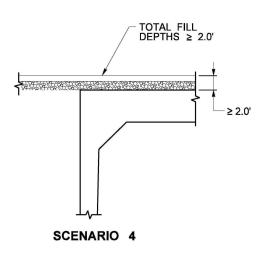


#### SPLIT BOX ~ SLAB ROADWAY CONDITIONS









#### SPLIT BOX ROADWAY CONDITIONS

	DESIGN OPTIONS	
	APPROACH SLAB SEAT	
CONCRE	ETE:	
	C.I.P.	SHEET 15
	C.I.P. END DIAPHRAGM	SHEET 17
STEEL:		
	W/O RESTRAINER	SHEET 13
	W/RESTRAINER	SHEET 14
	SEISMIC RESTRAINERS	
LONGITU	JDINAL:(REQ'D FOR SPLIT BO	DX-SLABS)
	EXTENDED SEAT WIDTH	SHEET 10
	OR PROVIDE AN:	
	APPROACH SLAB	SHEET 13-15
TRANSV	ERSE:	
	STEEL APPR. SEAT	SHEET 14
	ACCEL. BRIDGE CONST.	SHEET 16
	C.I.P. END DIAPHRAGM	SHEET 17

TOP UNIT LONGITUDINAL JOINT SELECTION CRITERIA 5										
ROADWAY CONDITION	SCENARIO 1	SCENA	RIO 2	SCENARIO 3	SCENARIO 4					
SCENARIO DESCRIPTION	5" MIN CONCRETE TOPPING SLAB	HMA OR CO		12" MIN. FULL ROADWAY SECTION (HMA & CSBC)	ROADWAY SECTION AND/OR BACKFILL					
FILL DEPTH 'FD'	TOPPING SLAB	OVER	LAY	1' TO 1.99'	≥ 2.0'					
VERTICAL SHEAR TRANSFER ACROSS JOINTS OF ADJACENT SEGMENTS	NOT REQUIRED	REQUI	RED	NOT REQUIRED	NOT REQUIRED					
ACCEPTABLE JOINT TYPE	TYPE 2A	TYPES	3 & 4	TYPES 1, 2 & 3	TYPES 1, 2 & 3					
EPOXY COATED REINF. REQUIRED IN TOP SLAB	NO	YE	S	YES	NO					
CLEAR COVER TO TOP MAT OF REINF.	2"	HMA 2.5"	CONC. 2"	2"	2"					
LONG. CONNECTION AT TOP SLAB BETWEEN FIRST THREE SEGMENTS AT EACH END	NOT REQUIRED	WELD (SEE SHI		LONGITUDINAL TIE PLATES (SEE SHEETS 2 AND 8)						

#### **KEY NOTES**

- 1> PROVIDE APPROACH SLAB WHEN REQUIRED IN THE CONTRACT DOCUMENTS.
- APPROACH SLAB AND EXPANSION ANCHOR SHALL BE PER STANDARD PLAN A-40.50 AND PER ADDITIONAL DETAILS ON SHEET
- 3 SEE SHEET 11 FOR 5" MIN. CONCRETE TOPPING SLAB DETAILS.
- 4 FOR APPROACH SLAB SEAT DETAILS SEE SHEETS 13, 14, 15, AND 17.
- 5 SEE SHEET 9 FOR TYPES AND DETAILS OF TOP SLAB LONGITUDINAL JOINTS.



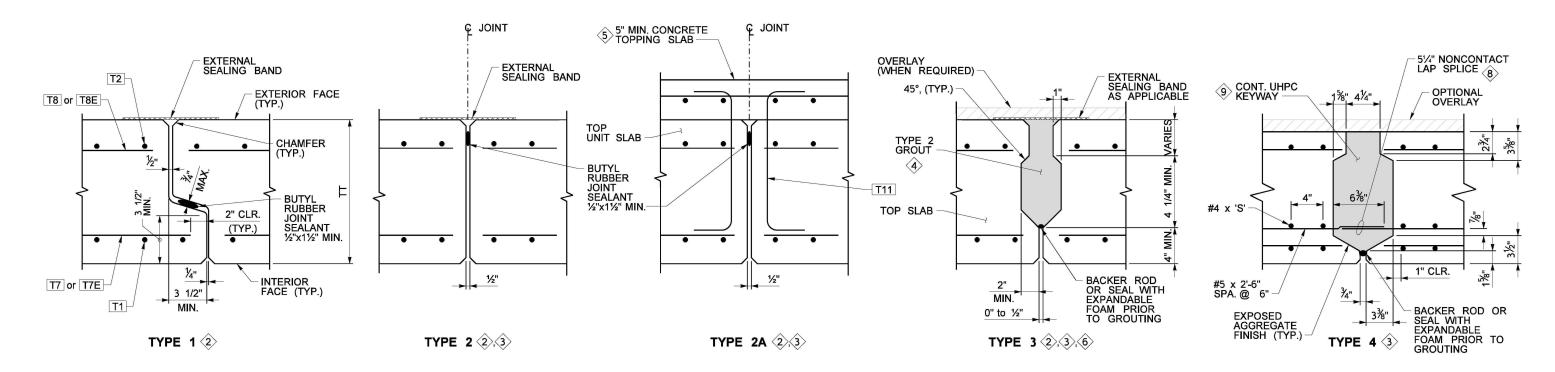
Sep 12, 2023

# SPLIT BOX STANDARD PLAN E-20.10-00

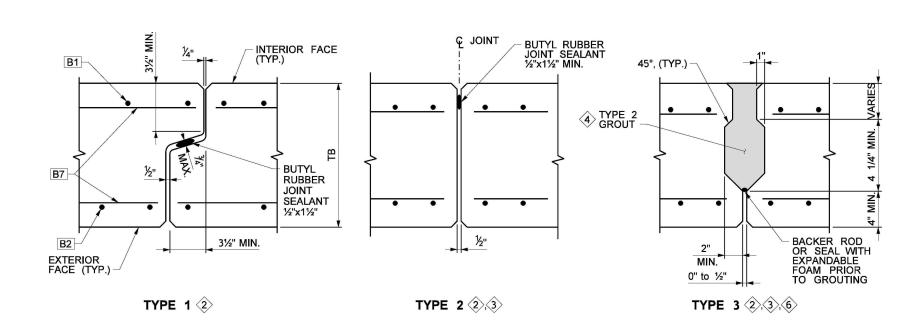
**BURIED STRUCTURE** 

SHEET 8 OF 17 SHEETS

APPROVED FOR PUBLICATION Sep 12, 2023 (Lours STATE DESIGN ENGINEER



ELEVATION
TOP SLAB LONGITUDINAL JOINTS ALTERNATIVES (1)



