

Western Bridge Engineers' Seminar 2023

# Design and Construction of SR520 West Approach Bridge South

Presented by:

**Kevin Almer, PhD PE SE (TYLI) &**

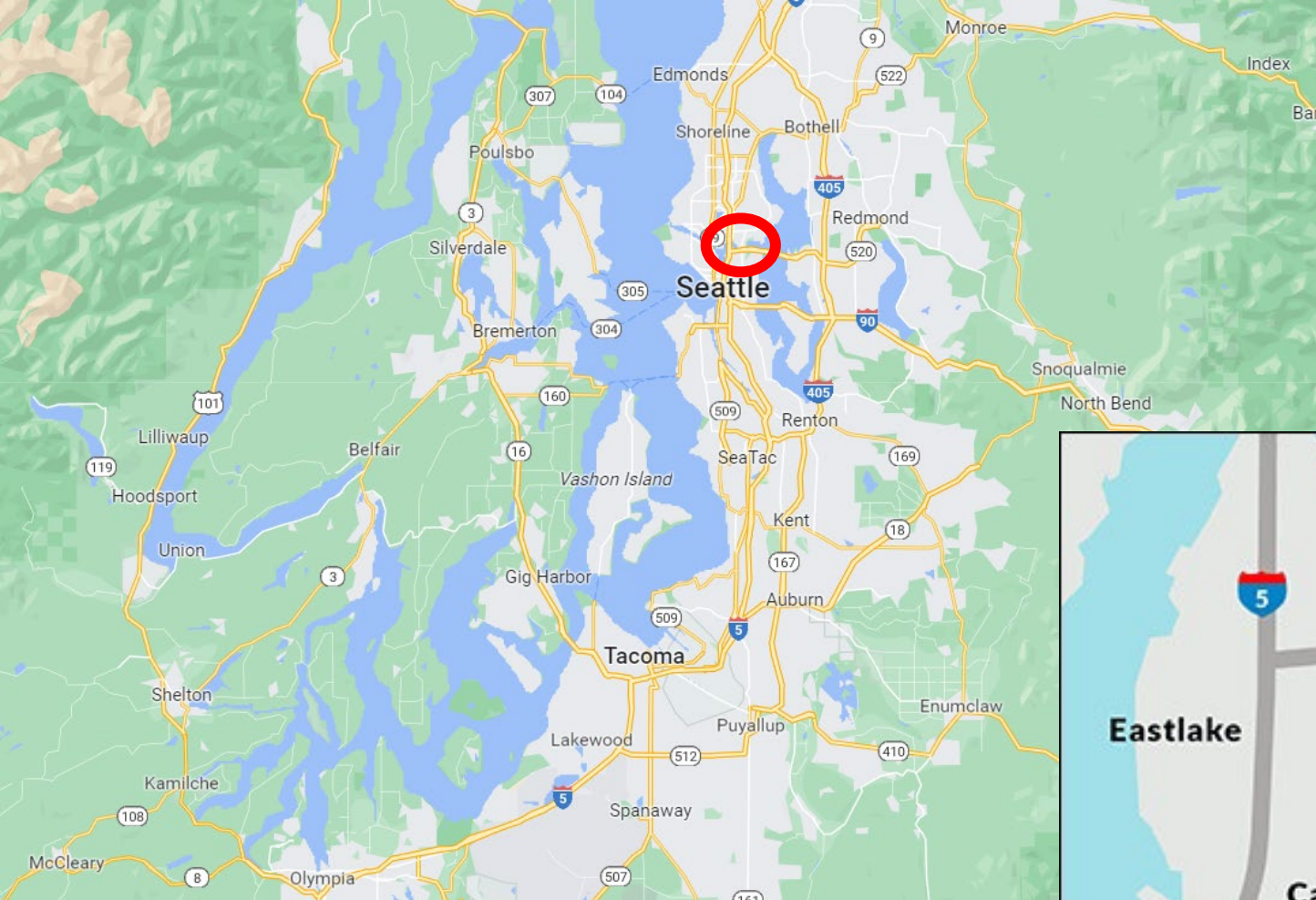
**Norman Smit, PE SE (TYLI)**



**TYLin**



# Project Location Map





# Agenda

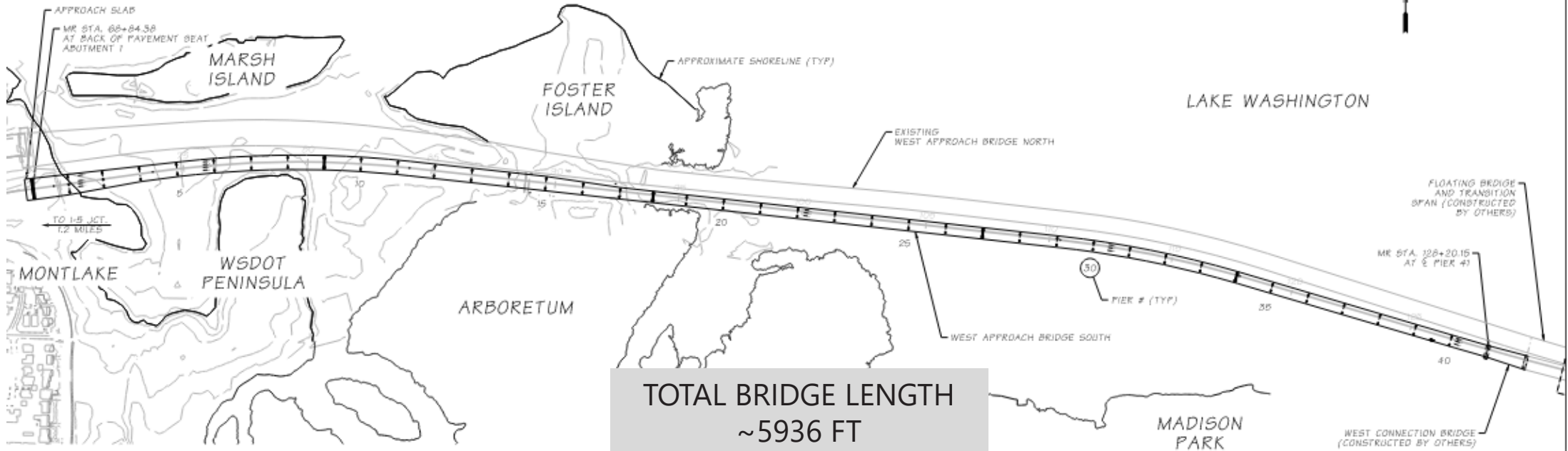
- DESIGN CHALLENGES
- SEISMIC DESIGN
- FUTURE COMPATIBILITY
- CONSTRUCTION STAGING
- CONSTRUCTION CHALLENGES



