

Project Team



Coffman Ames Joint Venture

DESIGN TEAM





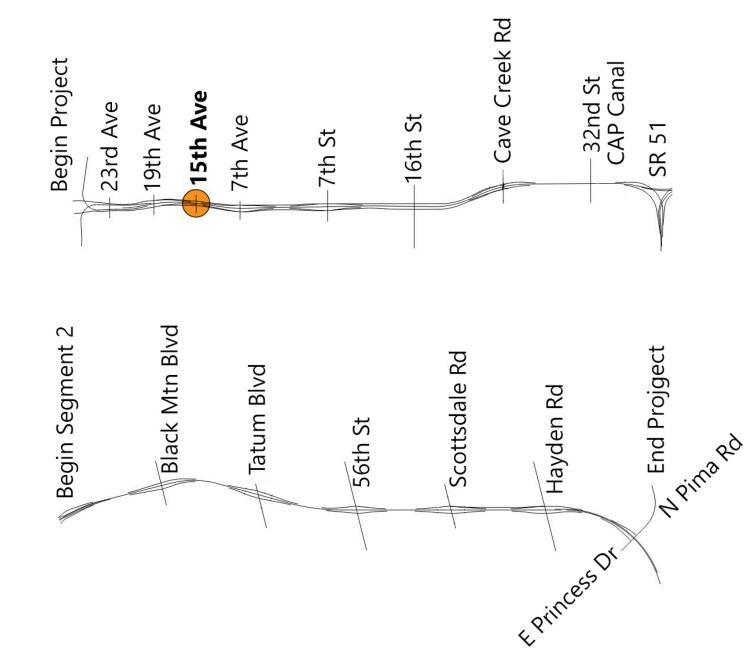






Site Location

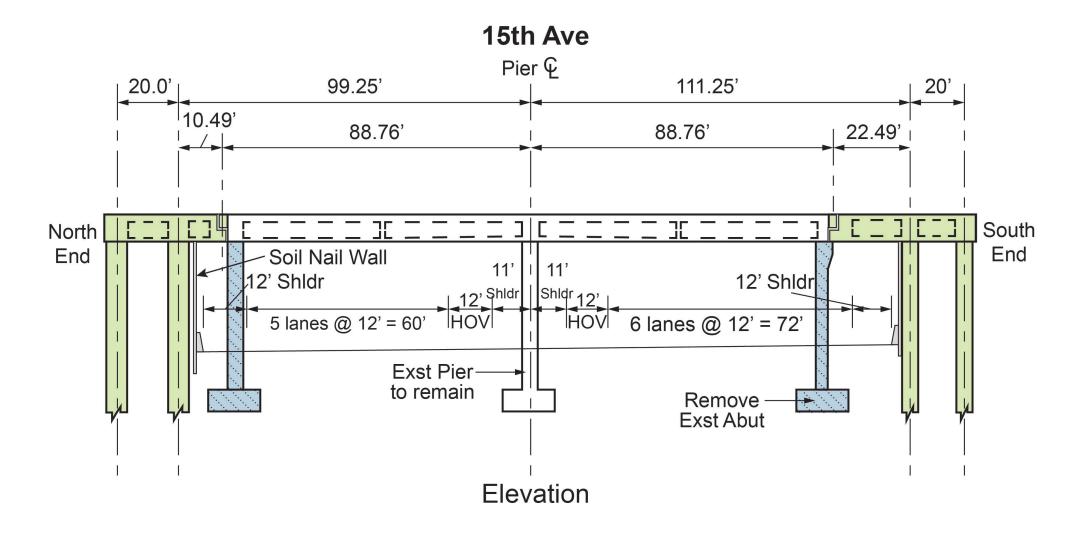
SR 101L, I-17 TO PRINCESS DR





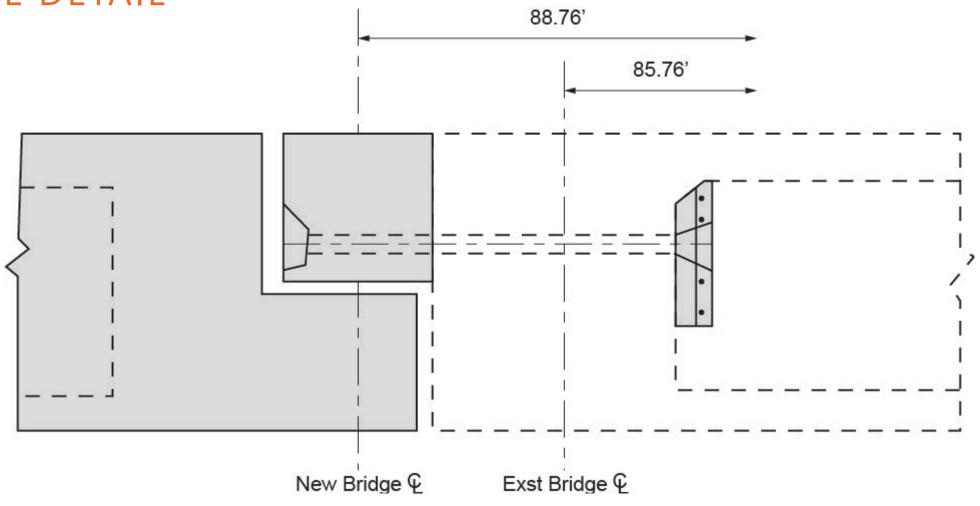
- -Idea: How to lengthen a span?
- Alternate Technical Concept ATC
 - Dimension Changes
 - Metric to English Lanes
 - Sidewalk 4.92 ft to 6.00 ft
 - Design Specification
 - Existing Bridge: AASHTO Std Specifications 17th Edition 2002
 - New Bridge: AASHTO LRFD Bridge Design Specifications, 8th Edition
 - Allowable Tension in Existing Bridge
 - 179 psi to 256 psi
 - AASHTO allowable 402 psi
 - Requested use of 4 sqrt(f'c) or 268 psi



