AECOM

2023 Western Bridge Engineer’s Seminar

The Gordie Howe International Bridge Project

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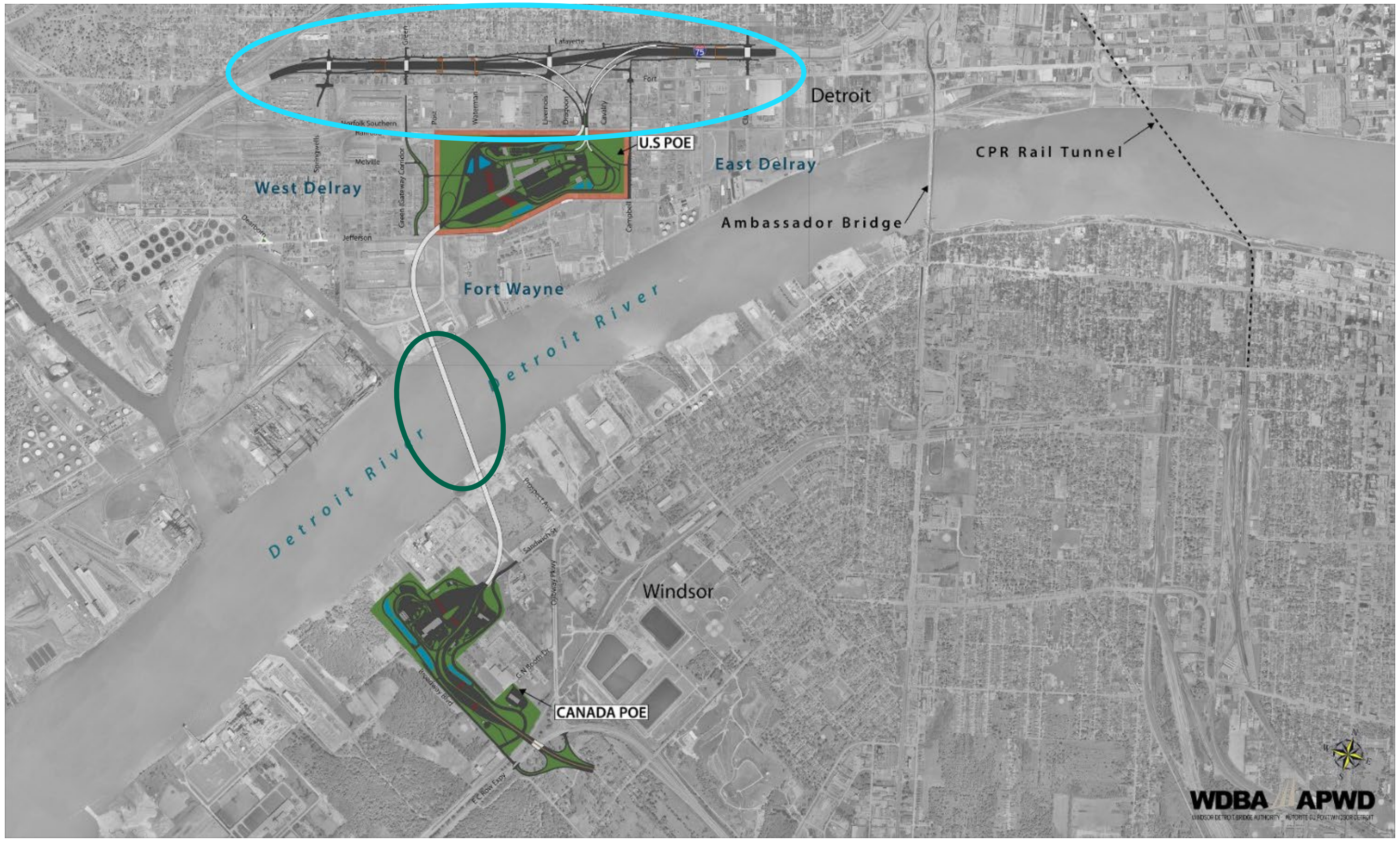
Agenda

- Introduction
- MAIN BRIDGE
- Bridge type and layout
- Foundations/Towers
- Superstructure
- Wind Design
- Stay Cables
- Durability
- Key Dates
- MICHIGAN INTERCHANGE
  - Local Road and Connecting Ramps Bridges
  - LTPs and Gateway Towers
  - Unique Features
  - Redundancy Analysis
- Acknowledgements

Introduction: Bridging North America

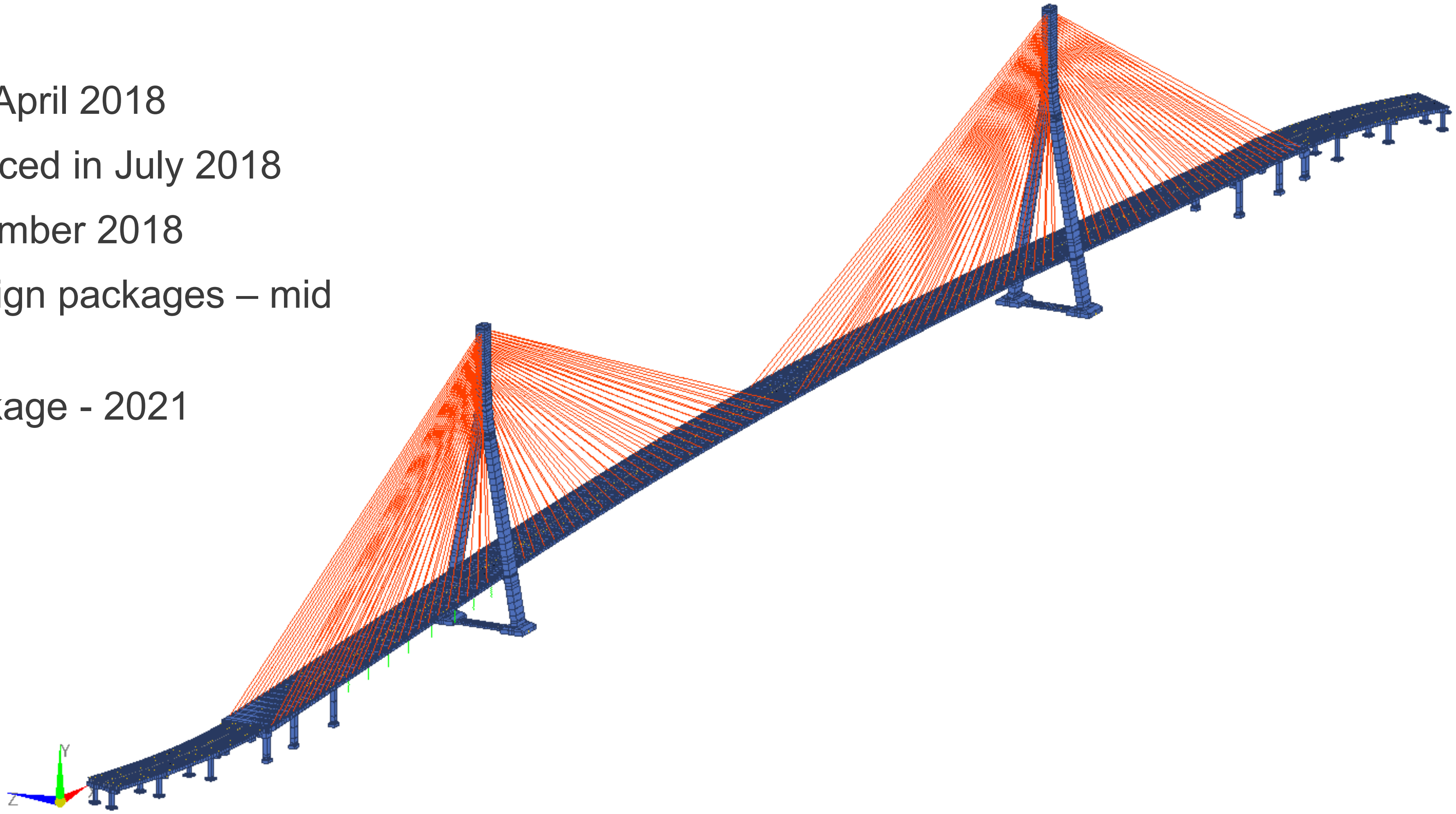
Introduction:

- Four major components:
  - Canadian POE
  - The Bridge
  - US POE
  - Michigan Interchange
- Contract Value: $5.7B
- Substantial Completion: Late 2024
- OM Period: 30 years after construction



Introduction: Design Timeline

- The RFP was released in November 2016
- Bid design start – early 2017
- Technical Submissions Due April 2018
- Preferred Proponent announced in July 2018
- Financial close was in September 2018
- Main Bridge early works design packages – mid 2019
- Main Bridge last design package - 2021