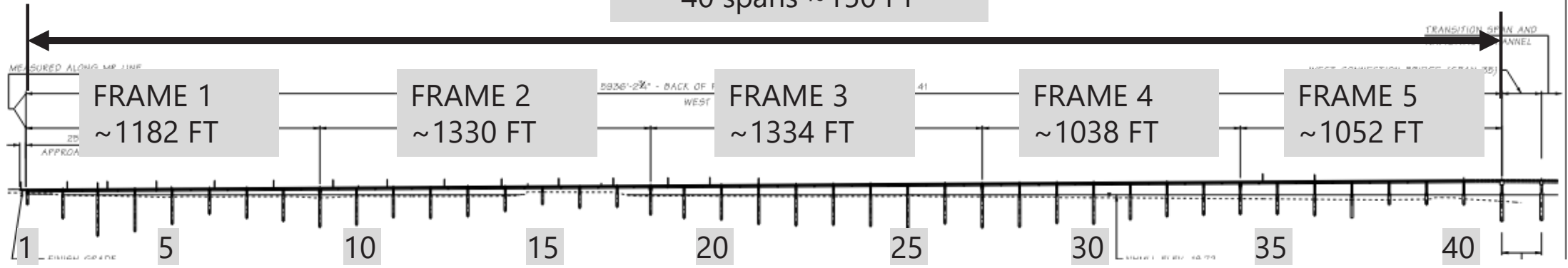
# Structure Layout

SEC. 21-22, T.25N., R.4E., W.M.  
CITY OF SEATTLE

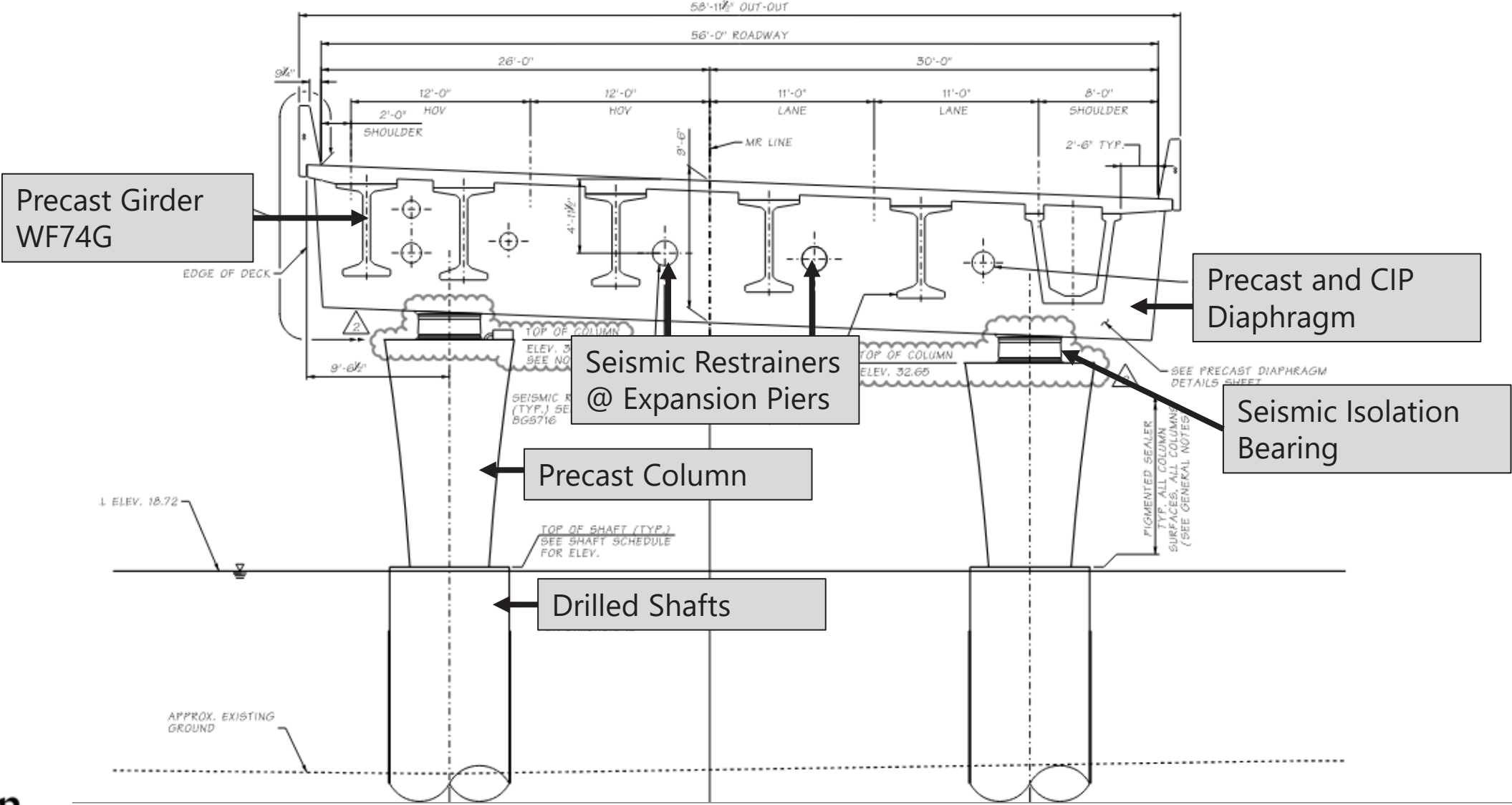
SR 520



**TOTAL BRIDGE LENGTH**  
~5936 FT  
40 spans ~150 FT



# Typical Cross-Section



Precast Girder  
WF74G

Precast and CIP  
Diaphragm

Seismic Restrainers  
@ Expansion Piers

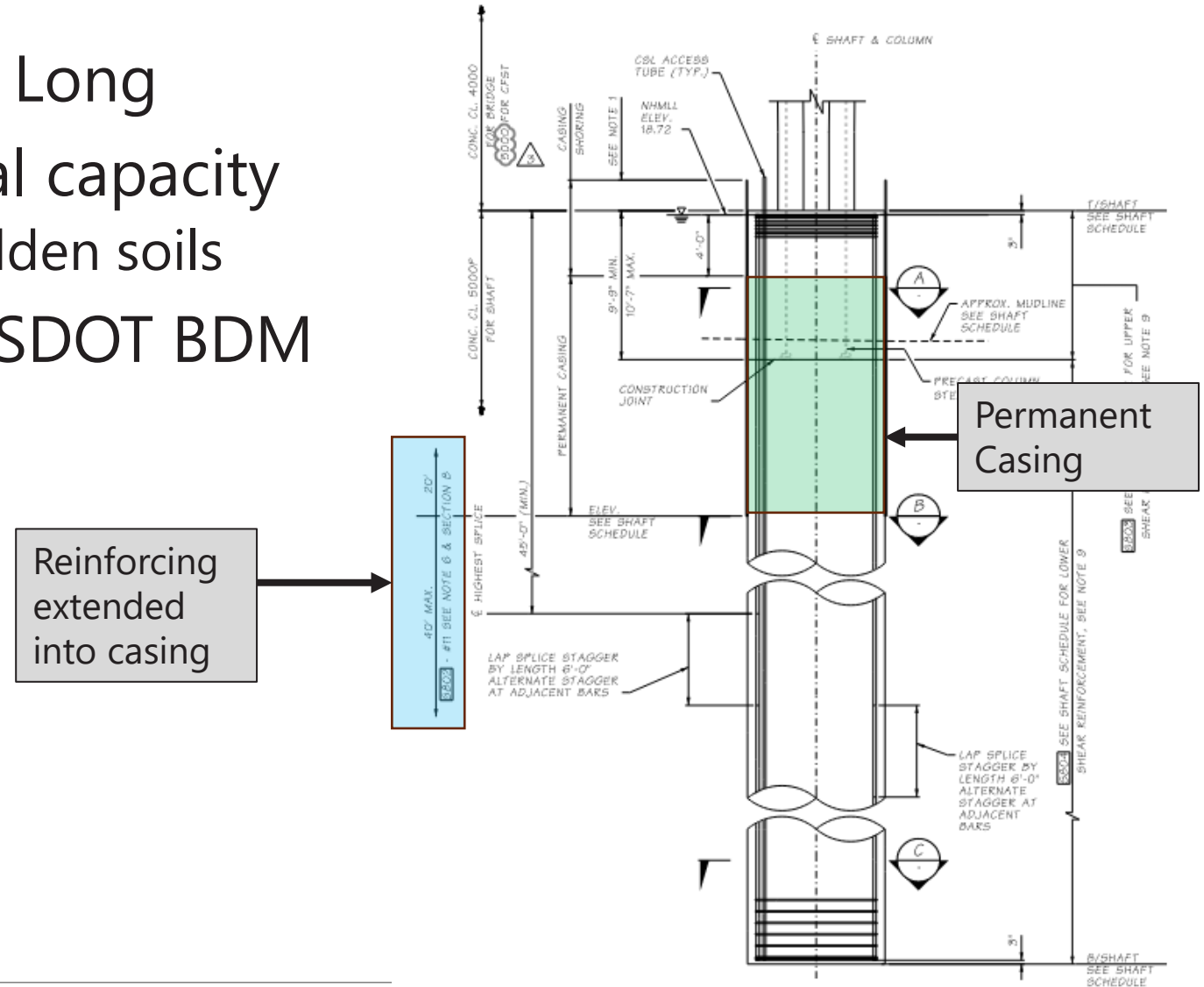
Seismic Isolation  
Bearing

Precast Column

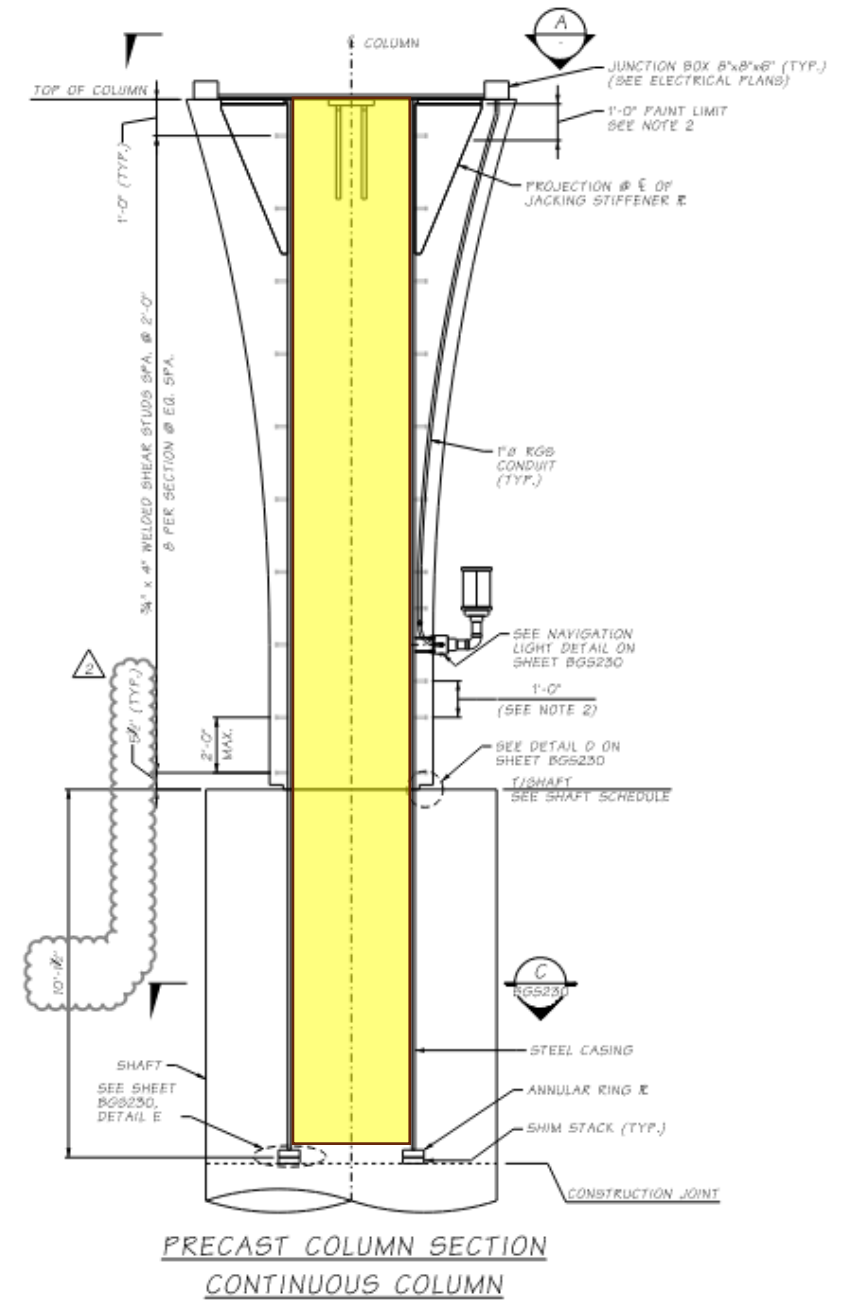
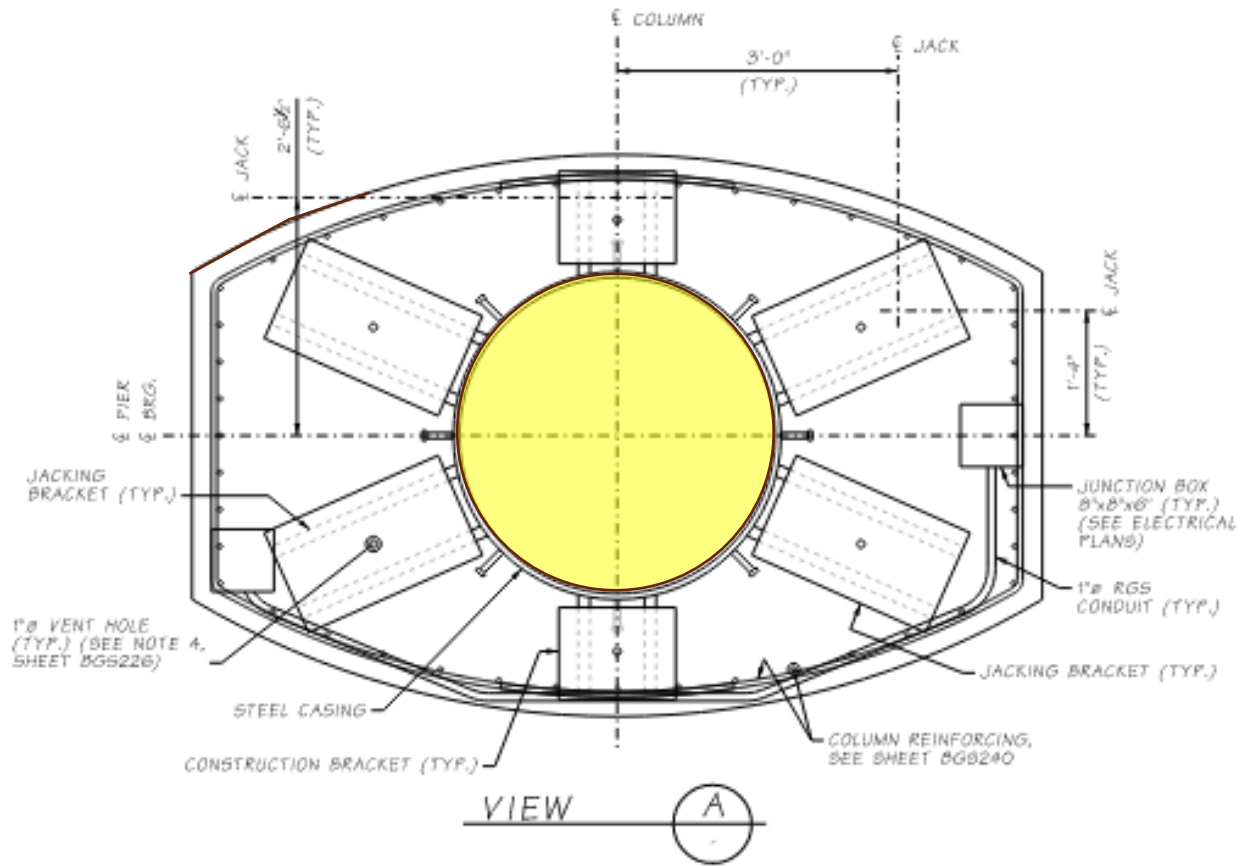
Drilled Shafts