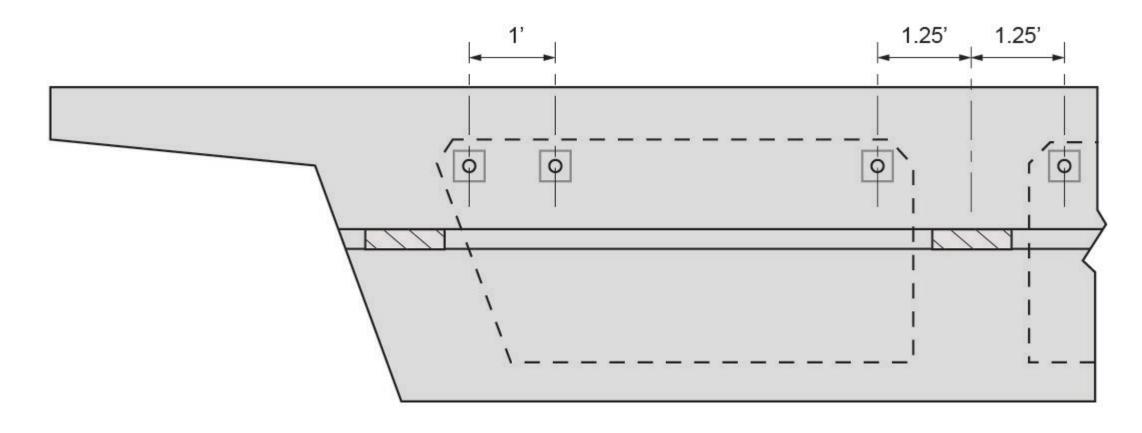




HINGE DETAIL



P/S BARS



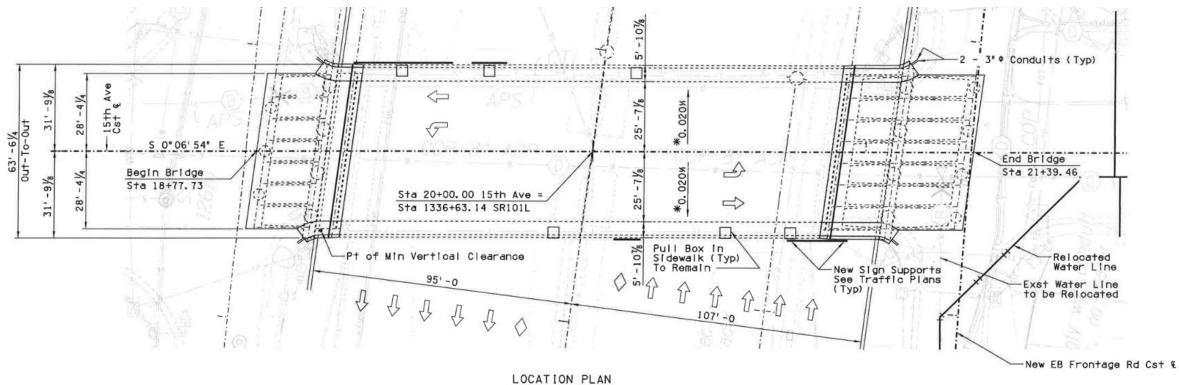
CHANGES FROM PURSUIT

- Hinge Connection Changed
 - Top Corbel Connection Changed to Full Depth Connection
 - Added Bottom P/S Bars with Dowels
 - Existing Span increased from 3.00 feet to 4.77 feet
 - Tension increased from 179 psi to 256 psi to 300 psi
- -Wall Changed
 - Soil Nail Wall Changed to Cast-in-Place Pier Wall
- End Span Changes
 - Frame 2 End Span increased to 25 feet

DESIGN ISSUES

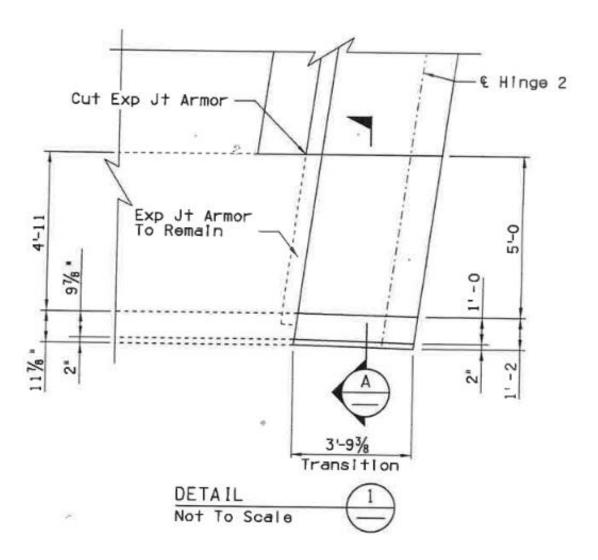
- -Conversion of Existing Metric to English
- -Shallow Superstructure Depth (3.94 feet)
 - Hinge Design
 - Construction Access
- Use of Two Design Specifications
 - Existing: Standard Specifications 17th Edition 2002
 - New: LRFD Bridge Design Specifications, 8th Edition
- -Traffic Access on Frontage Road
- -Existing Abutment Wall Strength

PLAN



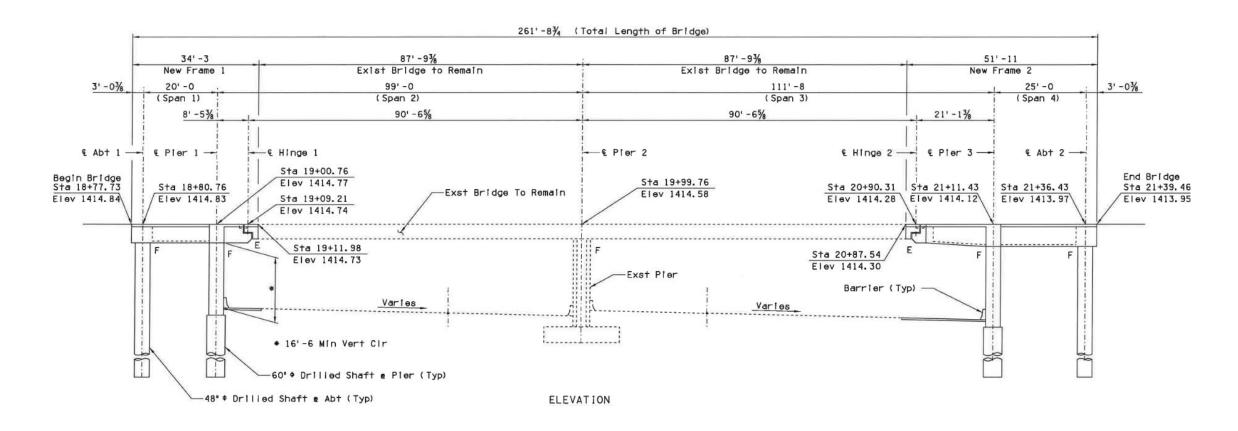
New C. I.P. Post-tensioned Box Girder Bridge Extension