Bridge type and layout

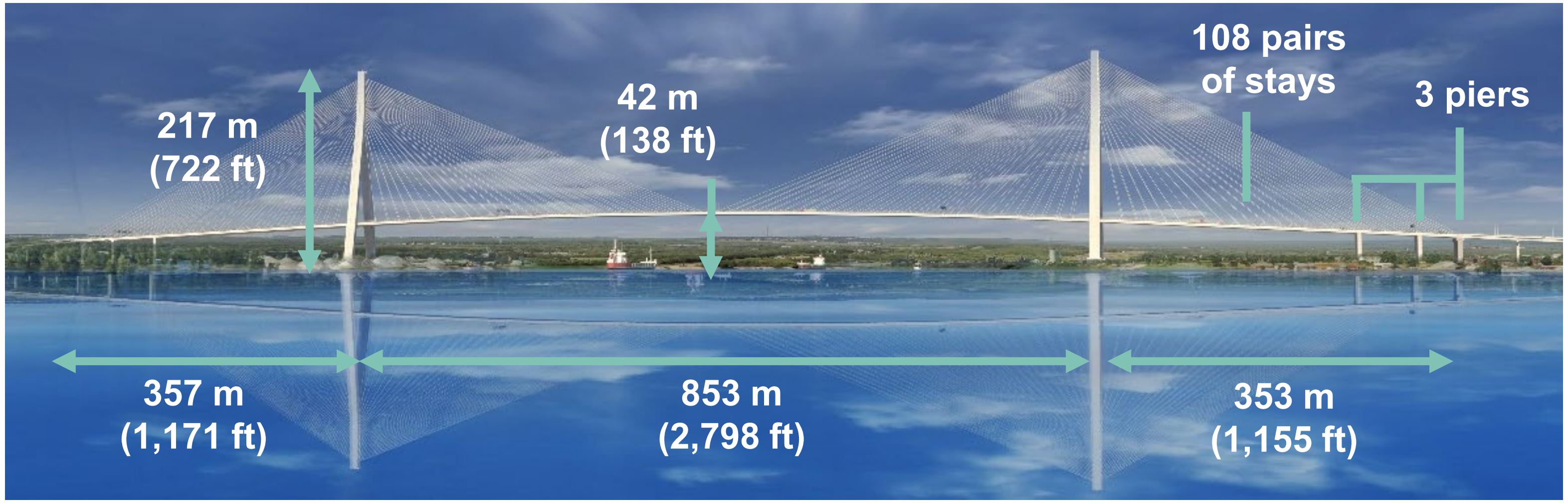
Cable Stayed Bridge

- 853m / 2,798ft main span
- 357m / 1,171ft US side span
- 352m / 1,155ft CAN side span
- 2 side span piers and 1 anchor pier
- 217m / 722 ft tower height
- 42m / 138 ft vertical clearance

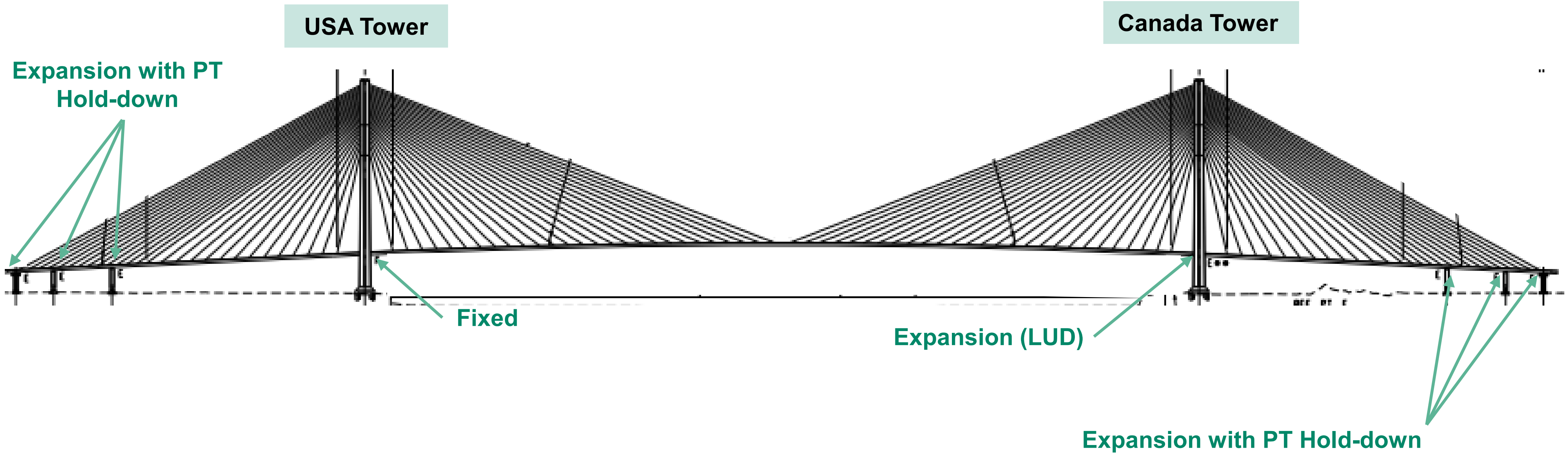
Longest CS Bridge in North America

10th Longest CS Bridge in World

Longest Composite Deck CS bridge in the World



Bridge type and layout: articulation



Bearings

- Vertical Bearings at Towers and Piers
- Longitudinally Fixed Bearing at US Tower
- Lateral Bearings at Towers

Hold-Downs

- Post Tension Hold Downs at Anchor and Side Span Piers

Lock Up Device (LUD)

- Longitudinal LUD At CAN Tower

Overall design: Tower foundations

Drilled Shafts:

- Diameter = 3.0 m (10 ft)
- Length > 30 m (98ft) (down to bedrock)
- Ultimate strength at rock socket level (per shaft):
  - Compression → 15,100 tonnes
  - Tension → 730 tonnes
- Loading/construction method verified by Osterberg cell load test
- One footing per tower leg
- Post-tensioned tie between footings

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Overall design: Towers

Inverted "Y" shape towers

- Conventionally reinforced concrete
- Hollow box section
- Steel anchor boxes for stay cable anchorage
- Corbels to support deck, no need for strut at deck level
- Uncoated reinforcing steel except stainless steel in the splash zone near deck level
- Completely accessible by ladders and elevators in all legs and upper part
- Transition room at legs' merging height

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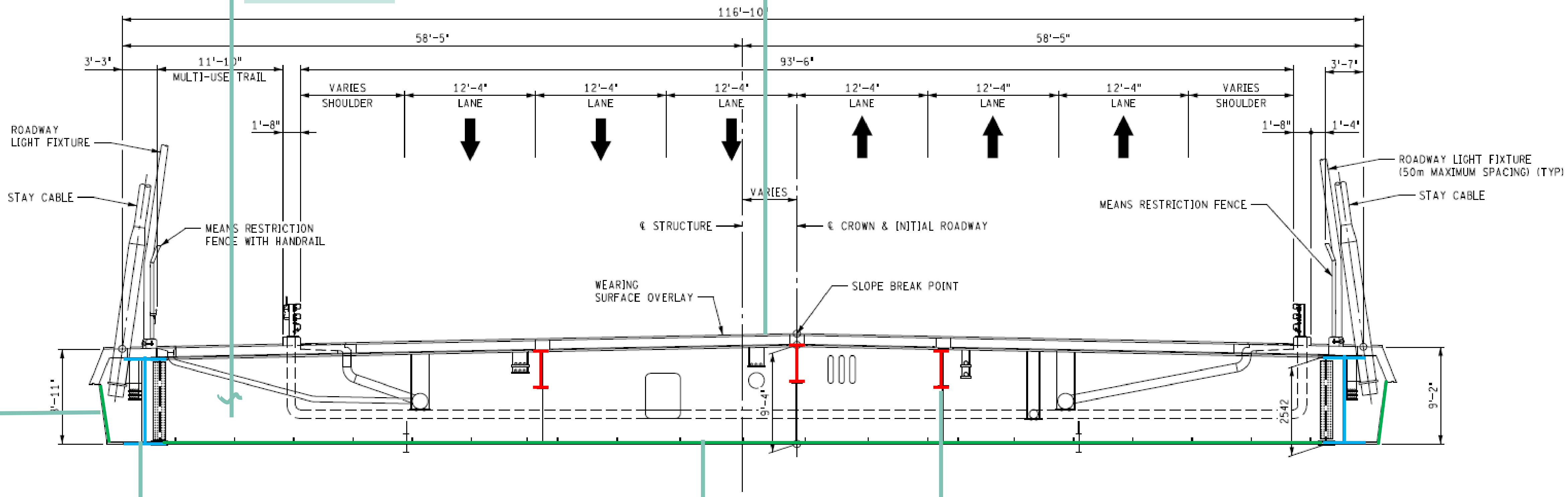
Overall design: Superstructure Cross Section (Initial traffic configuration)

- 37.50 m (11.43 ft) wide, asymmetric
- Multiuse trail + 2 x 3 lanes

Floorbeams

Concrete deck: constant depth 0.25 m (9.8 in)



Edge girders → depth 2.50 m (8.2 ft)