# Design Challenges – Drilled Shafts

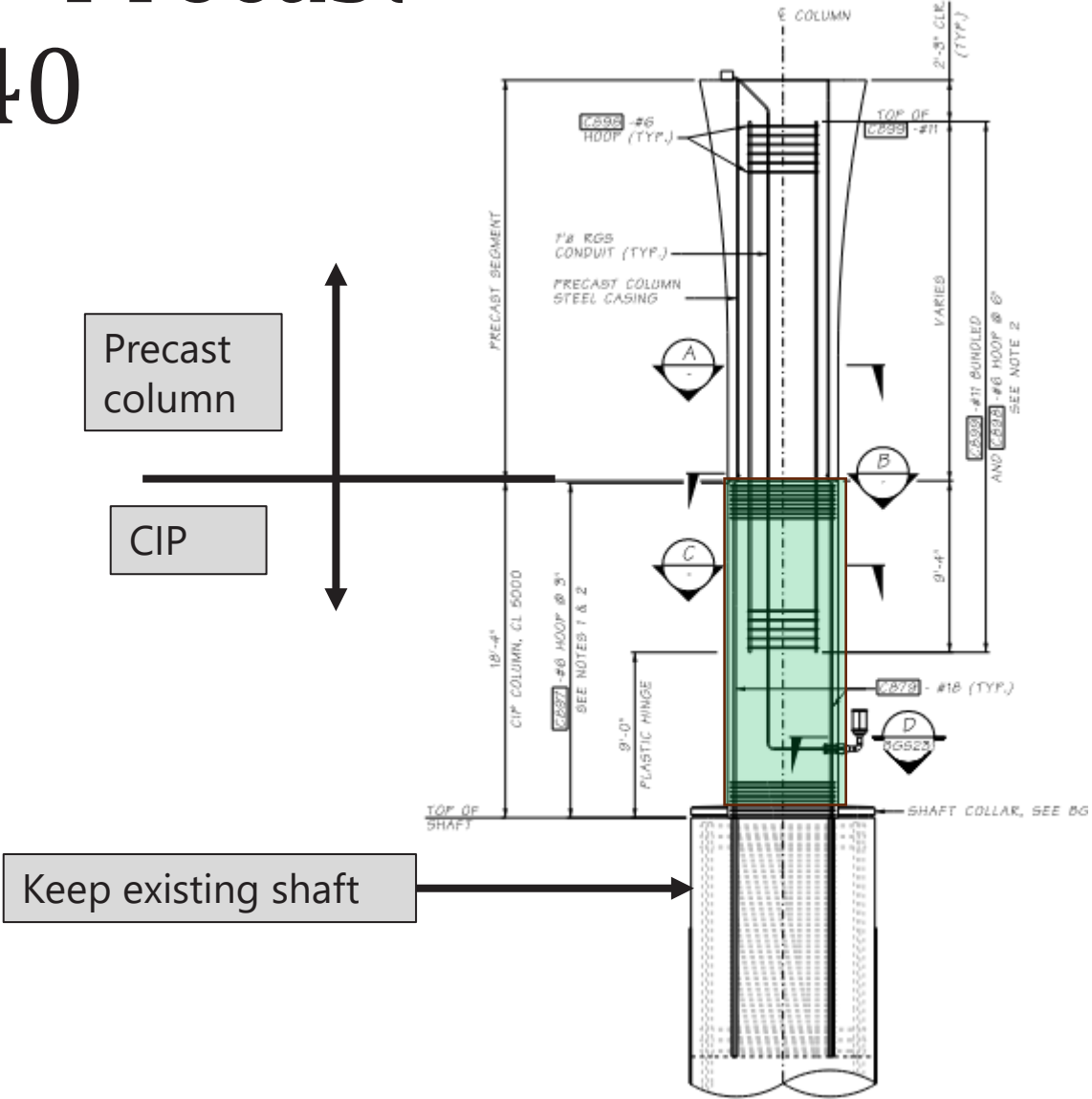
- Shaft Lengths up to 144' Long
- Casing used for structural capacity
  - Driven to glacially overridden soils
- Designed as CFST per WSDOT BDM