ELEVATION
BOTTOM SLAB LONGITUDINAL JOINTS ALTERNATIVES

- 1) SEE SHEET 8 FOR JOINT TYPE SELECTION CRITERIA
- THIS JOINT CAN BE USED IN COMBINATION WITH WELD TIES. SEE SHEET 12 FOR DETAILS.
- SEE JOINT TYPE 1 FOR TYPICAL REINFORCING AND DETAILS NOT SHOWN.
- GROUT SHALL CONFORM TO STANDARD SPEC. 9.20.3(2)
  AND SHALL ACHIEVE A MINIMUM COMPRESSIVE
  STRENGTH OF 2,500 PSI BEFORE BACKFILL PLACEMENT
- (5) SEE SHEET 11 FOR 5" MIN. CONCRETE TOPPING SLAB DETAILS.
- MANUFACTURER VARIATIONS OR NOMINAL
  ADJUSTMENTS TO THE DETAILED JOINT GEOMETRICS
  SHALL BE SHOWN IN THE FABRICATION SHOP
  DRAWINGS
- BOTTOM SLAB LONGITUDINAL JOINT TYPE 1, TYPE 2
  OR TYPE 3 ARE PERMISSIBLE. THE SAME JOINT TYPE
  SHALL BE USED FOR BOTH TOP AND BOTTOM UNITS
  FOR EACH STRUCTURE EXCEPT TYPE 4 IS ONLY
  REQUIRED FOR TOP JOINT. JOINT TYPE SHALL NOT BE
  INTERCHANGED WITHIN THE STRUCTURE.
- 8 STAGGER EXTENDED #5 BAR BETWEEN TOP UNIT SLABS TO PROVIDE 3" CENTER-TO-CENTER SPACING.
- REFER TO SPECIAL PROVISIONS FOR UHPC REQUIREMENTS.