BRIDGE FLARE





ELEVATION

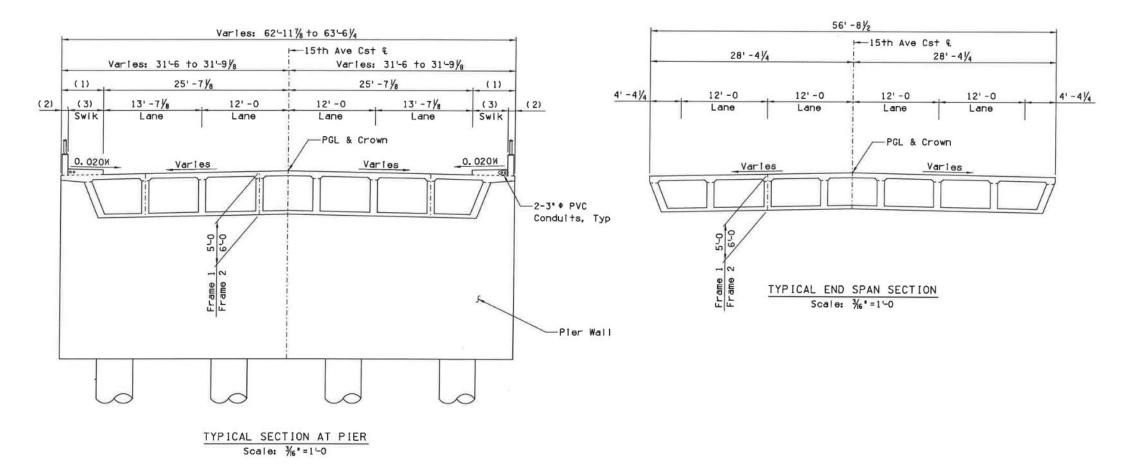




Location	Span (feet)	Depth (feet)	Width (feet)	Structure Type
Back Span 1	20.00	5.00	56.71	Box Girder
Overhang	8.45	5.00	63.00	Solid
Existing Span 2	90.55	3.94	63.00	Box Girder
Existing Span 3	90.55	3.94	63.00	Box Girder
Overhang	21.11	Varies: 5.0 to 6.0	63.00	Box Girder
Back Span 4	25.00	6.00	56.71	Box Girder



TYPICAL SECTION



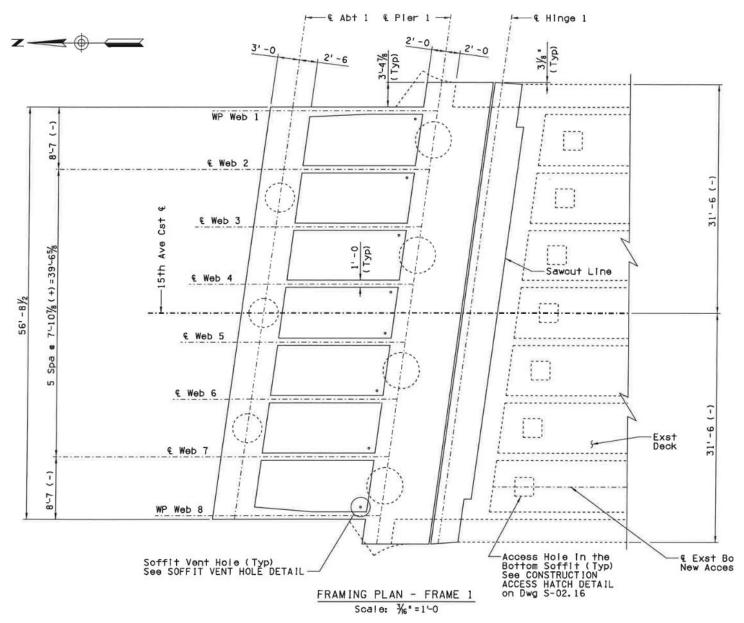


	Number Shafts	Diameter (inch)				Strength Load Uplift (Kips)	Strength Load Down (kips)
Abutment 1	3	48	35.0	-228	206	-338	328
Pier 1	4	60	41.0		869		1170
Pier 3	4	60	48.5		1109		1496
Abutment 2	4	48	35.5	-287	135	-379	175