# Design Challenges – Precast Columns



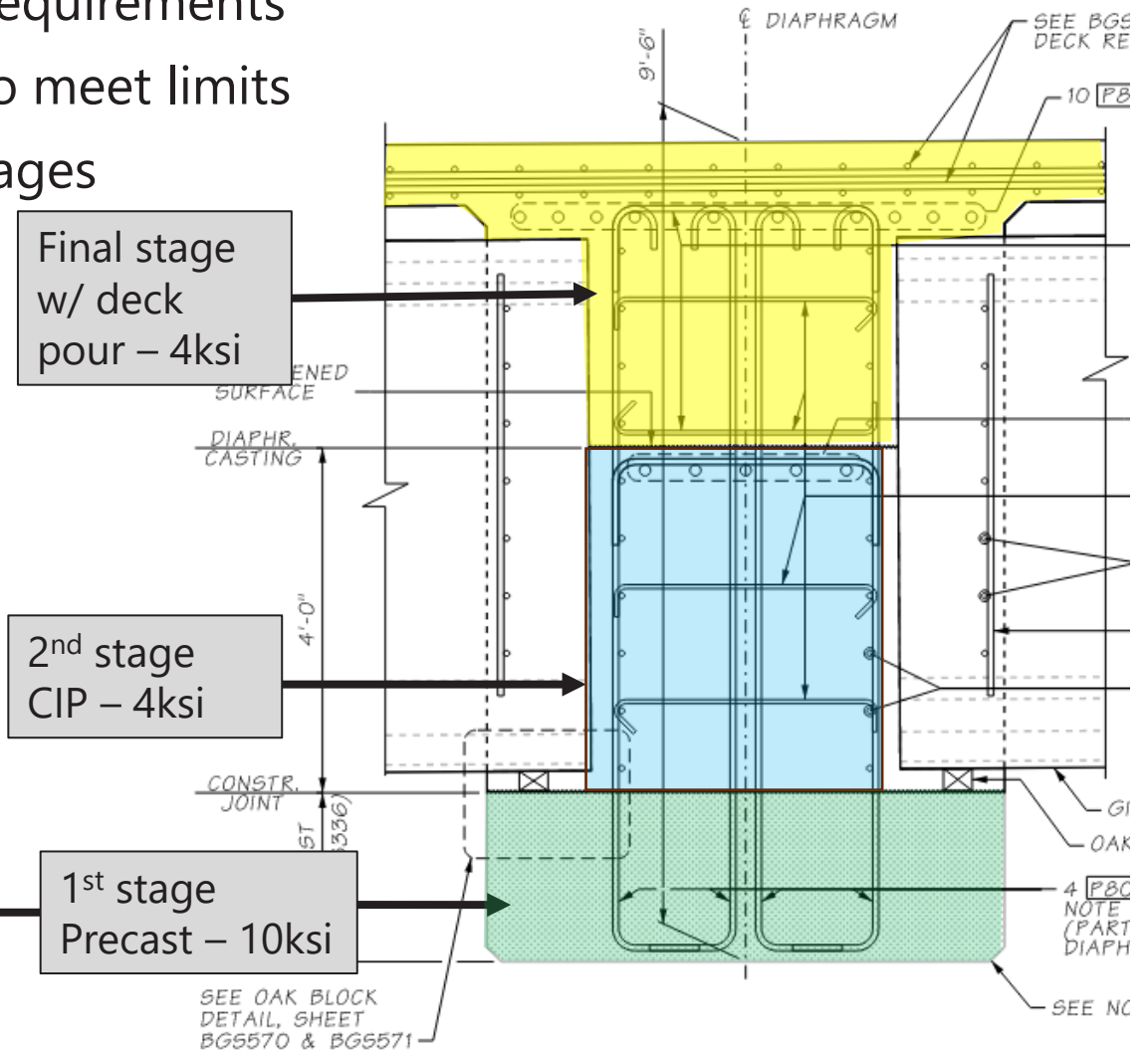
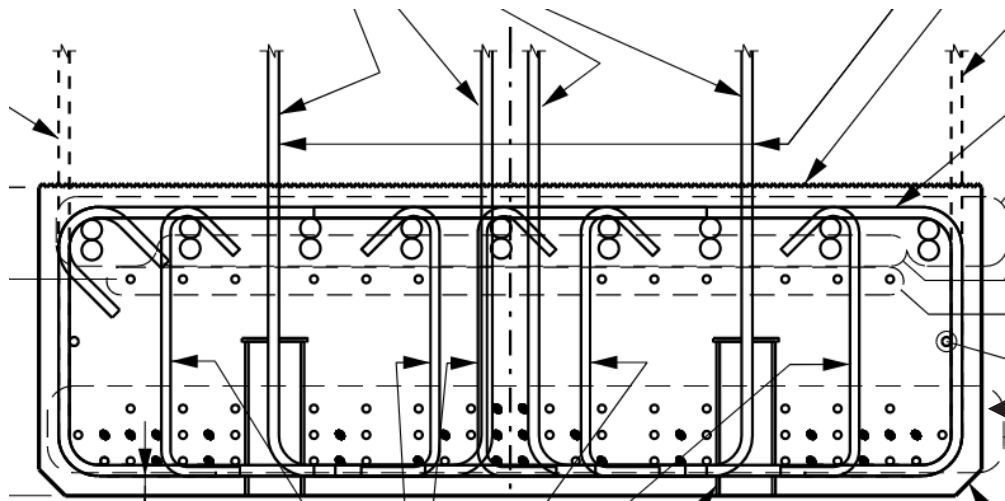
# Design Challenges – Precast Columns Piers 37-40





# Design Challenges – Precast Diaphragms

- Diaphragms limited in depth by contract requirements
- Mix of prestressing and reinforcing used to meet limits
- Checked crack width and strength in all stages
- Piers 4 and 5 has two pours for 2<sup>nd</sup> stage



# Design Challenges – Precast Diaphragms

## Deflections

- Oak block elevations supporting precast girders adjusted to account for prestressing and dead load deflections.

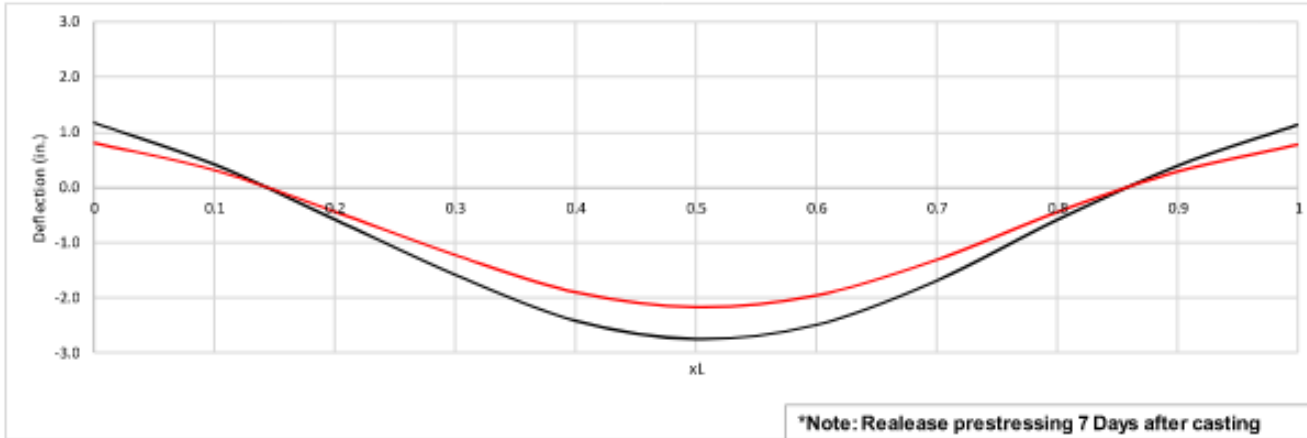


Figure 6.58 Total deflection of Pier cap 4.

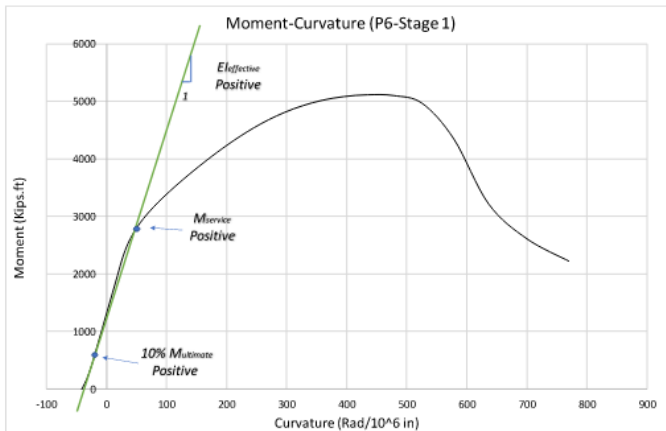


Figure 6.59 EI effective calculation concept (Pier 6)-Stage 1- Positive Section.

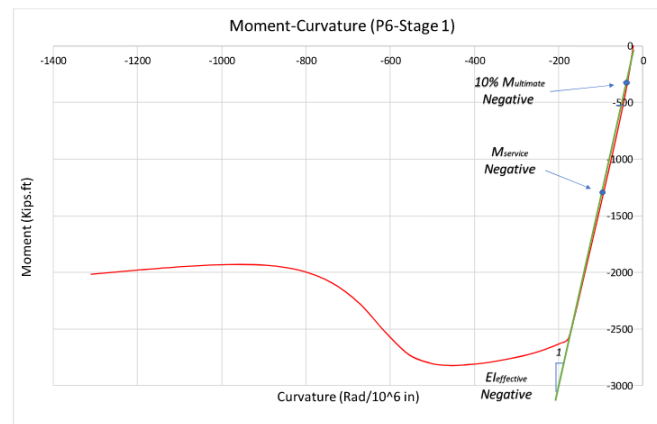


Figure 6.60 EI effective calculation concept (Pier 6)-Stage 1- Negative Section.

# Seismic Design – Time History Analysis

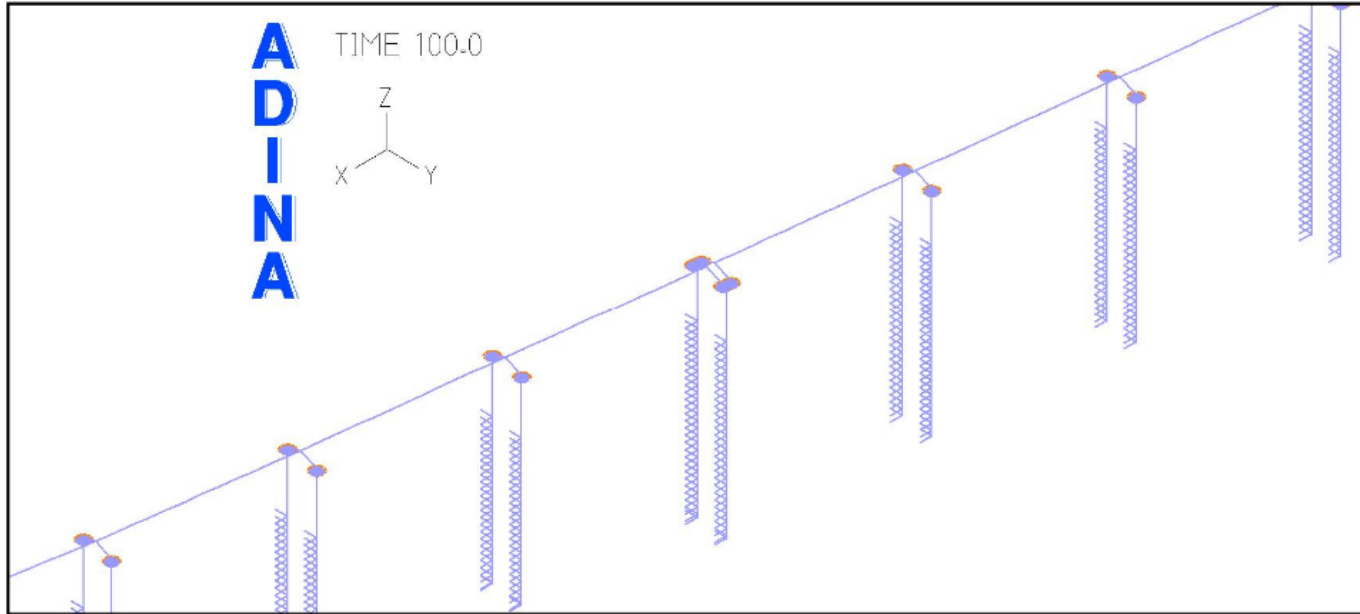


Figure 2: ADINA Typical Foundation Modeling (Shaft PY springs)