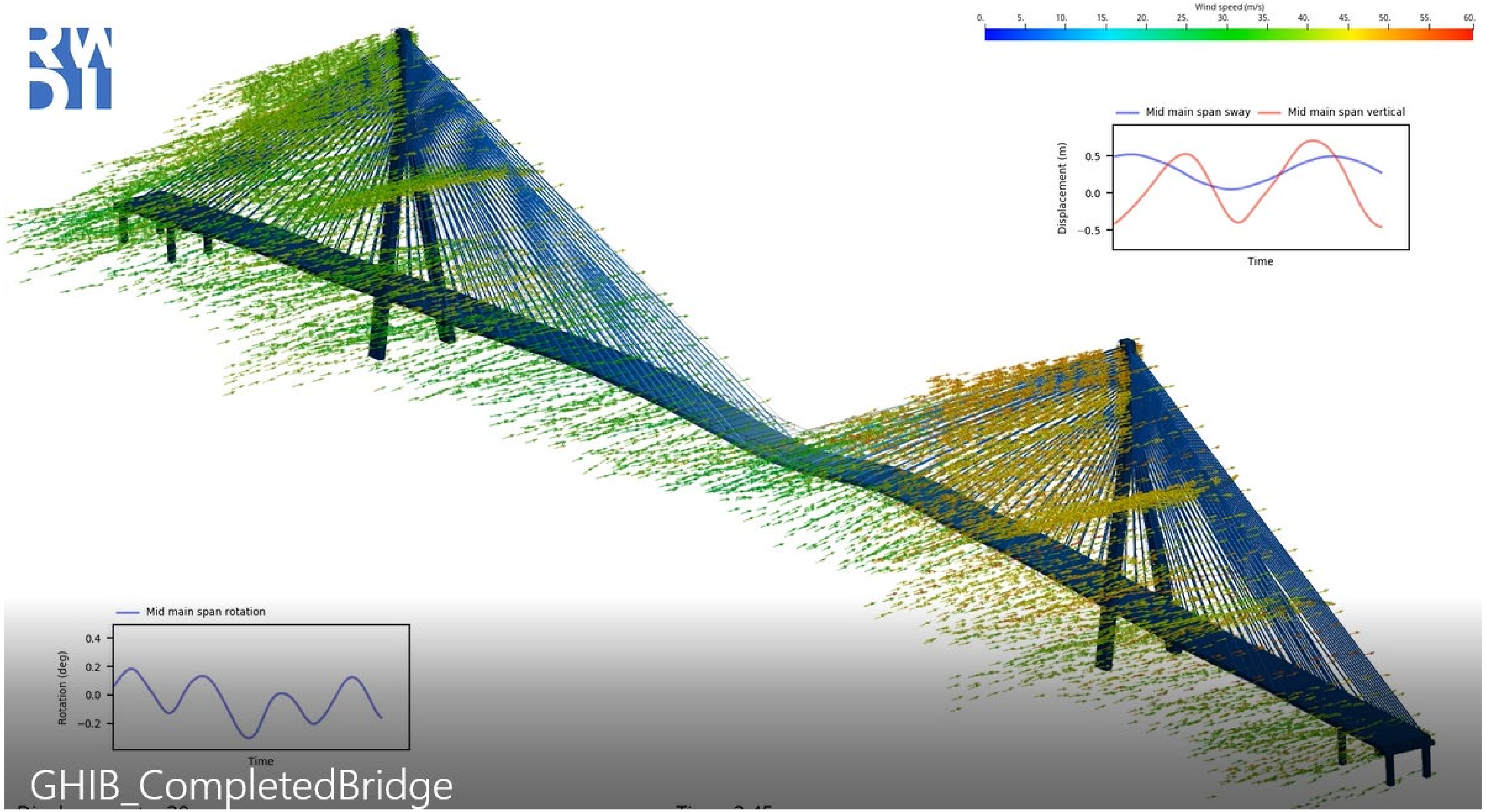
Redundancy girders → Structural

Cladding panels → Non structural: aerodynamics + aesthetics

Soffit panels → Non-structural: aerodynamics + aesthetics

Aerodynamic considerations

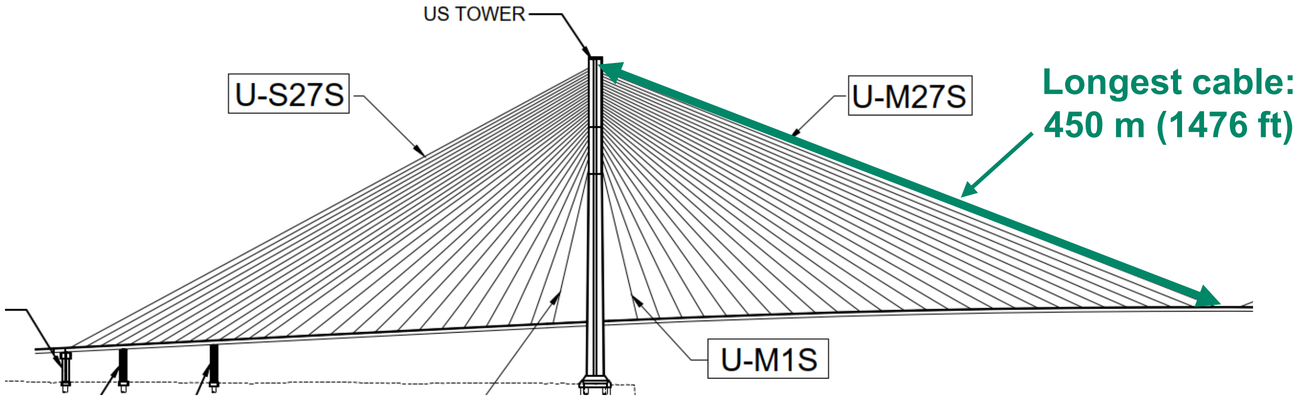
Wind buffeting analysis



Stay Cables

Parallel sheathed strands

- Parallel 0.6” diameter grade 270 post tensioning strand
- Sizing: from 38 to 121 strands per cable
- Greased and sheathed strand, encased in outer polyethylene sheath
- Design for passive and future active ice control measures



Durability

Service life: 125 years

- Excepting replaceable elements:
  - Cable stays → 100 years.
  - Bearings & expansion joints → 50 years
  - Others → 30 – 60 years
- Concrete durability based on fib Bulletin 34 "Model code for Service Life Design"
- STADIUM analysis software
- Maintenance painting → CAPP System® (Coating Assessment and Painting Priority)
- Comprehensive Durability Plan including assessment of all materials incorporated into the bridge

Key dates: Main Bridge

- Current focus is on Tower Upper Pylon Construction and Superstructure Erection
- Superstructure Main Span Erection Began 1st Quarter 2022
- Main Bridge Midspan Closure Summer 2024
- 30 Year Handover 2054