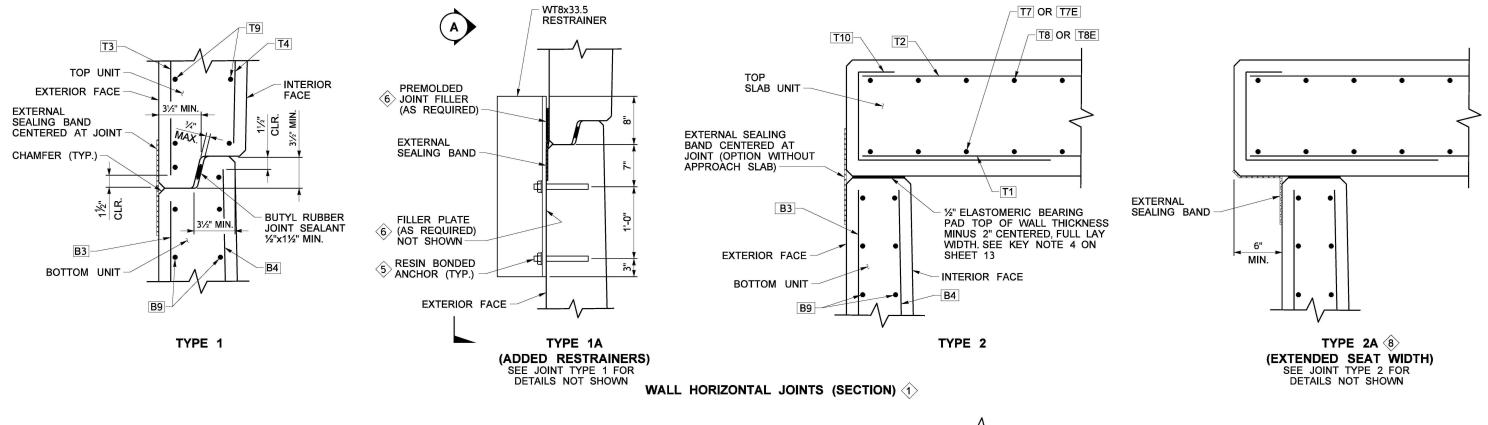


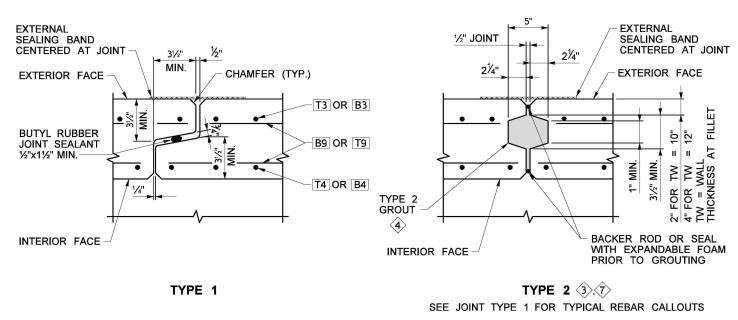
# SPLIT BOX STANDARD PLAN E-20.10-00

**BURIED STRUCTURE** 

SHEET 9 OF 17 SHEETS







## WALL VERTICAL JOINTS (PLAN) ②

#### **KEY NOTES:**

- SEE DESIGN TABLES FOR APPLICABLE WALL HORIZONTAL JOINT TYPES.
- 2 EITHER VERTICAL WALL JOINT TYPE 1 OR TYPE 2 IS PERMISSIBLE.
- MANUFACTURER VARIATIONS OR NOMINAL ADJUSTMENTS TO THE DETAILED JOINT GEOMETRICS SHALL BE SHOWN IN THE FABRICATION SHOP DRAWINGS.
- THE CONTRACTOR SHALL PLACE THE GROUT BEFORE PLACING THE BACKFILL ON TOP OR AGAINST THE SIDES OF THE STRUCTURE. GROUT SHALL CONFORM TO STANDARD SPEC. 9.20.3(2) AND SHALL ACHIEVE A
  MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI BEFORE BACKFILL
  - ANCHORS SHALL BE 7/8" DIA. SET IN 6" LONG HOLES. MANUFACTURER AND CONTRACTOR SHALL COORDINATE SPACING OF WALL REINFORCING BARS AND LOCATION OF RESIN BONDED ANCHORS TO AVOID CONFLICT DURING INSTALLATION.

### EXTERNAL SEALING BAND 7/8" Ø RESIN BONDED ANCHOR IN 1"x2" HORIZONTALLY SLOTTED HOLE (TYP.) (5) **( (C) (** (C)) **© ( © (** 3" | 3" LW/4 LW/4 LW/2 ¢ RESTRAINER (TYP.) LAY WIDTH OF SEGMENT, LW

SECTION ( A

- 6 IF EXTERIOR SURFACES OF TOP AND BOTTOM UNITS ARE MISALIGNED BY 1/4" OR GREATER, PLACE PREMOLDED JOINT FILLER (PMJF) IN THE GAP BETWEEN RESTRAINER AND TOP UNIT OR STEEL FILLER PLATE IN THE GAP AT THE BOTTOM UNIT. FILLER PLATE OR PMJF SHALL BE SIZED FOR THE FULL WIDTH AND HEIGHT OF CONTACT AREA WITH WT RESTRAINER.
- THIS JOINT CAN BE USED IN COMBINATION WITH WELD TIES.
  ALTERNATIVELY, SHEAR KEY AND GROUT MAY BE ELIMINATED IF WELD TIES ARE UTILIZED. SEE SHEET 12 FOR WELD TIE DETAILS.
- (8) THIS JOINT SHALL BE USED FOR CLASS 2 STRUCTURES WITHOUT AN APPOACH SLAB.

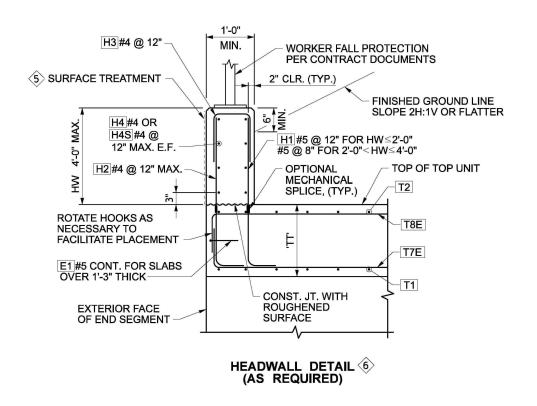
#### **BURIED STRUCTURE** SPLIT BOX

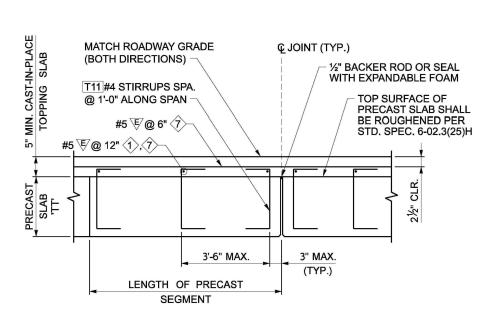
Sep 12, 2023

#### STANDARD PLAN E-20.10-00

SHEET 10 OF 17 SHEETS



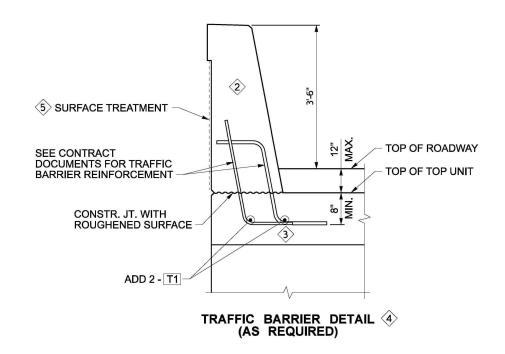


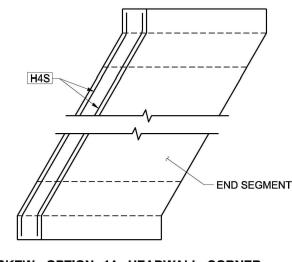


# 5" MINIMUM CONCRETE TOPPING SLAB DETAIL (AS REQUIRED)

CONSTRUCTION AIDS (E.G. WELD TABS) CAN BE PROVIDED AT CONTRACTOR'S DISCRETION.