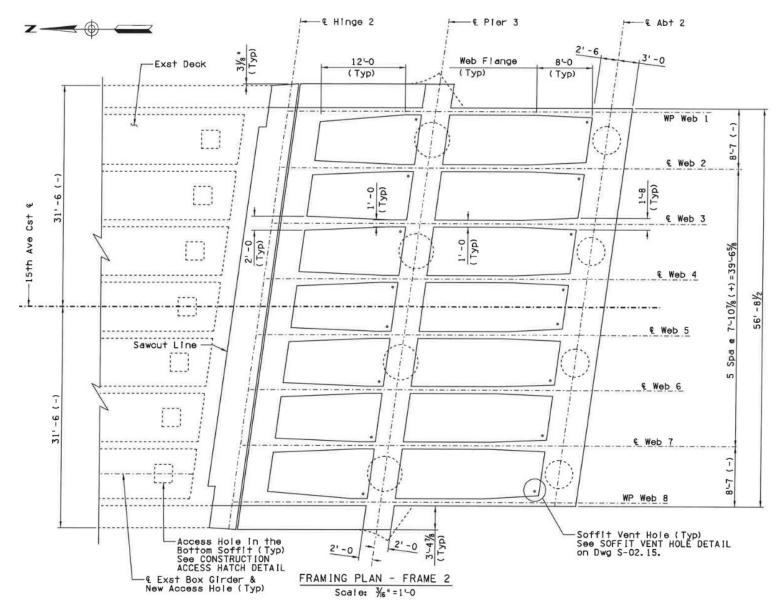


FRAME 1

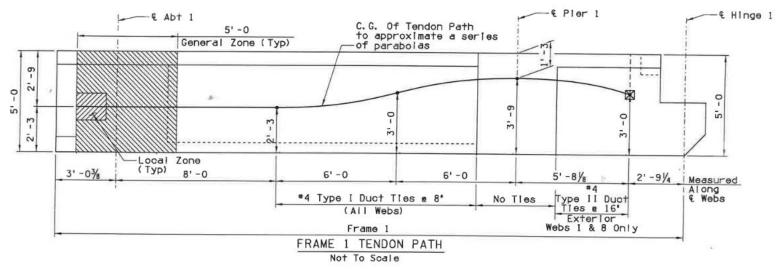
- Web Spacing
- -DS Location
- -Solid Span
- -Flare
- -Width Change