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Michigan Interchange

I-75 Improvements

4 Local Road Bridges

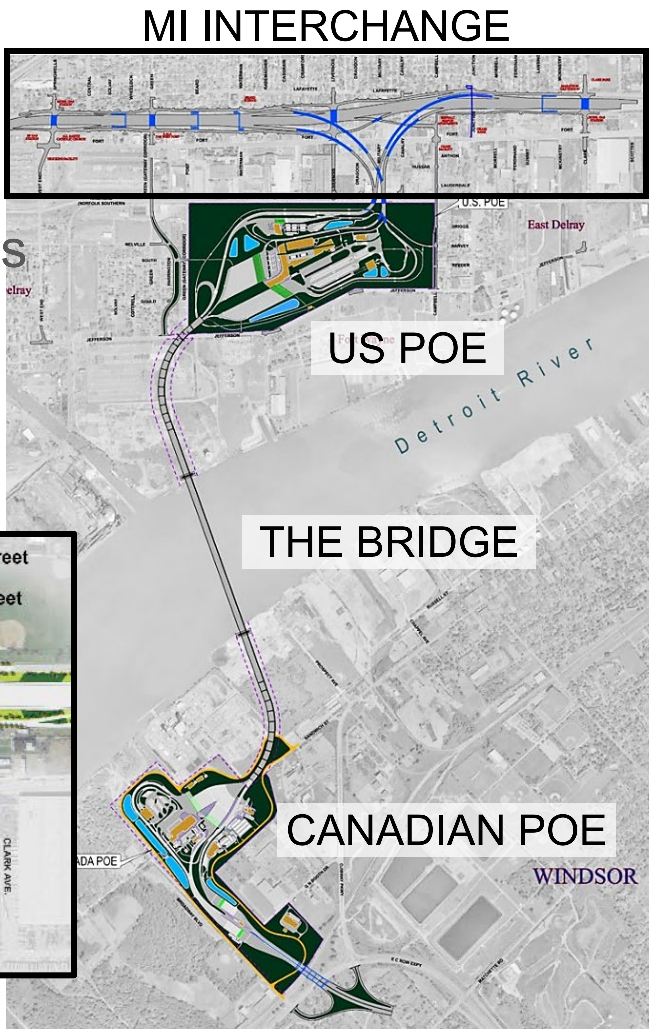
5 Pedestrian Bridges

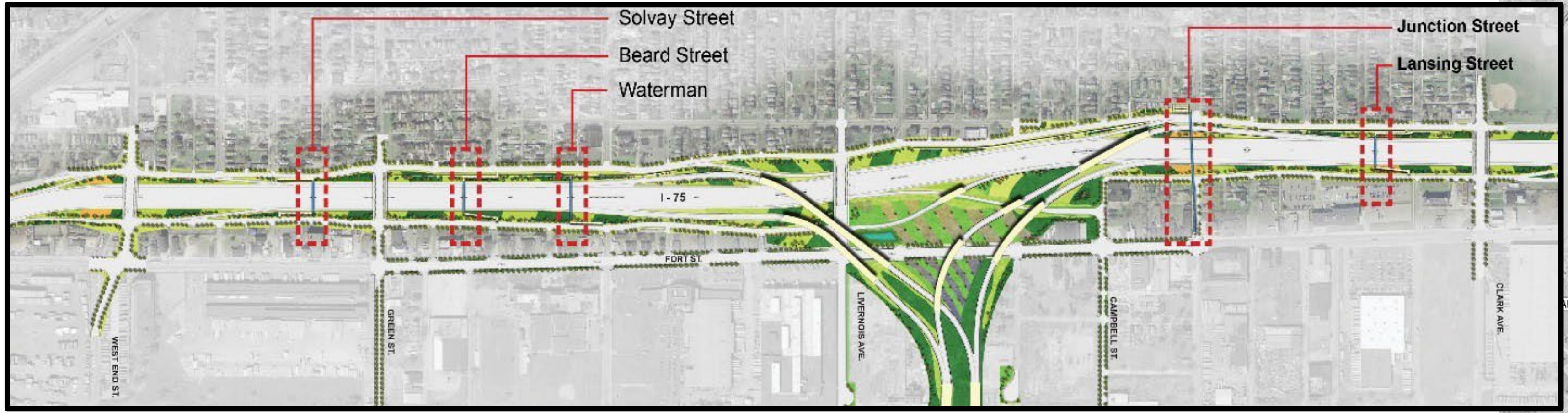
Noise Barrier Walls

Connecting Ramps

- 5 Steel Superstructure Flyover Bridges
- 5 Concrete Superstructure Ramp Bridges
- 8 Load Transfer Platforms
- 4 Gateway Towers –