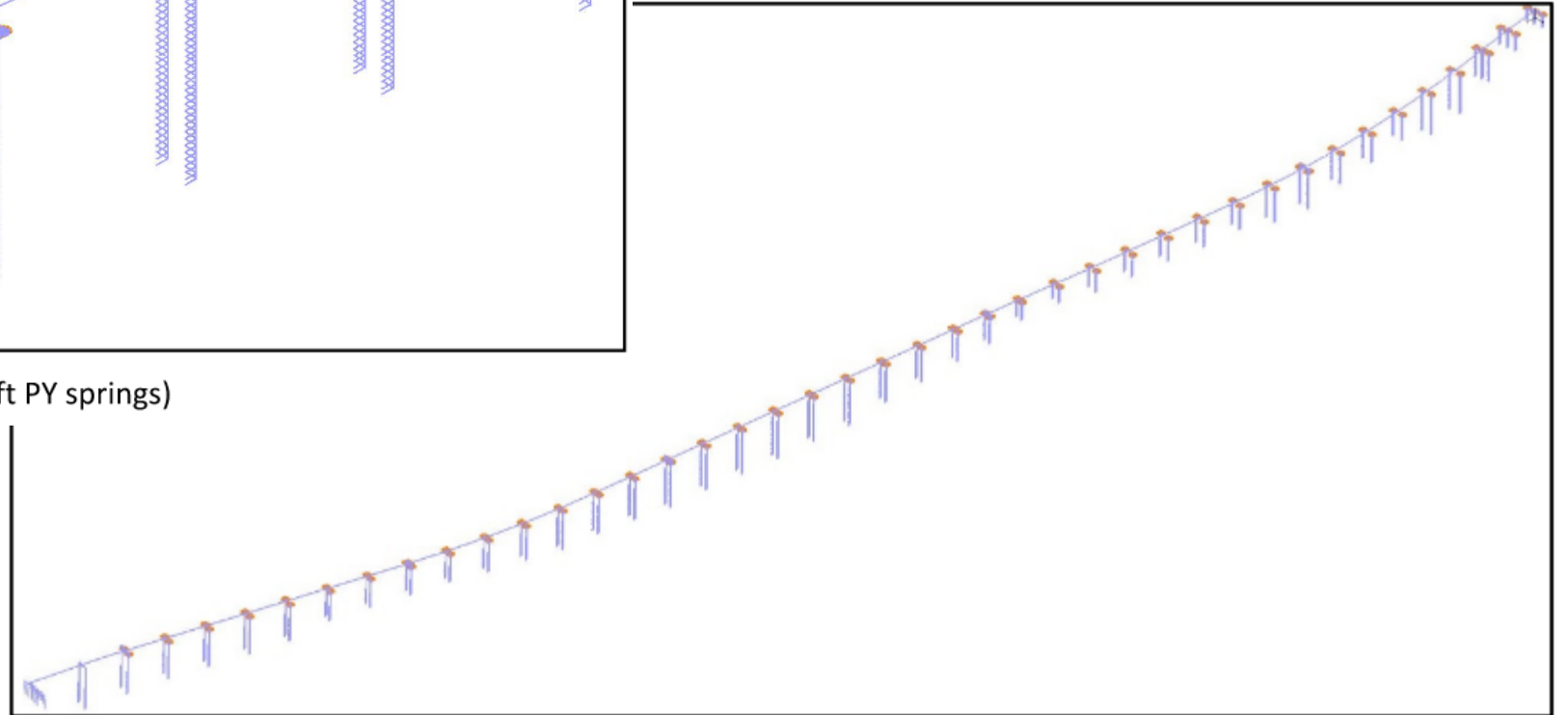
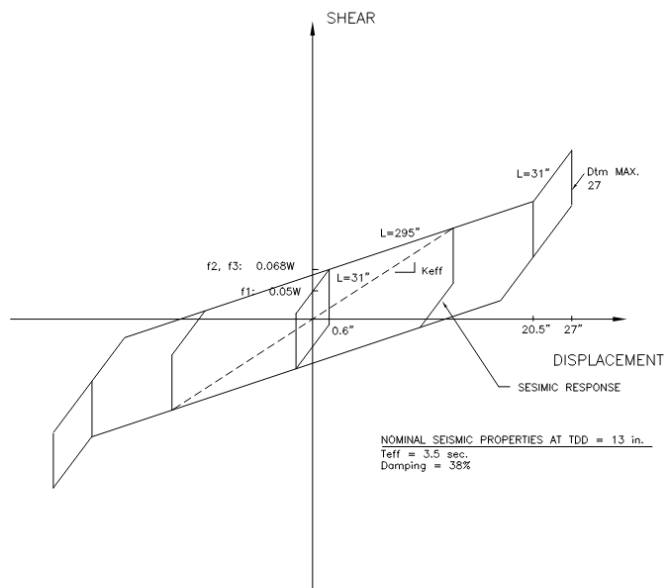
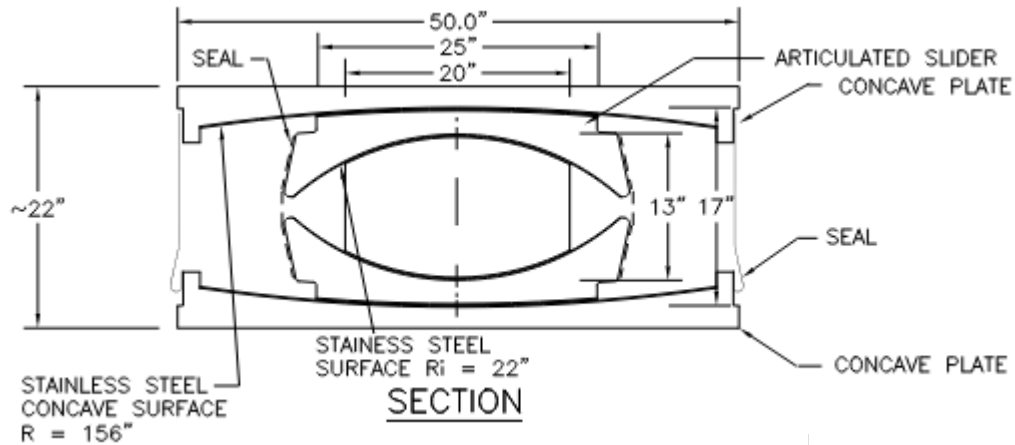