IF TOPPING SLAB THICKNESS EXCEEDS 7", ADD SECOND MAT OF #5 REINFORCEMENT





SKEW OPTION 1A HEADWALL CORNER REINFORCEMENT DETAIL

## KEY NOTES

- RUN PARALLEL TO SKEW FOR SKEWED SEGMENTS
- FOR TRAFFIC BARRIER DETAILS SEE CONTRACT DOCUMENTS
- TOP UNIT REINFORCEMENT IS NOT SHOWN. SEE HEADWALL DETAIL THIS SHEET FOR TYPICAL REINFORCING IN TOP SLAB OF END UNIT.
- TOP SLAB DESIGN IS BASED ON A SINGLE SLOPE 42" TRAFFIC BARRIER (TL4) WITH MAX 12" FILL OR ROADWAY ON SLAB.
- SURFACE TREATMENT(S) SHALL BE PROVIDED AS FOLLOWS; TRAFFIC BARRIER SHALL BE FRACTURED FIN, HEADWALLS, WINGWALLS, AND STRUCTURES SHALL BE IN ACCORDANCE WITH STD. SPEC. 6-02.3(14). UNLESS OTHERWISE SHOWN IN THE CONTRACT DOCUMENTS.
- A PERMANENT GEOSYNTHETIC WALL MAY BE EMPLOYED AS A HEADWALL (SEE STANDARD PLAN D-3.09 FOR DETAILS).
- 7 2'-0" MIN. LAP SPLICE. ALTERNATE LAP SPLICE LOCATION ON ADJACENT REINFORCEMENT SO THAT NO MORE THAN 50% OF REINFORCEMENT IS SPLICED IN THE SAME LOCATION.

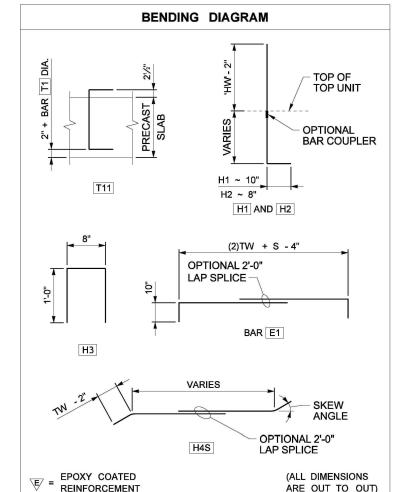


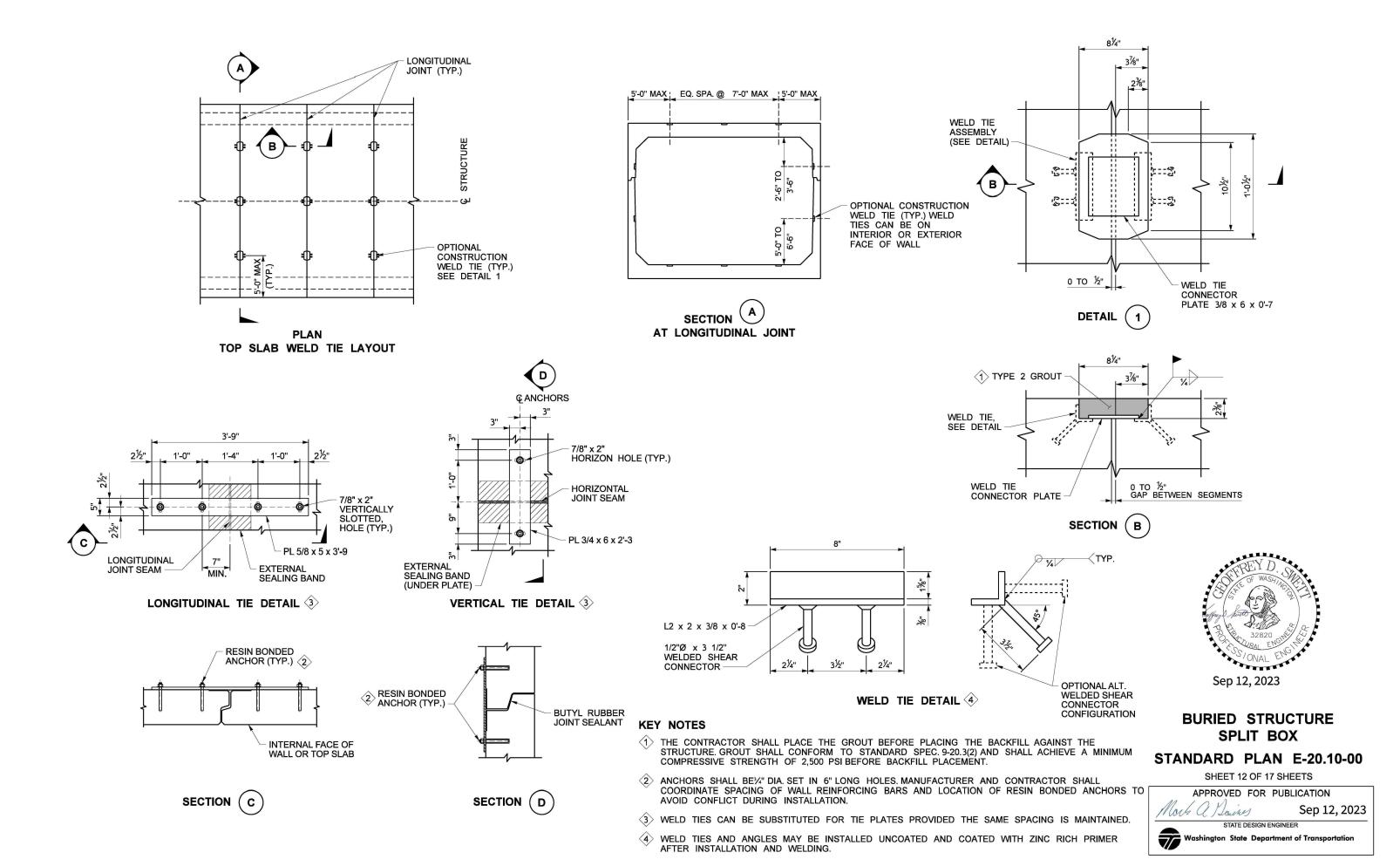
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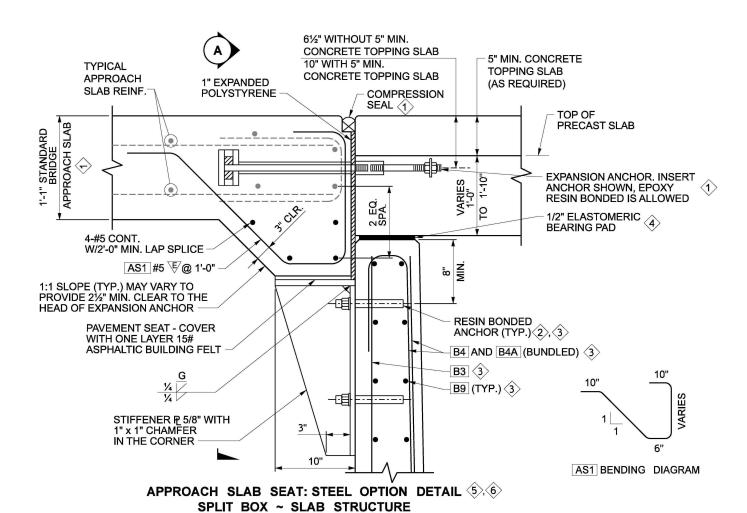
SHEET 11 OF 17 SHEETS