  Transition from Steel to Concrete





MI Interchange – Pedestrian Bridges

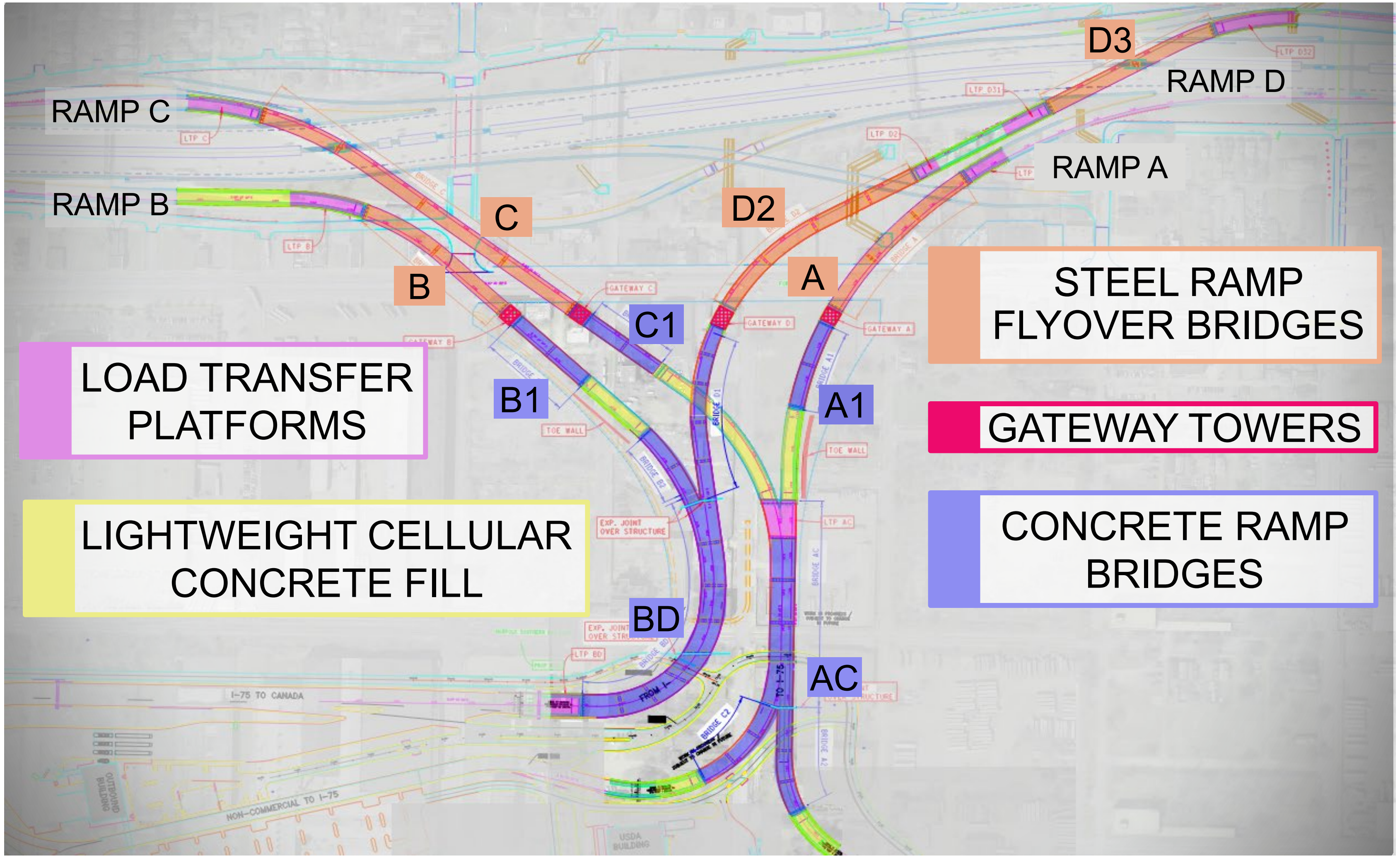
MI Interchange – Structures along I-75

Pedestrian & Local Bridges

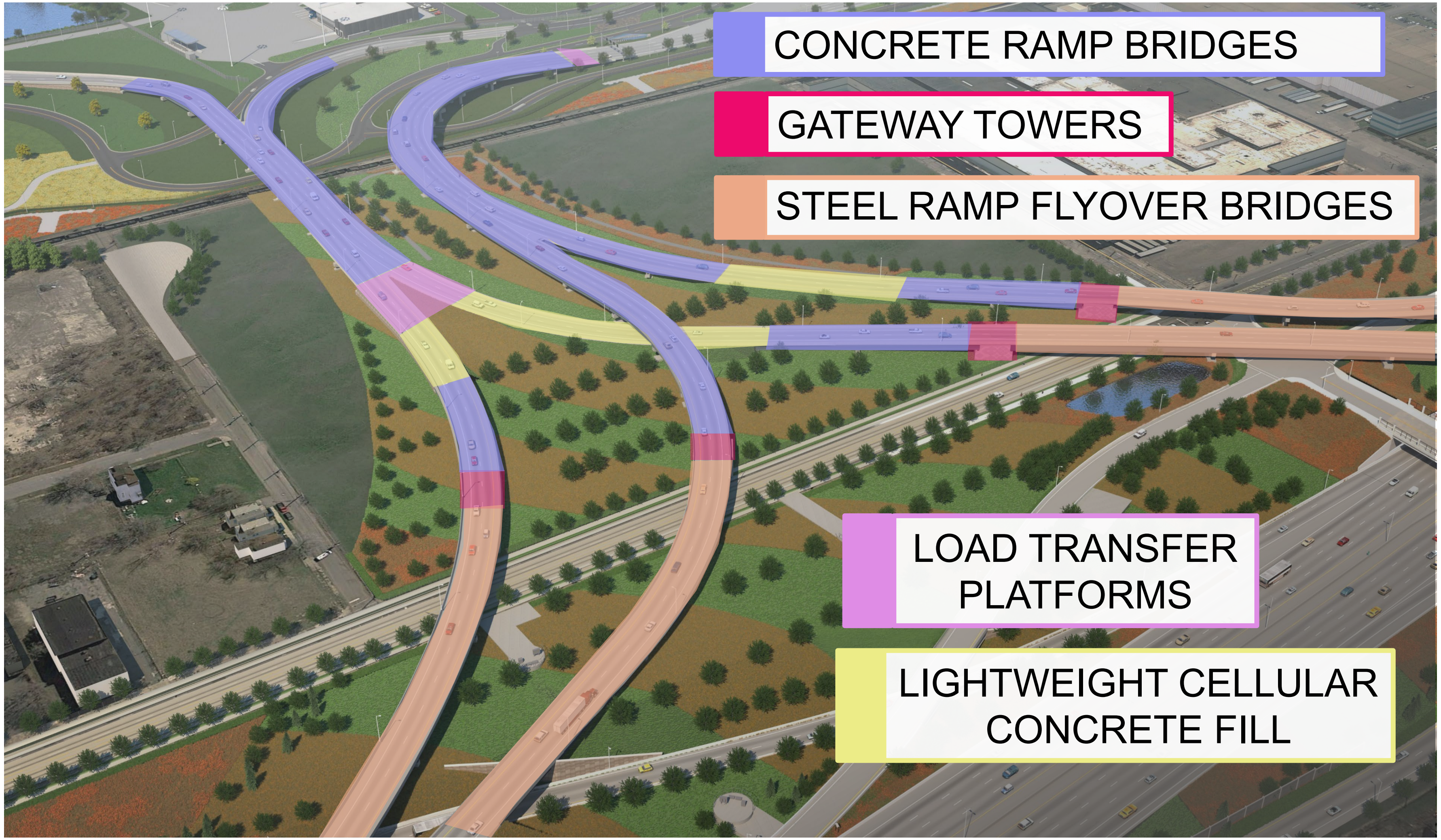
Arch was chosen option by the community

Aesthetic treatments for local road bridges

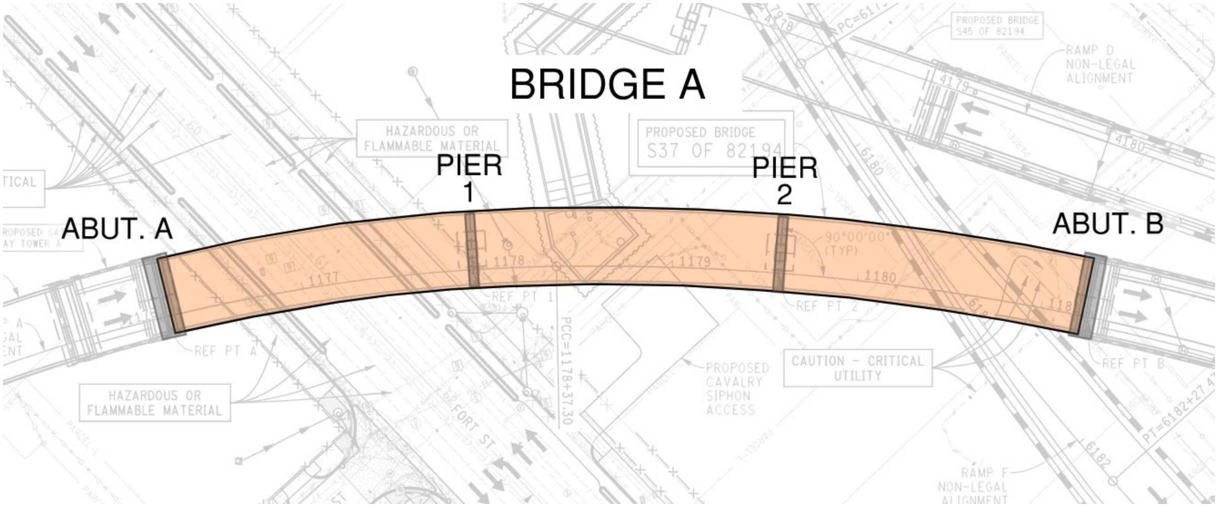
MI Interchange – Connecting Ramps

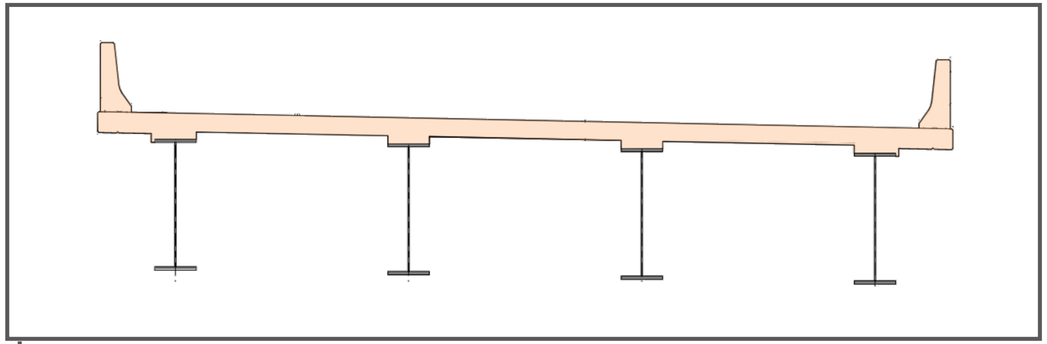


MI Interchange – Connecting Ramps



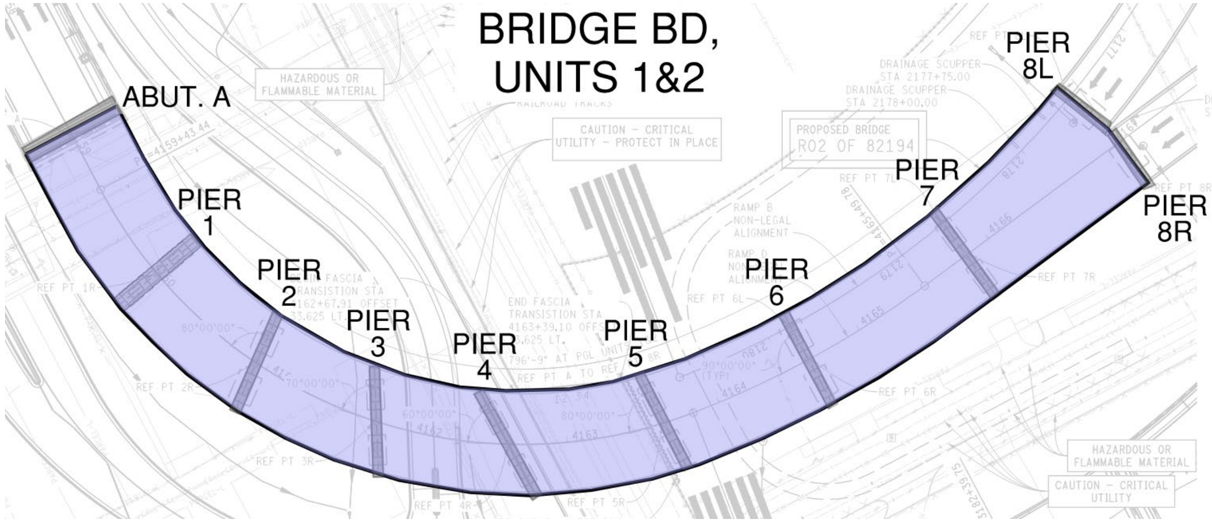
MI Interchange – Typical Flyover Bridges