Figure 1: ADINA Global Model Abutment 1 to Floating Bridge Pontoon

# Seismic Design – Isolation Bearings



NOMINAL FORCE DISPLACEMENT LOOP  
AT QUALITY CONTROL TEST VERTICAL LOAD

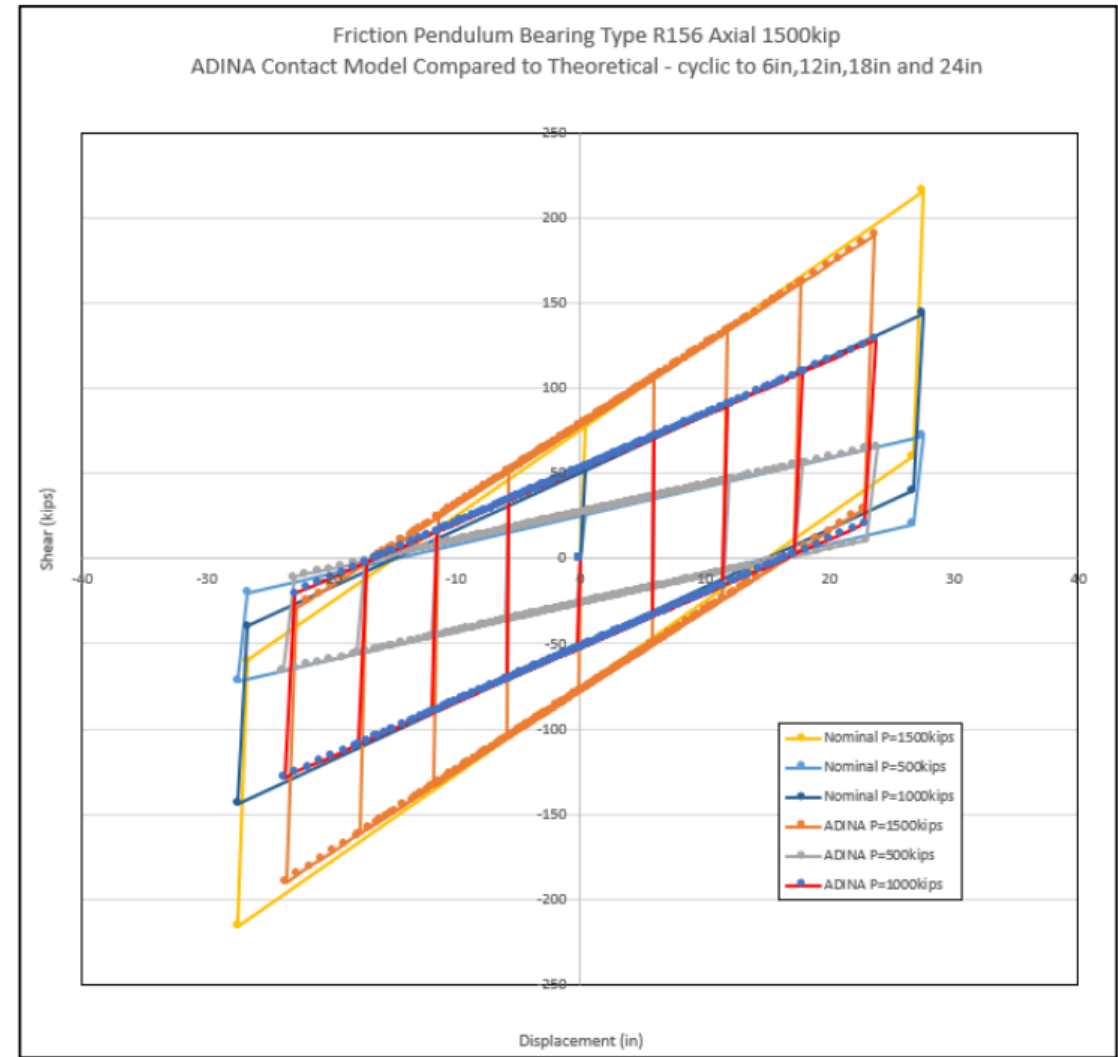


Figure 3C: ADINA Friction-Pendulum Hysteretic Response with Varying Axial Load

# Seismic Design – Time History Analysis

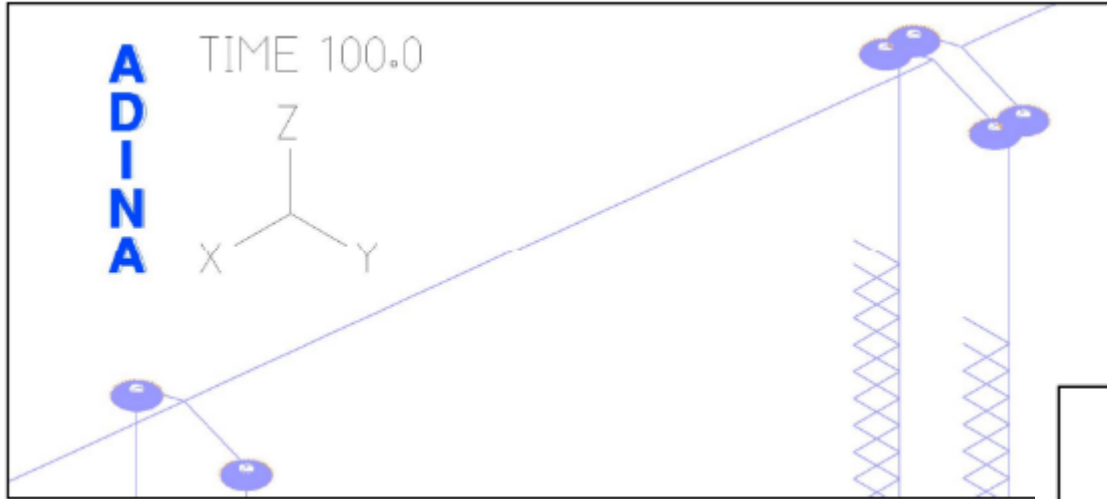


Figure 3A: ADINA Friction-Pendulum Modeling (Expansion Joint and Typical)

