Lateral Spreading Design Solutions

Seismic Design & Retrofit

Western Bridge Engineer's Seminar

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Lateral Spreading (Quick Recap)

Seismically induced Soil Liquefaction causing excessive horizontal ground displacement





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Far From Home, UCSD/UCI, August 2011

California Department of Transportation (Caltrans) provides Loading Guidance:



Figure A3 Two possible design cases for the calculation of the ultimate passive load due to the soil crust

MTD 20-15 Attachment 1, Caltrans, May 2017



Figure 5 Interaction Curve (1) shows the total resisting force (R_{Tot}) vs. the foundation displacement. Curve (2) uses a running average of the resisting force to correct Curve (1). Curve (3) represents the displacement response of the sliding mass as calculated in Step 6. The design displacement demand is determined by the intersection of curves (2) and (3).

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Application of Lateral Spreading

Existing Bridge Assessment and Seismic Retrofits

- Caltrans MTD 20-4: Seismic Retrofit Guidelines for Bridges in California
- Assess & Design for No-Collapse
- Allows application of Ground Shaking, Liquefaction and Lateral Spreading separately based upon reduced remaining service life of existing bridge

New Bridge Design

- Must meet ductility requirements of Caltrans Seismic Design Criteria (SDC)
- Must combine Lateral Spreading with other applicable seismic hazards such as Ground Shaking and Liquefaction

Design Options for Lateral Spreading

Preventing lateral spreading movement requires extensive resistance

- Ground Improvements
- Extensive Foundation Systems



Allowing deformation of bridge system reduces restraining force

- Utilize passive pressure against opposite abutment
- Utilize column resistance
- Reduce restraining force



Case Study: Eureka Hill Road Bridge over Garcia River Seismic Retrofit, Mendocino County, CA



- Lateral Spreading was predicted at Piers and Abutments:
- 10 inches at Abutment 1 and Pier 2
- 24 inches at Pier 3
- Unlimited movement at Pier 4 and Abutment 5



Figure 8 Lateral spreading of existing piles showing evolution from plastic hinges to pins

Table 1 Allowable Drift Capacity of Existing Piles and Shafts for Lateral Spreading

Element	Drift Ratio	Comments
Step Tapered in Steel Shell Piles	0.05	Larger capacity allowed through analysis
CIDH Piles with reinforcement only in top 10 ft of pile	0.05	Larger capacity allowed through analysis
Timber Piles	0.20	-
Driven Precast Piles	0.20	
Steel Pipe Piles	0.20	Larger capacity allowed through analysis
Steel H Piles	0.20	Larger capacity allowed through analysis



- Drift is 10" over 7 ft
- Drift Ratio = 12%
- Allowable Drift = 20%
- Existing piles able to accommodate expected displacement based upon Drift Capacity in MTD 20-15.
- No Retrofit Necessary

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Eureka Hill – Piers 2, 3 & 4



Checked for Moment and Shear Demand:

- Retrofitted Column
- Pile Cap
- Piles



- Drift is 60" over 4 ft
- Drift Ratio = 125%
- Allowable Drift = 20%
- Existing piles will exceed allowable drive ratio based upon Drift Capacity in MTD 20-15.



Initial retrofit strategy:

Add Large Diameter CIDH Piles at Abutment 5 to resist lateral spreading load

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Use resistance from entire structure to resist lateral sliding forces

- Contributions from:
 - Abutment 1 passive resistance
 - Piers 2, 3 & 4 flexural resistance
 - Abutment 5 piles



- Revised retrofit strategy:
 - Use resistance from entire structure to resist lateral sliding forces
 - Abutment 1 passive resistance
 - Piers 2, 3 & 4 flexural resistance
 - Abutment 5 piles
- Eliminates the need for large CIDH piles at Abutment 5
- \$500,000 Cost Savings

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Case Study: Washington Street Bridge over Petaluma River Seismic Retrofit, Petaluma, CA



• 2 inches of Lateral Spreading was predicted at both Abutments

Washington Street Bridge Original Retrofit



- Large Diameter CIDH Piles at Abutments
- \$1 M Construction Cost

Washington Street Bridge Structure Resistance



- Transfer load through abutments and superstructure to opposite embankment
- Primary resistance from opposite abutment
- \$1 M Construction Cost Savings

Washington Street Abutments



Check Abutment Capacity:

 Transfer loading to single pile foundation & to fixed connection with superstructure

Case Study: Aldercroft Heights Road Bridge over Hooker Creek & Los Gatos Creek, Santa Clara County, CA



• 20" of Lateral Spreading was predicted at Abutment 4

Aldercroft Heights Road – Total Structure Resistance



- Resist with total structure
 - Abutment 1 Passive Pressure
 - Pier 2 & 3 Flexural Resistance
 - Abutment 4 Pile Resistance

Aldercroft Heights Road – Abutment 4



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- New Structure must meet SDC ductility limits
- Total structure resistance reduces deflection of Abutment 4 piles from 8" to 4"

Aldercroft Heights Road – Abutment 4



Check Abutment Capacity:

- Transfer Loading to Pile Cap
- Transfer Loading to Backwall

Aldercroft Heights Road – Piers 2 & 3



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- New Structure must meet SDC displacement ductility limits
- Add Ground Motion longitudinal displacement to Lateral Spreading displacement

Aldercroft Heights Road – Abutment 2



• Lateral Spreading was predicted at Abutment 2

Aldercroft Heights Road – Abutment 2



- Use Grade Control Sill Beams in Creek & Superstructure to transfer loading to Abutment 1 and resist with Passive Pressure
- Piles only required to resist vertical loading

Summary

Designer should account for total resistance of bridge structure to resist lateral spreading

- Include column resistance
- Include passive pressure from opposite abutment

Existing Structures

- Allow piles to hinge
- Check displaced piles for their ability to continue to carry vertical loading

New Structures

- Increase foundation flexibility to allow deflection
- Reduces lateral spreading force demands
- Allows other components to help resist lateral spreading demands

Questions?

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