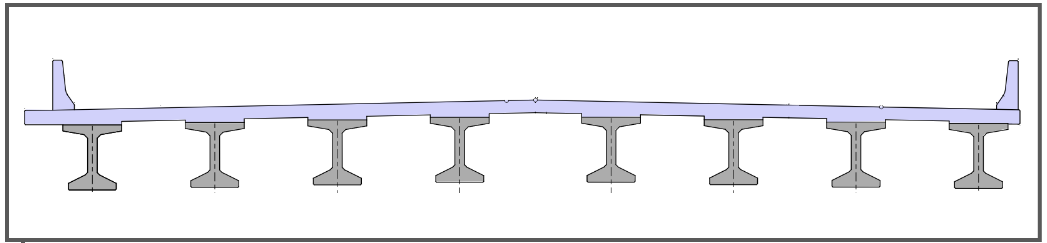




Four Curved Steel Plate Girders

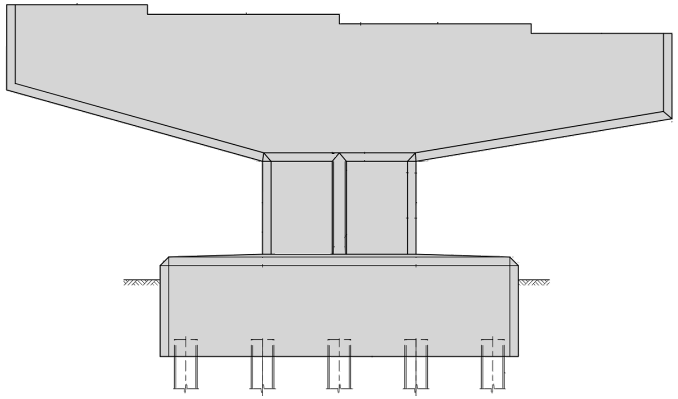
72” & 96” Webs

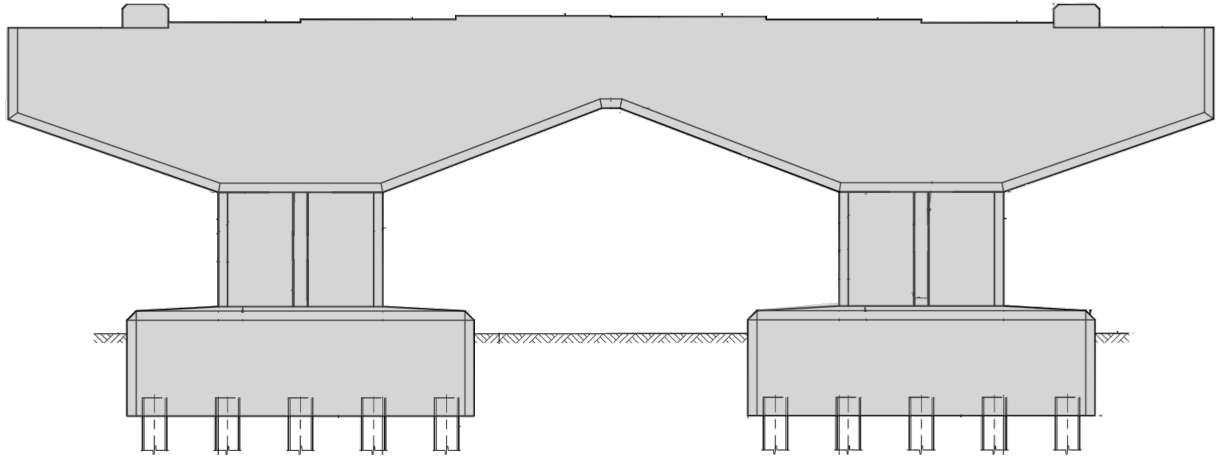


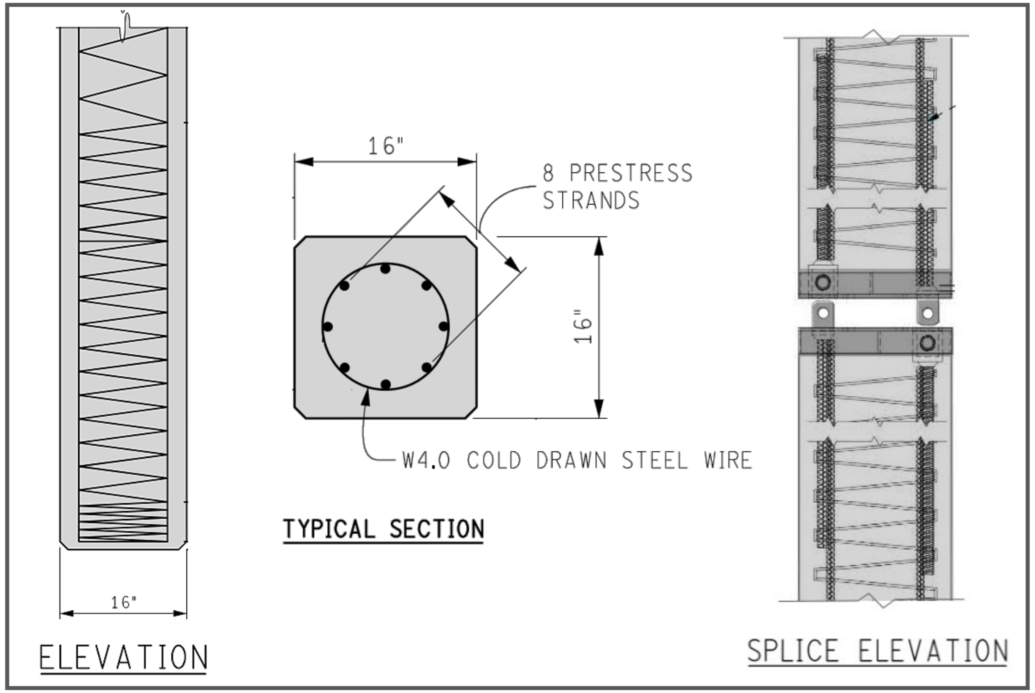


- Eight Prestressed Concrete Bulb-Tee Beams
- 54” Depth

MI Interchange – Typical Flyover Bridges

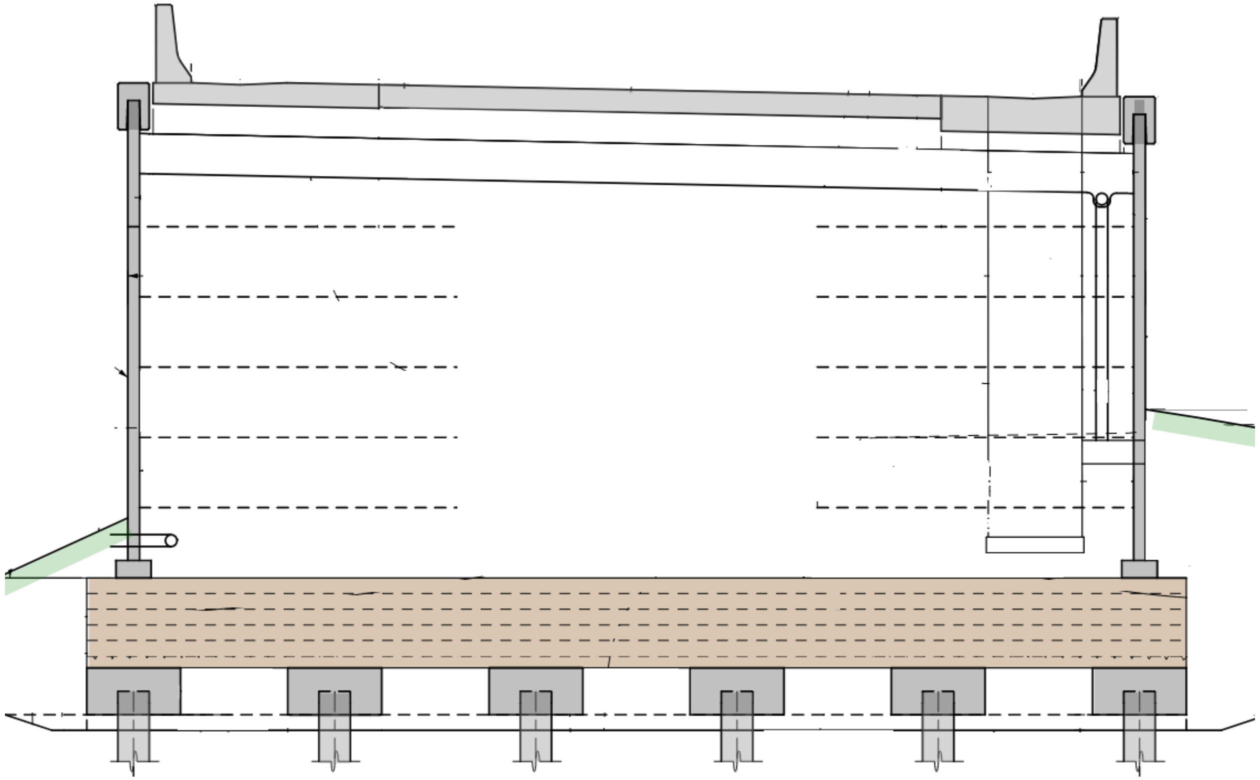






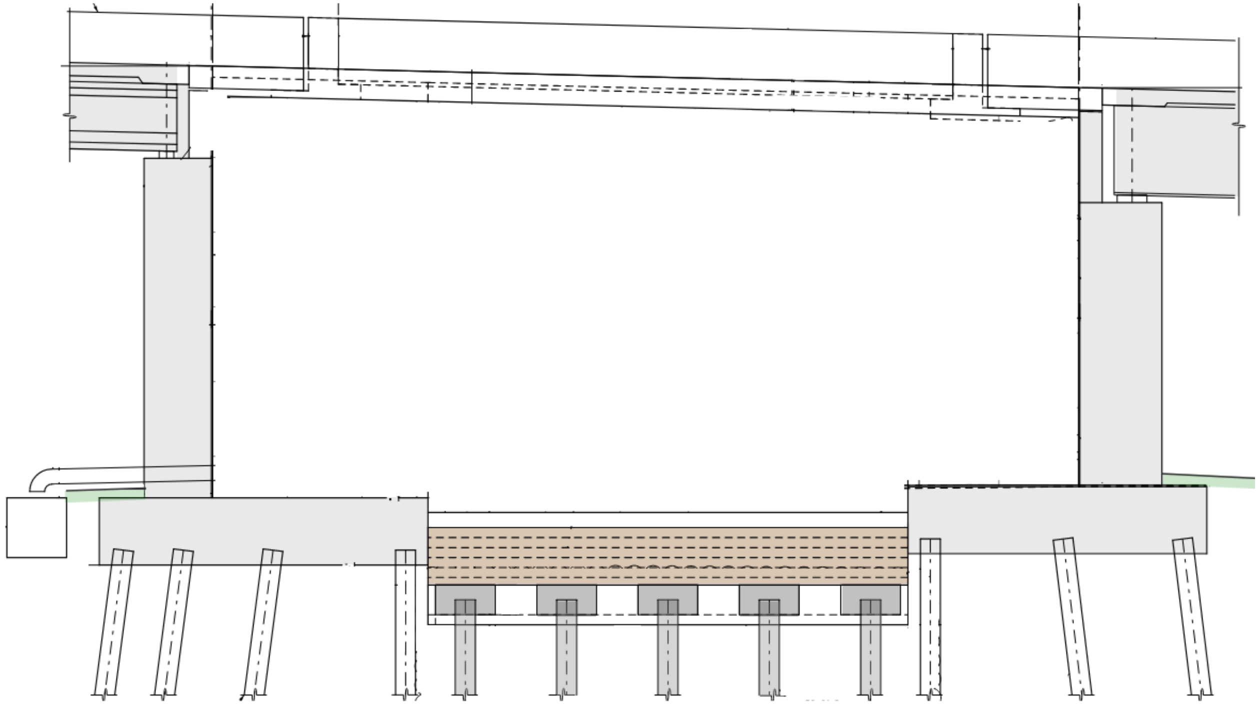
- Post-Tensioned Hammerhead Caps
- Single-Column Piers
- Prestressed Precast Concrete Piles