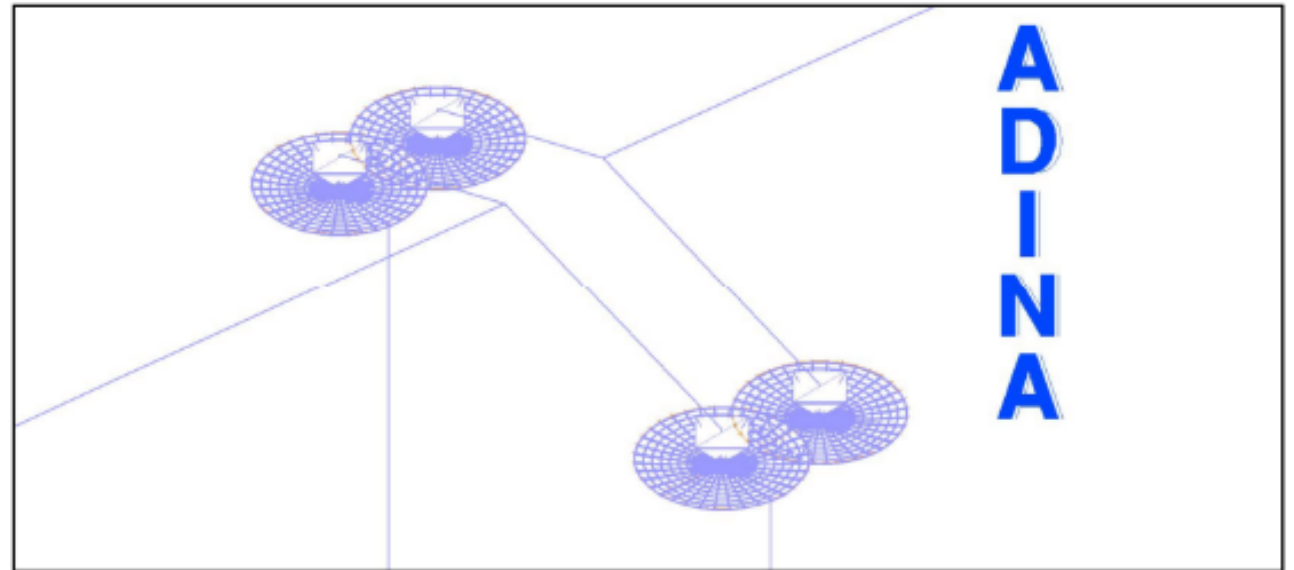


Figure 3B: ADINA Friction-Pendulum Modeling Close-Up of Contact Surface Modeling



# Seismic Design – Ground Motions

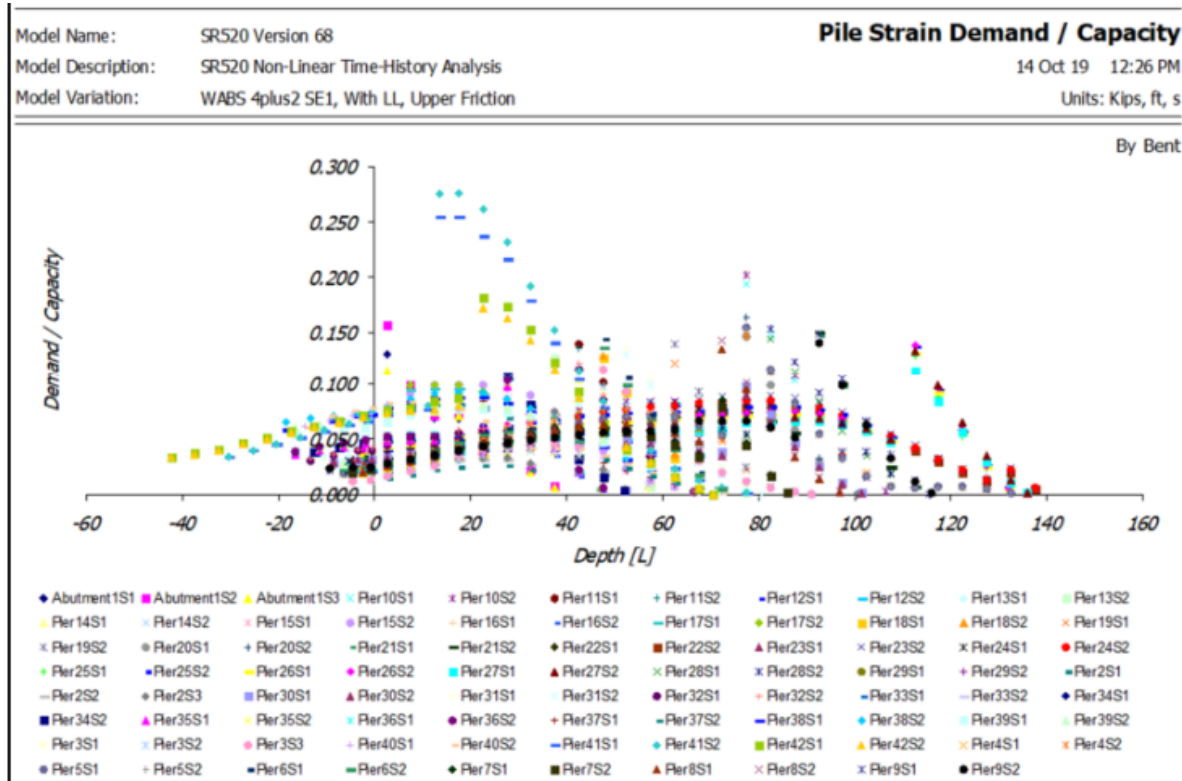


Figure 13-5: Pile Strain Demand/Capacity Ratio 5 of 18 Analysis

**SR 520 Montlake to Lake Washington I/C and Bridge Replacement**

**Control Table for WABS input motions**

**General Notes:**

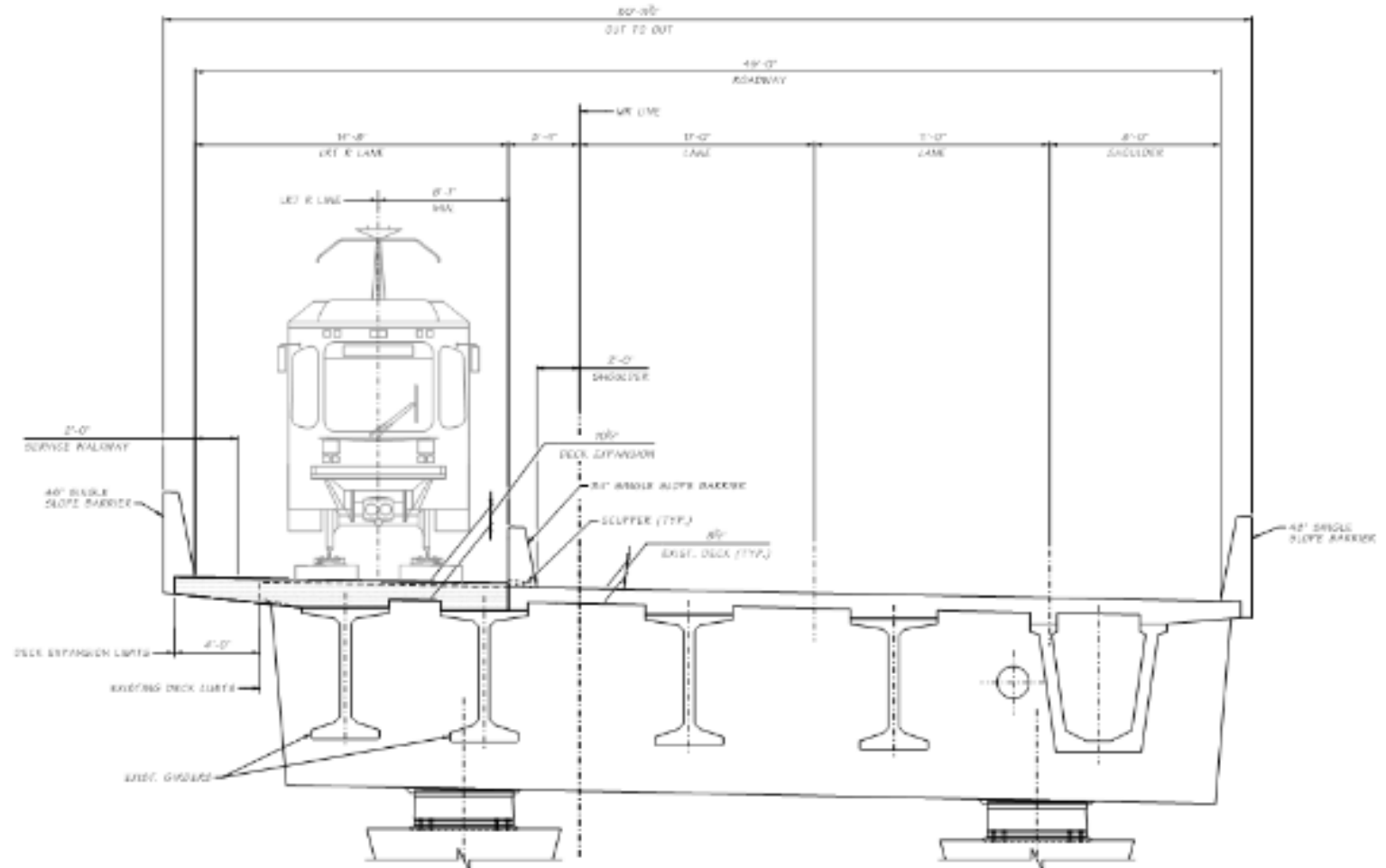
- Motions spectrally matched to the envelope of the CMS spectra provided in Appendix G3 of the RFP
- FN = Fault Normal; FP = Fault Parallel; CE = Crustal Earthquake, SE = Subduction, RSN = record sequence number
- Units: Time in seconds; Displacement in feet; Acceleration in feet/second/second.
- Refer to "Earthquake Acceleration Time Histories" memorandum prepared by Golder dated July 5<sup>th</sup>, 2019

Source	Motion ID	Component ID	Earthquake (Station)	Time Step (sec)	Total Duration (sec)	Recommendation
All Crustal	CE1	Horizontal: FN/FP Vertical: UD	Chichi, Taiwan Mw7.6 (RSN1511, TCU)	0.005	89.995	Apply FN along North-South; Apply FP along East-West
	CE2	Horizontal: C1/C2 Vertical: UD	KernCounty, USA Mw7.4 (RSN15, TLS)	0.01	66.39	No significant directionality
Crustal Background	CE3	Horizontal: 00/90 Vertical: UD	Loma Prieta, USA Mw6.9 (RSN762, FRE)	0.005	54.245	No significant directionality
	CE4	Horizontal: EW/NS Vertical: UD	Chuetsu-oki, Japan Mw6.8 (RSN4866, Kawanishi Izumozaki)	0.01	63.99	No significant directionality
Source	Motion ID	Component ID	Earthquake (Station)	Time Step (sec)	Total Duration (sec)	Recommendation
Subduction Interface	SE1	Horizontal: 90/360 Vertical: UD	Maule, Chile Mw8.8 (STL)	0.01	159.99	No significant directionality
	SE2	Horizontal: EW/NS Vertical: UD	Tohoku, Japan Mw9.0 (IBR011)	0.01	259.99	No significant directionality
	SE3	Horizontal: 90/360 Vertical: UD	Maule, Chile Mw8.8 (SJCH)	0.01	159.99	No significant directionality
	SE4	Horizontal: EW/NS Vertical: UD	Tohoku, Japan Mw9.0 (CHB004)	0.01	259.99	No significant directionality
Subduction Intraslab	SE5	Horizontal: 270/360 Vertical: UD	El Salvador Mw7.6 (MG-4359)	0.005	64.495	No significant directionality

Figure 4: Time History Ground Motions

# Future Compatibility – 4+2 Configuration

- Design structure to accommodate possible future light rail configuration



# Future Compatibility – 6+2 Configuration

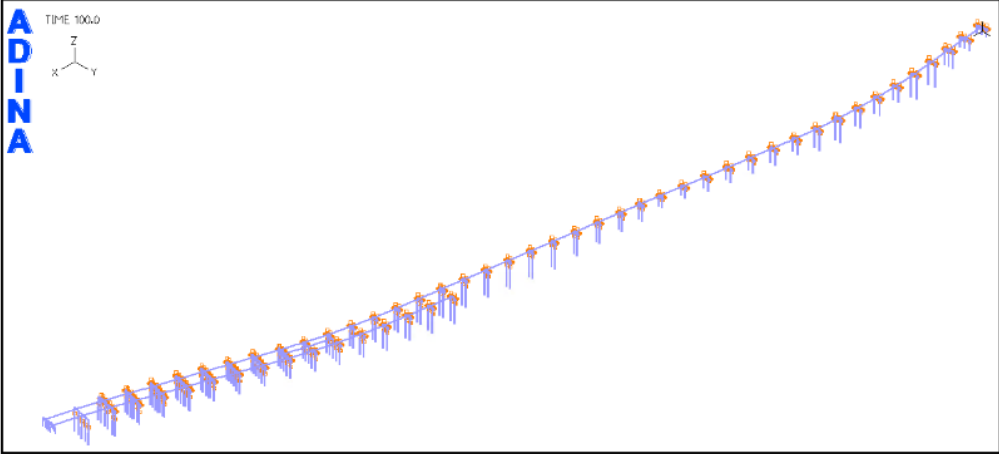
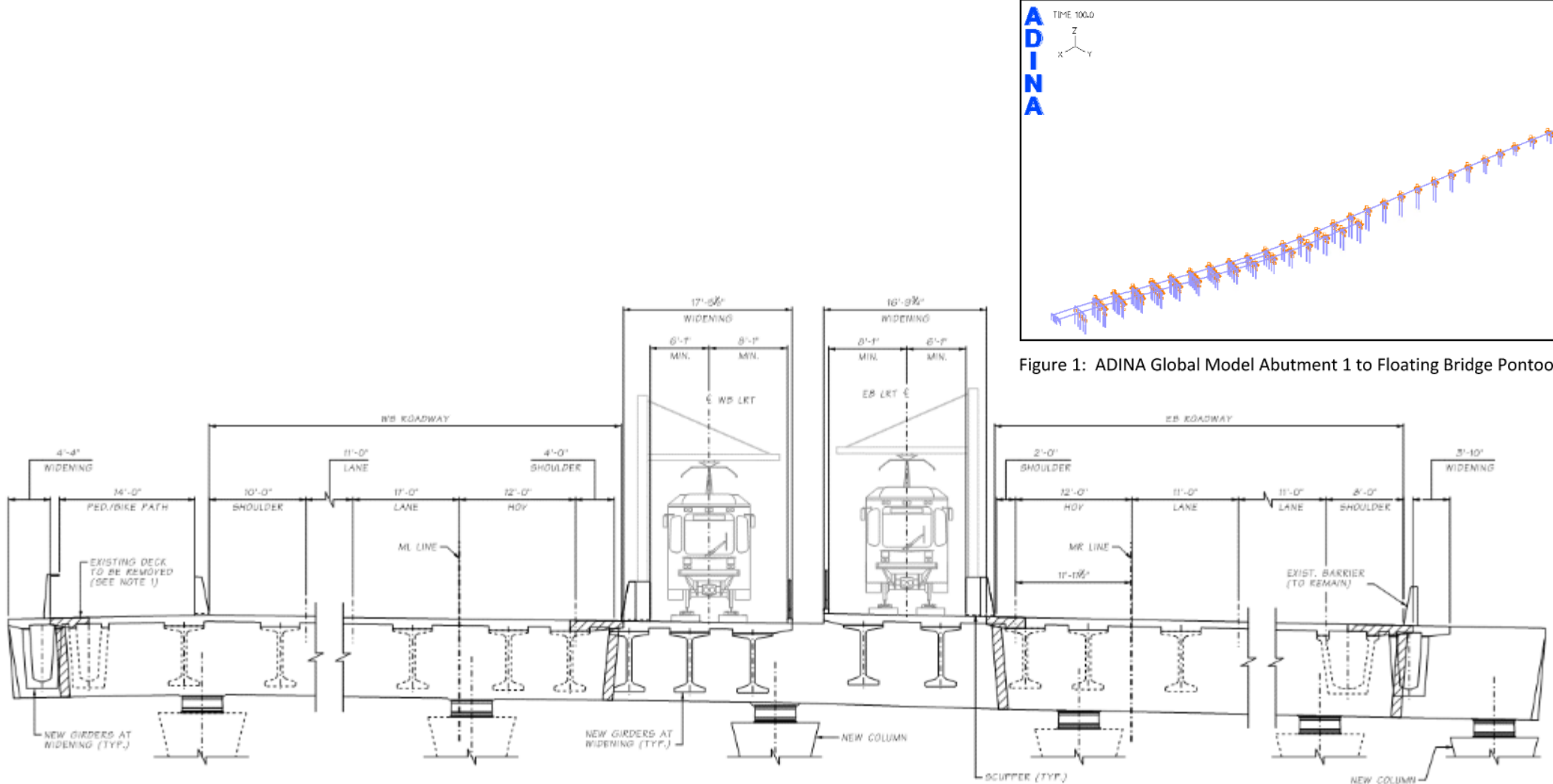
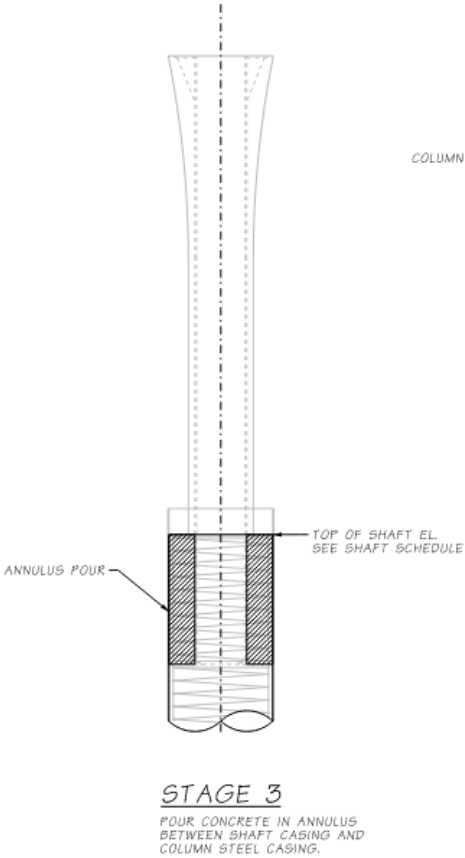
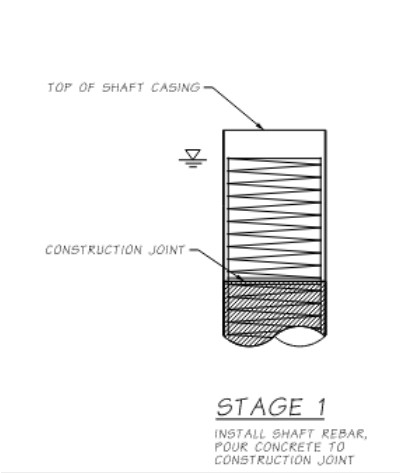