**BURIED STRUCTURE** 









APPROACH SLAB SEAT (STEEL BRACKET OPTION SHOWN)

APPROACH SLAB SEAT: STEEL OPTION DETAIL \$5,6 SPLIT BOX STRUCTURE

SAWCUT AND SEAL PER STD. PLAN

A-40.20 DETAIL 5. FOR CONCRETE

HMA OVERLAY

OVERLAY USE DETAIL 7.

FOR DETAILS NOT SHOWN SEE APPROACH SLAB SEAT: STEEL OPTION DETAIL SPLIT BOX ~ SLAB STRUCTURE THIS SHEET.
FOR REINFORCEMENT NOT SHOWN SEE TYPICAL SECTION ON SHEET 4.

APPROACH SLAB

#### **KEY NOTES**

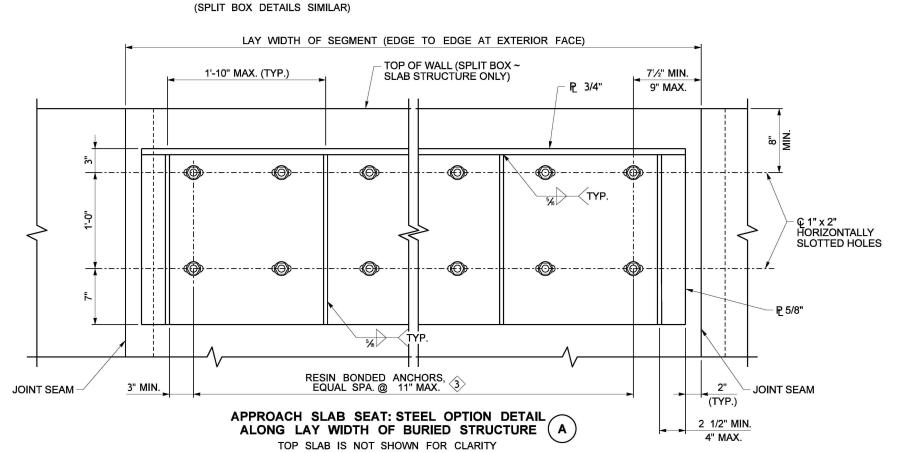
- FOR APPROACH SLAB DETAILS INCLUDING REINFORCING, EXPANSION ANCHOR, COMPRESSION SEAL, AND OTHER, SEE STANDARD PLAN A-40.50.
- ANCHORS SHALL BE 7/8" DIA. SET IN 6-INCH LONG HOLES.
- MANUFACTURER AND CONTRACTOR SHALL COORDINATE SPACING OF WALL REINFORCING BARS AND LOCATION OF RESIN BONDED ANCHORS TO AVOID CONFLICT DURING INSTALLATION.
- THE CONTRACTOR MAY ADD A 1" MINIMUM THICK GROUT PAD FOR LEVELING TOP OF BOTTOM UNIT PRIOR TO SETTING BEARING PAD. ADJUST FINAL ELEVATIONS AS NEEDED. THE CONTRACTOR MAY SUBMIT ALTERNATE METHODS OF VERTICAL ADJUSTMENT TO THE ENGINEER FOR ACCEPTANCE.
- 5 ADDITIONAL REINFORCEMENT IS REQUIRED FOR TOP AND BOTTOM UNITS, SEE SHEET 4 FOR DETAILS.
- FOR DETAILS OF STEEL APPROACH SLAB SEAT OPTION WITH SEISMIC RESTRAINER SEE SHEET 14. FOR DETAILS OF C.I.P. APPROACH SLAB SEAT OPTION SEE SHEET 15.