MI Interchange – Load Transfer Platforms (LTP)



- Piles @ 8’ centers, both ways
- Geosynthetic Grid – 8” lifts
- Dense-Graded Aggregate fill
- 4’ x 4’ Pile caps
- MSE walls supporting fill

MI Interchange – Gateway Towers



- Transition between different bridge types and varying superstructure heights
- Consists of LTP with MSE wall to support fill between high wall abutments

MI Interchange – Connecting Ramps

MI Interchange – Unique Features

- 125 years of design service life for the Connecting Ramp bridges
- AASHTO LRFD (HL-93 MOD) and WDBA specific live loads
- Minimize expansion joints
- Project specific reports:
    - Bridge Design Criteria report
    - Bridge Access report
    - Durability plan
    - **Redundancy report**
    - Erection Procedure report

MI Interchange – Redundancy Analysis

Design-Build Specifications

Load Path Redundancy

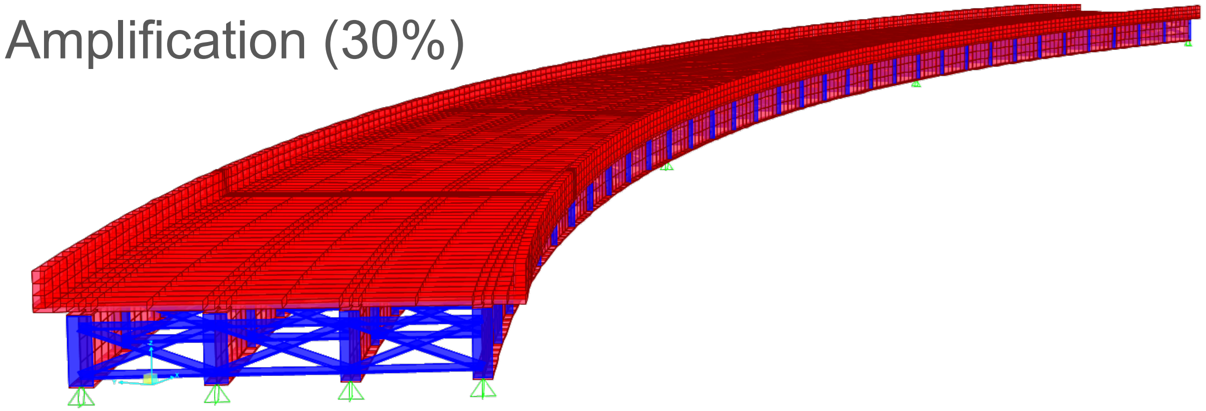
Positive & Negative Moment Fractures

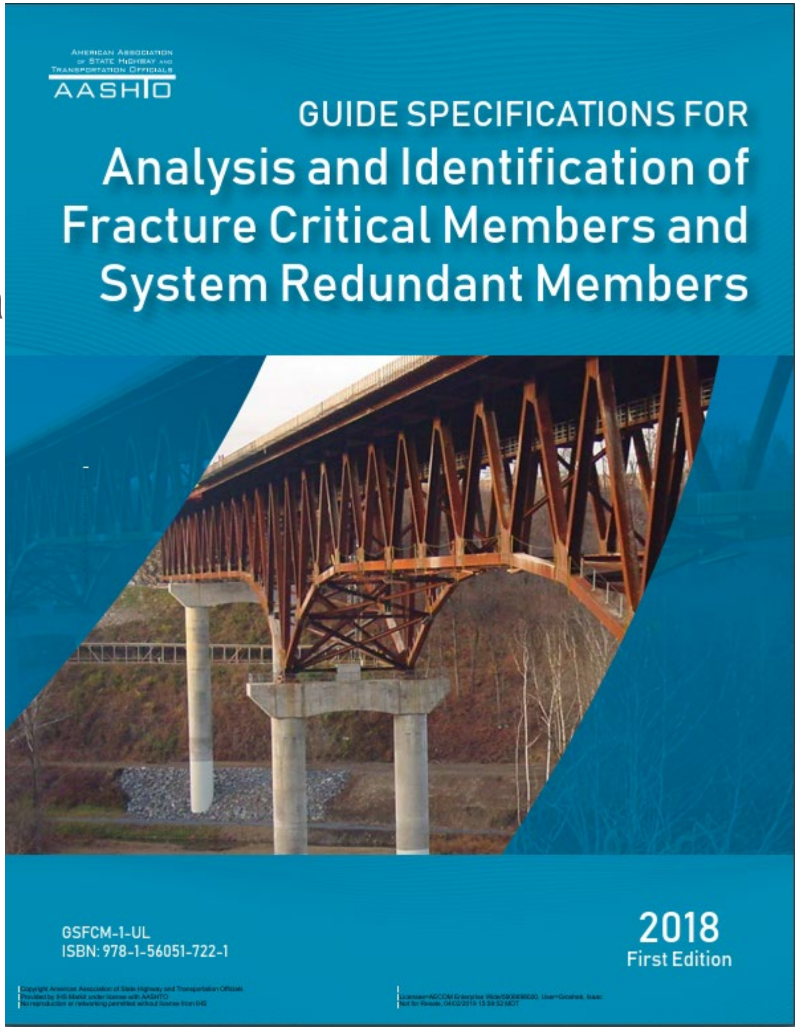
AASHTO Guide Specifications for Analysis and Identifica of Fracture Critical Members and System Redundant Members

Nonlinear Analysis Guidance

Strain-based Failure Criteria

Dynamic Amplification (30%)