Figure 1: ADINA Global Model Abutment 1 to Floating Bridge Pontoon

# Construction

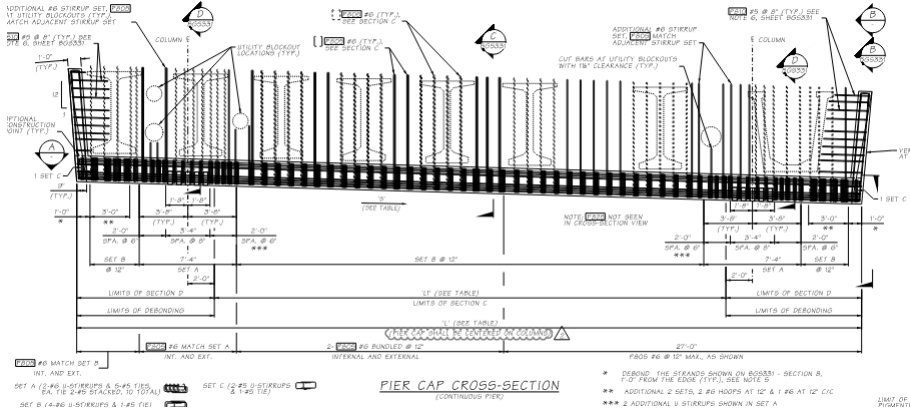
- Precast Columns for Accelerated Construction
- May 2021 to June 2021





# Construction

## Precast Cross Beams Avoids Falsework (Oct 2021 – Nov 2021)





# Construction

– Precast Crossbeam





# Construction

- Setting the Girders  
(March 2022)
- Unbalanced loads on support jacks critical





# Construction

## Deck Forms (March 2022)

Starting at one end

Working West to East

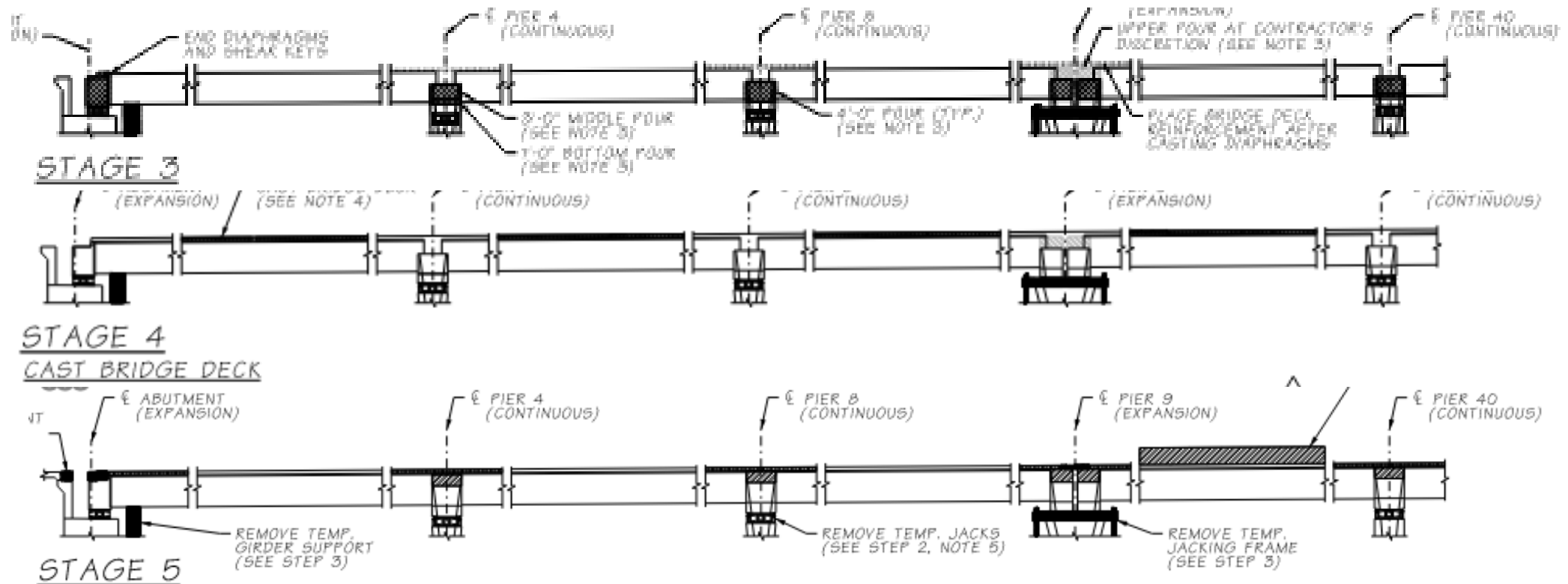
Assembly Line fashion



# Construction

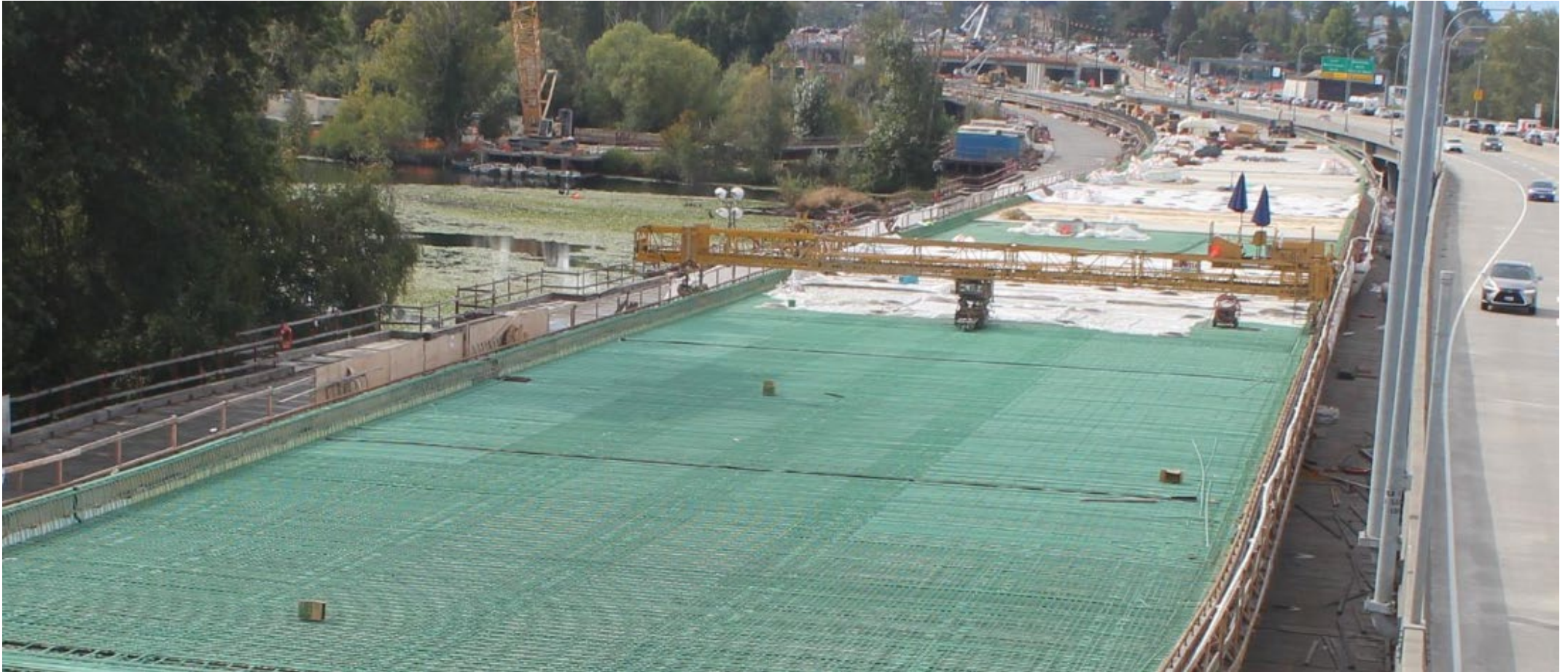
## - Deck Sequence Critical

Detailed check of the all the critical stage was required as part of the design.





# Deck Construction





# Deck Construction

- Cast each side ready for the deck over the pier (Oct 2022)





# Questions?