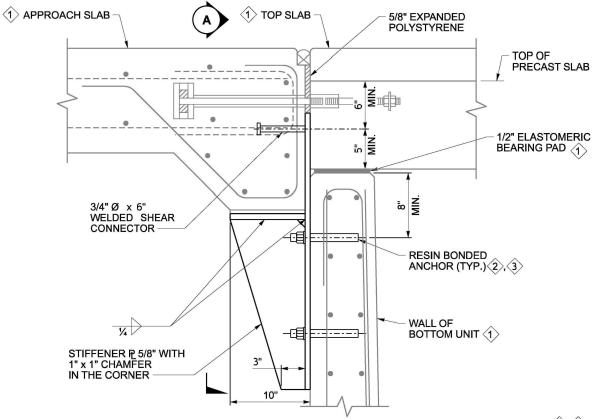


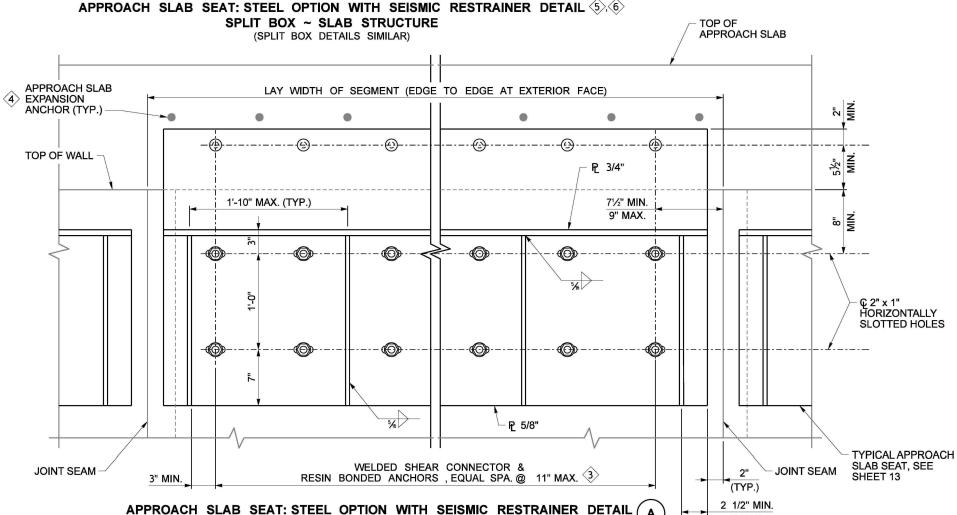
## BURIED STRUCTURE SPLIT BOX STANDARD PLAN E-20.10-00

SHEET 13 OF 17 SHEETS









ALONG LAY WIDTH OF BURIED STRUCTURE

4" MAX.

# SEISMIC LATERAL FORCE RESTRAINER SELECTION CRITERIA

- . Seismic lateral force restraint is required for Split Box ~ Slab structures with less than 2 feet of Fill Depth. Options for Lateral Seismic Restraint are as follows:
  - a. Steel Option with Seismic Restrainer. See details this Sheet.
  - b. Accelerated Bridge Construction (ABC) Option. See details on Sheet 16.
  - c. C.I.P. End Diaphragm with Girder Stop Option. See Sheet 17.
- Seats with lateral restrainers shall be installed symmetrically in both walls of the segment. Minimum two segments with lateral restrainers are required along the length of buried structure. The total length (feet) of seats with lateral restrainers along each wall shall not be less than LR = w L PGA(2R), where:
  - w (kip/ft) Weight per linear foot of structure and roadway above the walls
  - L (ft) Total length of structure
  - PGA Effective Site Peak Ground Acceleration Coefficient
  - R = 10 kip/ft Average shear resistance of anchors in the approach slab per foot length of bracket

#### **KEY NOTES**

- 1) FOR DETAILS AND NOTATIONS NOT SHOWN SEE SHEET 13.
- 2 ANCHORS SHALL BE 7/8" DIA. SET IN 6-INCH LONG HOLES.
- MANUFACTURER AND CONTRACTOR SHALL COORDINATE SPACING OF WALL REINFORCING BARS AND LOCATION OF RESIN BONDED ANCHORS TO AVOID CONFLICT DURING INSTALLATION.
- 4 ARRANGE SPACING OF APPROACH SLAB EXPANSION ANCHORS AND WELDED SHEAR CONNECTORS TO PROVIDE MINIMUM 3 INCH DISTANCE BETWEEN.
- ADDITIONAL REINFORCEMENT IS REQUIRED FOR TOP AND BOTTOM UNITS, SEE SHEET 4 FOR DETAILS.
- FOR DETAILS OF STEEL APPROACH SLAB SEAT OPTION WITHOUT SEISMIC RESTRAINER SEE SHEET 13. FOR DETAILS OF C.I.P. APPROACH SLAB SEAT OPTION SEE SHEET 15.