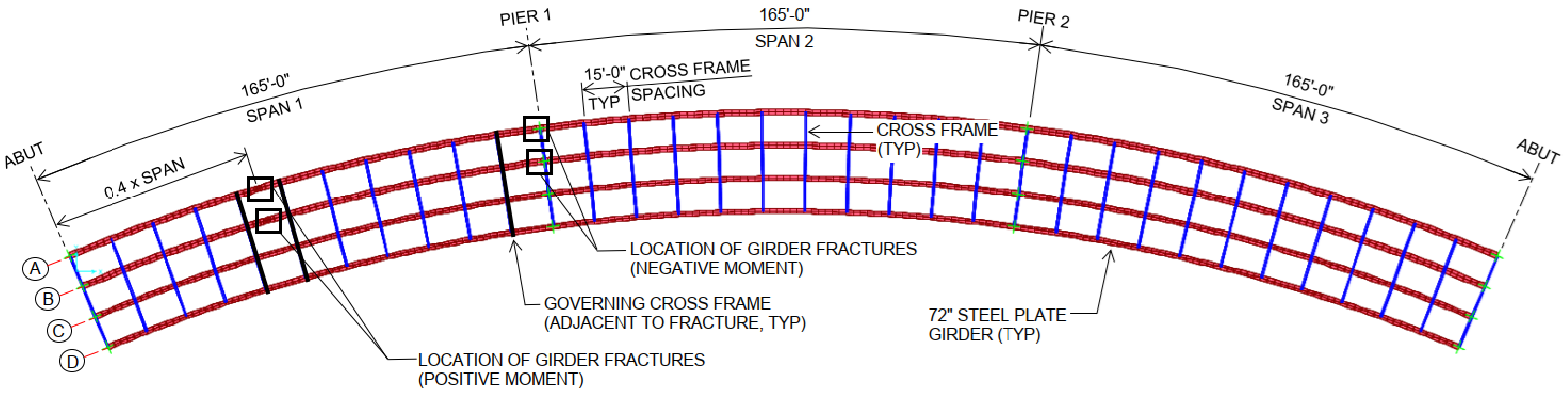


MI Interchange – Redundancy Analysis Fracture Locations

Positive Moment Fractures

Negative Moment Fractures

Governing Cross Frames

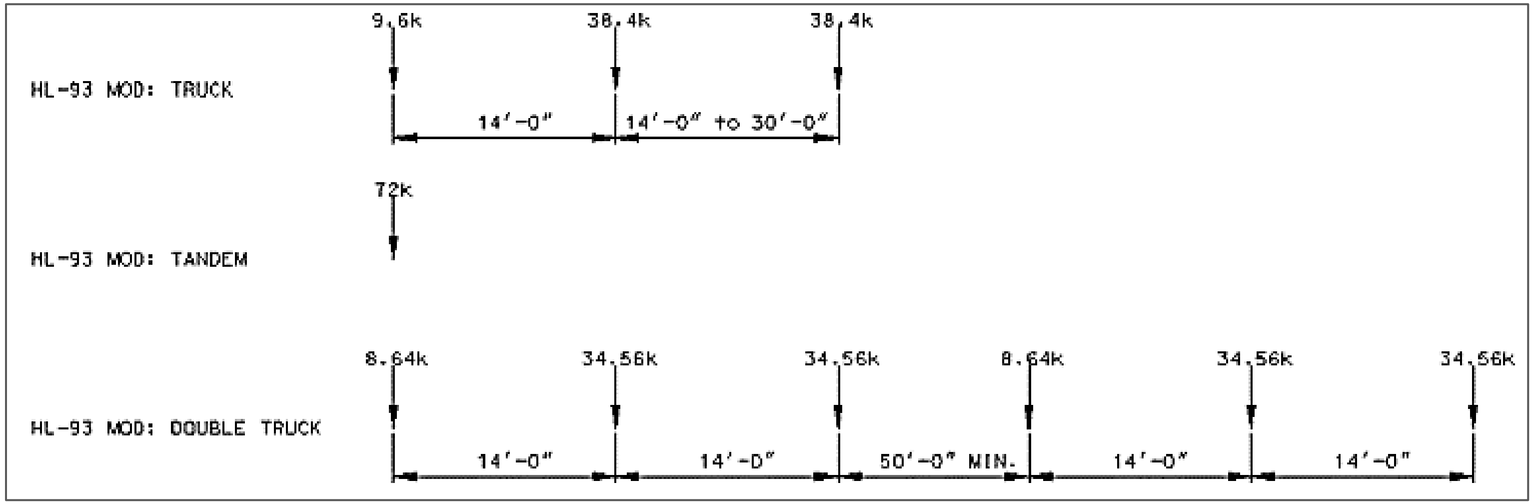


BRIDGE D1 - FRACTURE LOCATIONS

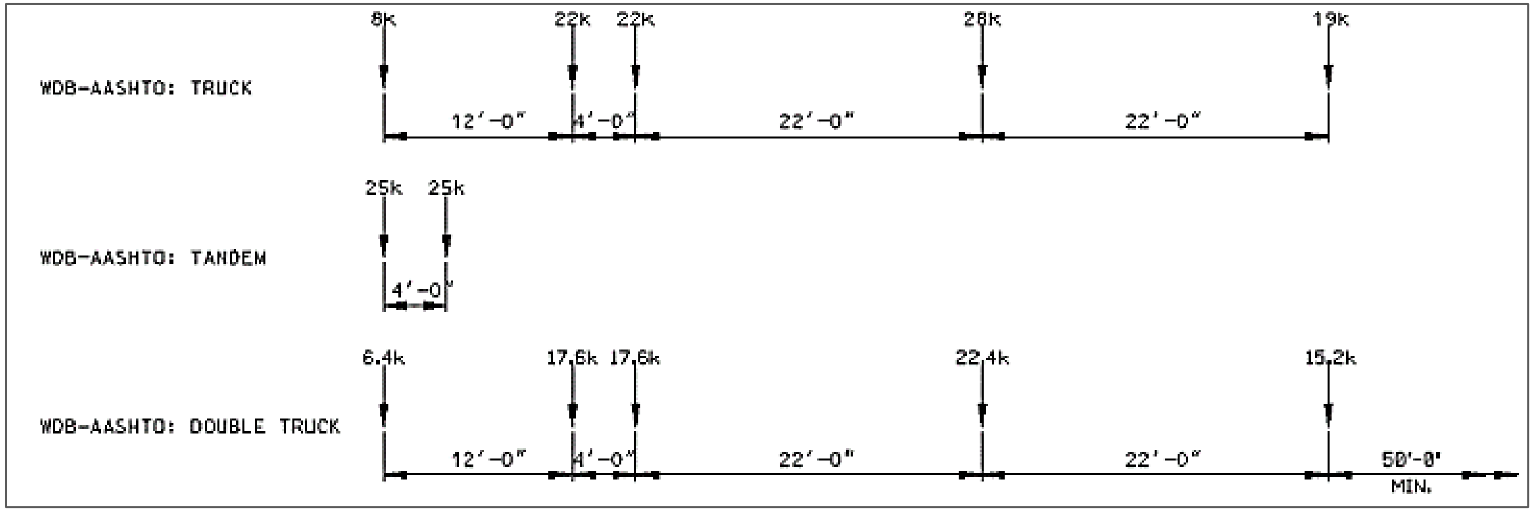
MI Interchange – Redundancy Analysis Vehicular Loading

HL-93 (MOD) Vehicular Loading

- 20% greater loading than standard HL-93 Loading



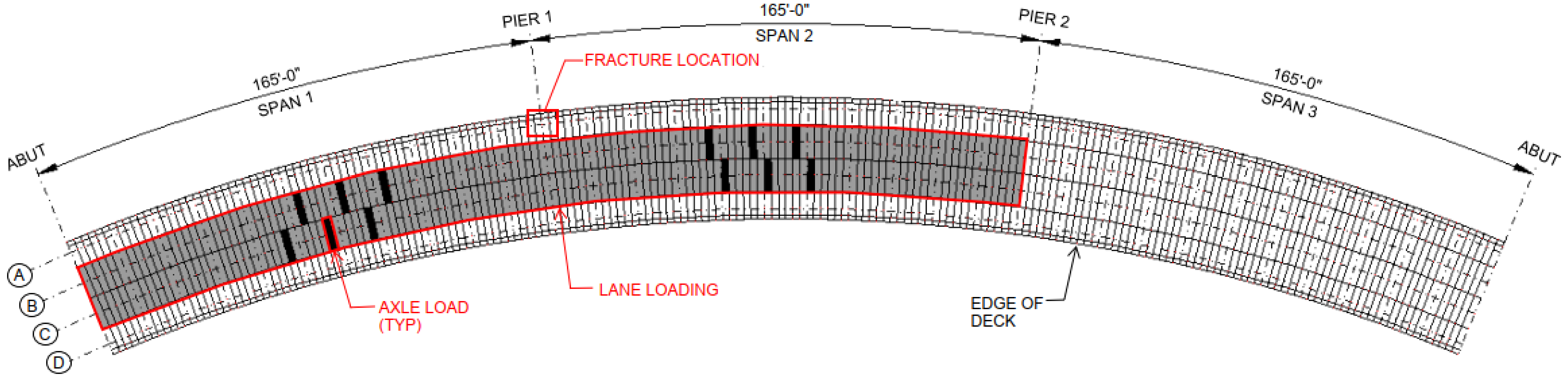
WDBA Vehicular Loading



MI Interchange – Redundancy Analysis Loading Application

Specific Loading Configurations for each fracture case determined by linear analysis.

Area loads used to apply truck and lane loads.



BRIDGE D1 - LIVE LOAD PATTERN - EXTERIOR GIRDER FRACTURE

MI Interchange – Redundancy Analysis Redundancy Load Combination

Importance Factor, ηI = 1.05

- Required by D-B Specification
- Applies to all loading types

Dynamic Amplification Factor, DAR = 0.30

- Accounts for bridge oscillation after fracture
- Based upon research at University of Austin, TX

Static Force Amplification

- DC Loading = 1.05 x 1.30 x 1.25 = 1.71
- DW Loading = 1.05 x 1.30 x 1.50 = 2.05
- LL + IM Loading = 1.05 x 1.30 x 1.30 x 1.33 = 2.36!!!

Redundancy Load Combination:

$$\eta_I \times (1+DA_R) \times [1.25DC + 1.5DW + 1.30(LL+IM)], \quad IM = 33\%$$

Reminder: HL-93 (Mod) is 20% heavier than HL-93 Loading

- 1.2 x 2.36 = 2.83

MI Interchange – Redundancy Analysis Challenges & Solutions

- Challenges:
  - Deck Crushing Failure – Positive Moment Fracture
  - Unacceptable Tensile Strains in Flanges
  - Unacceptable Strains in K-Style Cross Frames

- Solutions:
  - Confinement Reinforcement for Deck & Haunches
  - Ensure Flange Size for Increased Tensile Strains
  - X-Frame Style Cross Frames

Acknowledgements

Windsor-Detroit Bridge Authority

- Responsible for delivery of Gordie Howe International Bridge Project

Bridging North America

- Private sector partner responsible for design, build, finance, operate and maintain the facility
    - ACS Infrastructure
    - DRAGADOS
    - Fluor
    - AECON

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- RS Engineering — RS Engineering, LLC
- NTH Consultants — NTH
- HNTB (IDC) — HNTB


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