# SPLIT BOX STANDARD PLAN E-20.10-00

**BURIED STRUCTURE** 

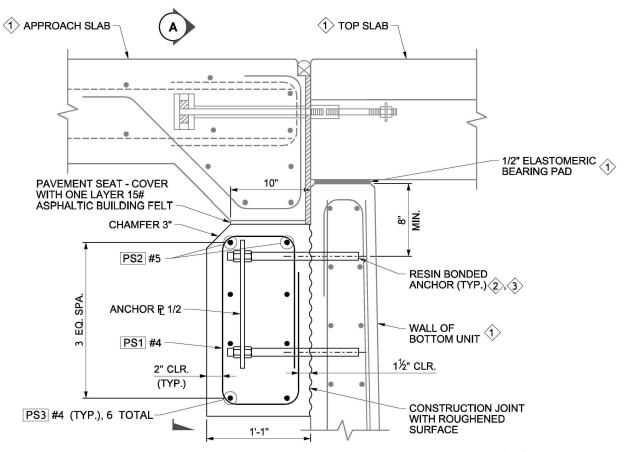
SHEET 14 OF 17 SHEETS

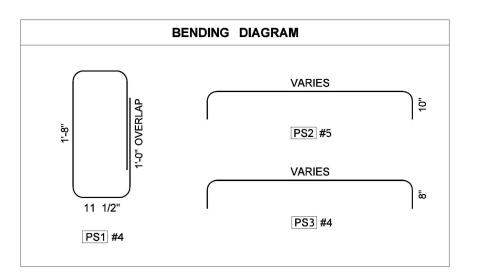
APPROVED FOR PUBLICATION

Moch a Source Sep 12, 2023

STATE DESIGN ENGINEER

Washington State Department of Transporta

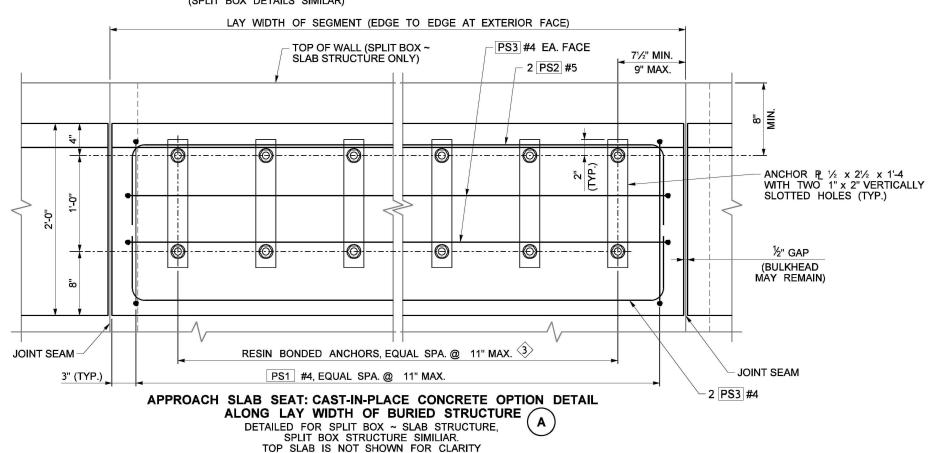




APPROACH SLAB SEAT: CAST-IN-PLACE CONCRETE OPTION DETAIL 4,5

SPLIT BOX ~ SLAB STRUCTURE

(SPLIT BOX DETAILS SIMILAR)



#### **KEY NOTES**

- FOR DETAILS AND NOTATIONS NOT SHOWN SEE SHEET 13.
- 2 ANCHORS SHALL BE 7/8" DIA. SET IN 6-INCH LONG HOLES.
- MANUFACTURER AND CONTRACTOR SHALL COORDINATE SPACING OF WALL REINFORCING BARS AND LOCATION OF RESIN BONDED ANCHORS TO AVOID CONFLICT DURING INSTALLATION.
- ADDITIONAL REINFORCEMENT IS REQUIRED FOR TOP AND BOTTOM UNITS, SEE SHEET 4 FOR DETAILS.
- 5 FOR DETAILS OF STEEL APPROACH SLAB SEAT OPTION WITHOUT SEISMIC RESTRAINER SEE SHEET 13. FOR DETAILS OF STEEL APPROACH SLAB SEAT OPTION WITH SEISMIC RESTRAINER SEE SHEET 14.

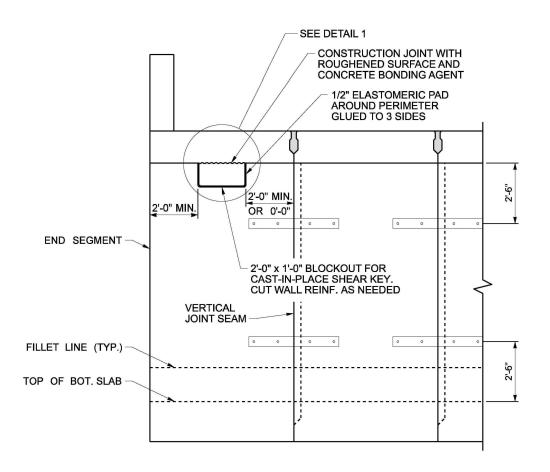


# SPLIT BOX STANDARD PLAN E-20.10-00

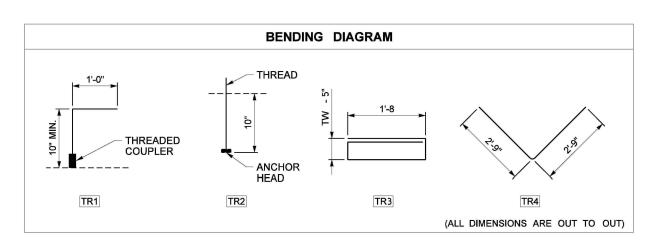
**BURIED STRUCTURE** 

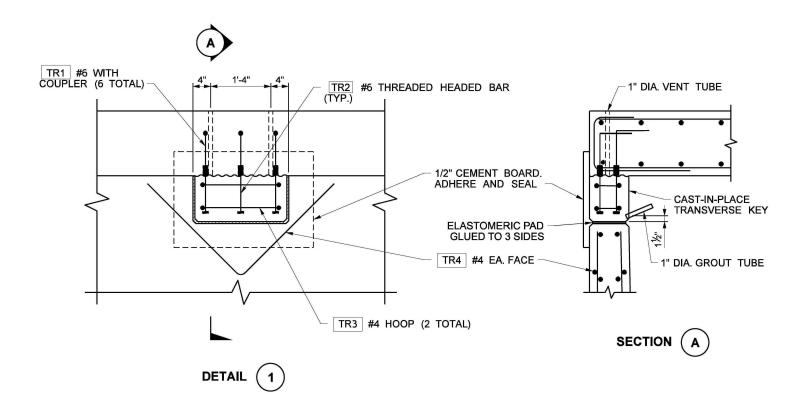
SHEET 15 OF 17 SHEETS





SEISMIC LATERAL FORCE RESTRAINER
ACCELERATED BRIDGE CONSTRUCTION (ABC) OPTION
SPLIT BOX ~ SLAB STRUCTURE





#### SEISMIC LATERAL FORCE RESTRAINER SELECTION CRITERIA:

- Seismic lateral force restraint is required for Split Box ~ Slab structures with less than 2 feet of Fill Depth.
   Options for Lateral Seismic Restraint are as follows:
  - a. Accelerated Bridge Construction (ABC) Option. See details this Sheet.
  - b. Steel Option with Seismic Restrainer. See details on Sheet 14.
  - c. C.I.P. End Diaphragm with Girder Stop Option. See Sheet 17.
- Two lateral restrainers shall be installed symmetrically in both walls of the segment. Minimum two segments with lateral restrainers are required along the length of buried structure. The number of segments with lateral restrainers can be calculated using formula: N = w L PGA /(2R) (rounded up to integer), where:
  - w (kip/ft) Weight per linear foot of structure and roadway above the walls
  - L (ft) Total length of structure
  - PGA Effective Site Peak Ground Acceleration Coefficient
  - R = 100 kips, shear key lateral resistance
- The lateral restrainers shall be installed in the end segments and spaced equally along the length of the structure.
- Shear key material shall be self-consolidating concrete placed using a 'bird feeder' technique, or a Type 2 Grout for Nonshrink Applications in accordance with Std. Spec. 9-20.3(2), placed via grout tubes.
- 5. Shear keys can be installed after backfilling and open to traffic when applicable
- 6. This option not applicable with approach slabs.



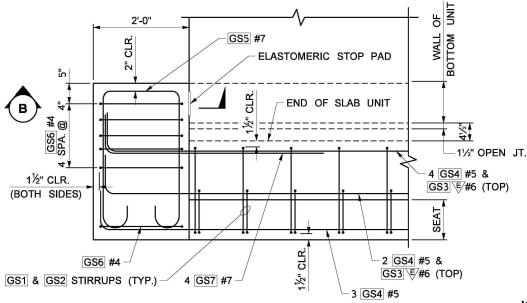
## BURIED STRUCTURE SPLIT BOX STANDARD PLAN E-20.10-00

SHEET 16 OF 17 SHEETS



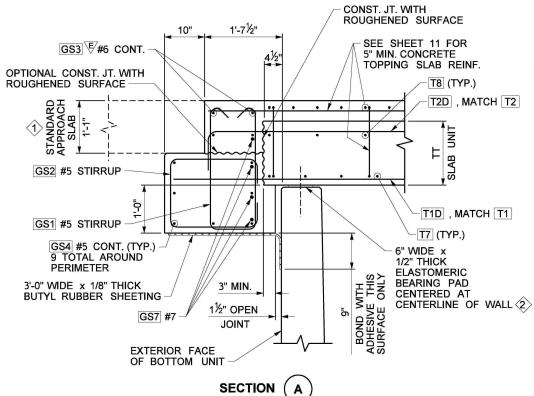
# TRAFFIC BARRIER, FOR DETAILS SEE SHEET 11 APPROACH SLAB SEAT 2 GS3 #6 CONT. A GS6 #4 GS5 #7 (TYP.) (NEAR FACE) GS1 #5 AND GS2 #5 STIRRUPS SPA. @ 1'-0" MAX

ELEVATION
SEISMIC LATERAL FORCE RESTRAINER
C.I.P. END DIAPHRAGM WITH GIRDER STOP OPTION DETAIL
SPLIT BOX ~ SLAB STRUCTURE



PLAN
SEISMIC LATERAL FORCE RESTRAINER
C.I.P. END DIAPHRAGM WITH GIRDER STOP OPTION DETAIL
SPLIT BOX ~ SLAB STRUCTURE

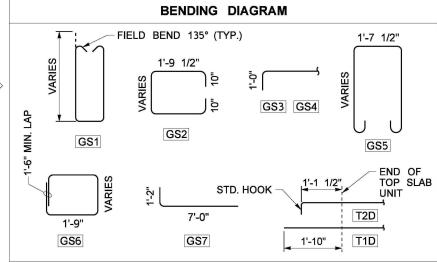
5" MIN. CONCRETE TOPPING SLAB REINFORCING NOT SHOWN FOR CLARITY

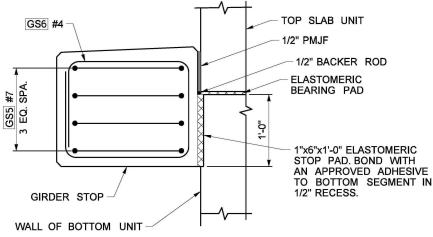


# SEISMIC LATERAL FORCE RESTRAINER SELECTION CRITERIA

- Seismic lateral force restraint is required for Split Box ~ Slab structures with less than 2 feet
  of Fill Depth. C.I.P. End Diaphragm with Girder Stop details may be used for Split Box ~
  Slab structures with 5" min. concrete topping slab and approach slab. Options for Lateral
  Seismic Restraint are as follows:
  - a. C.I.P. End Diaphragm with Girder Stop Option. See details this Sheet.
  - b. Steel Option with Seismic Restrainer. See details on Sheet 14.
  - c. Accelerated Bridge Construction (ABC) Option. See Sheet 16.
- 2. Partial length of the structure is shown. End Diaphragm must be installed along a full length of both walls. Four girder stops shall be installed (two at each end of the structure).
- 3. Lateral resistance of each girder stop, R=80 kips. Earthquake force per one girder stop shall not exceed its resistance. Earthquake force can be calculated using formula:
- V = 0.5 w L PGA, where:
- w (kip/ft) Weight per linear foot of structure and roadway above the wall
- L (ft) Total length of structure

PGA - Effective Site Peak Ground Acceleration Coefficient





#### **KEY NOTES**

- FOR APPROACH SLAB DETAILS INCLUDING REINFORCING, EXPANSION ANCHOR, COMPRESSION SEAL, AND OTHER, SEE STANDARD PLAN A-40.50.
- THE CONTRACTOR MAY ADD A 1" MINIMUM THICK GROUT PAD FOR LEVELING TOP OF BOTTOM UNIT PRIOR TO SETTING BEARING PAD. ADJUST FINAL ELEVATIONS AS NEEDED. THE CONTRACTOR MAY SUBMIT ALTERNATIVE METHODS OF VERTICAL ADJUSTMENT TO THE ENGINEER FOR ACCEPTANCE.



SECTION

	JM LAP LENTGH
#4	2'-1"
#5	2'-7"
#6	3'-1"
#6	3'-8"
#7	3'-7"



# SPLIT BOX STANDARD PLAN E-20.10-00

**BURIED STRUCTURE** 

SHEET 17 OF 17 SHEETS