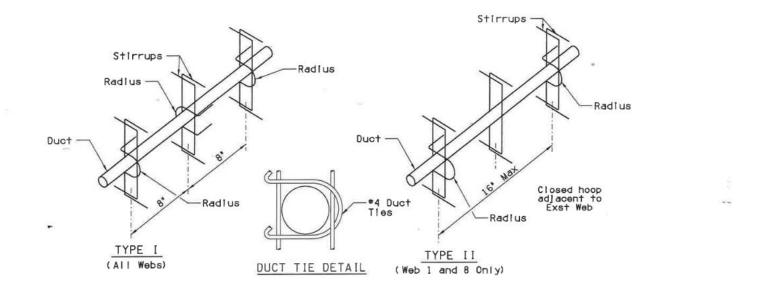


FRAME 2



FRAME 1

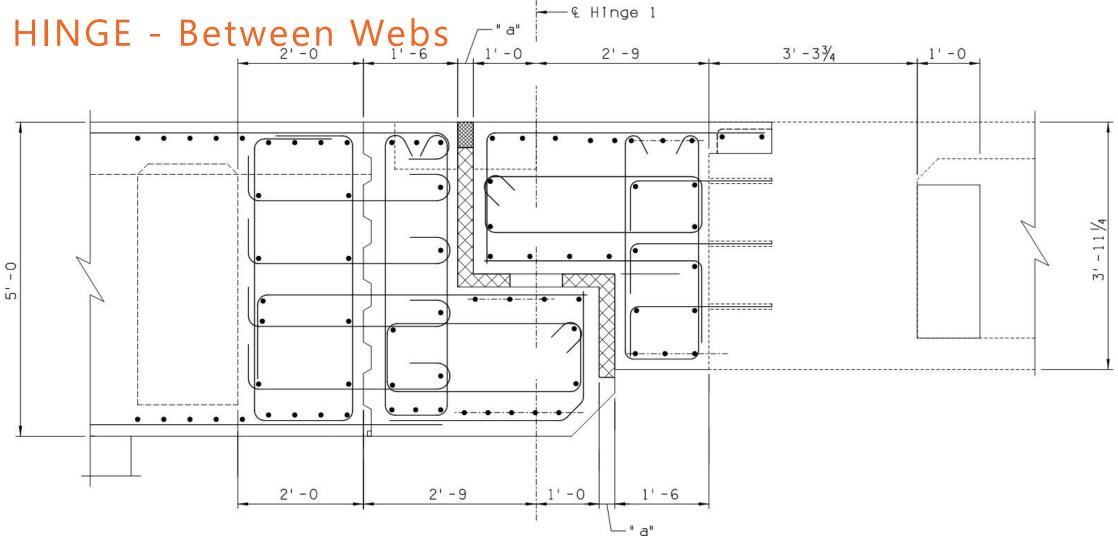


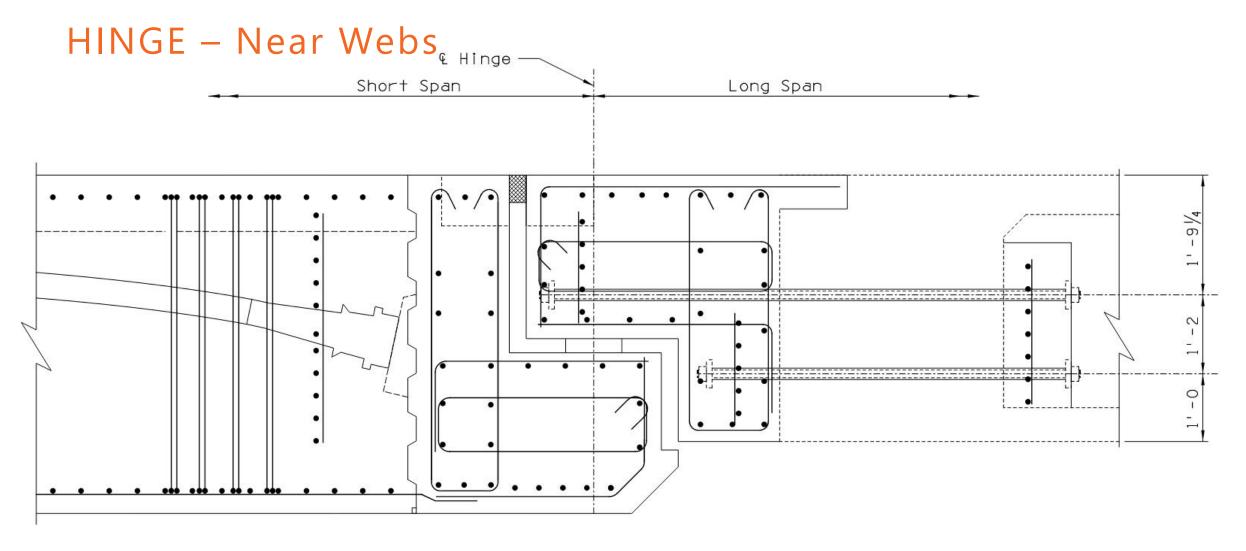




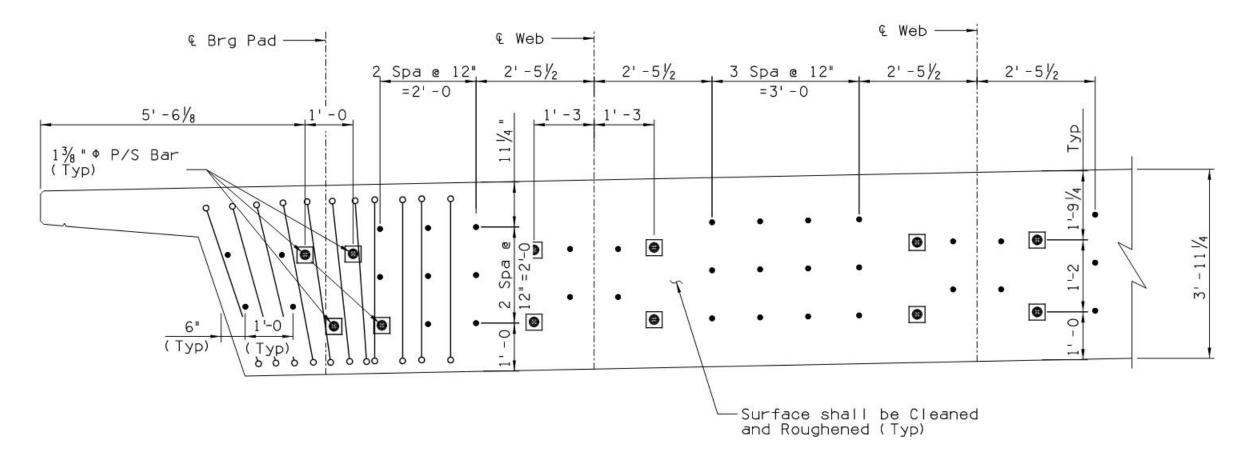
	Frame 1	Frame 2	
Pjack	7031	16,874	kips
Number Strands	160	384	(0.6" Dia.)
Strands	20	48	Per web
Duct Ties	Yes	No	
f'c Deck	3.5	3.5	ksi
f'c Web	3.5	4.0	ksi



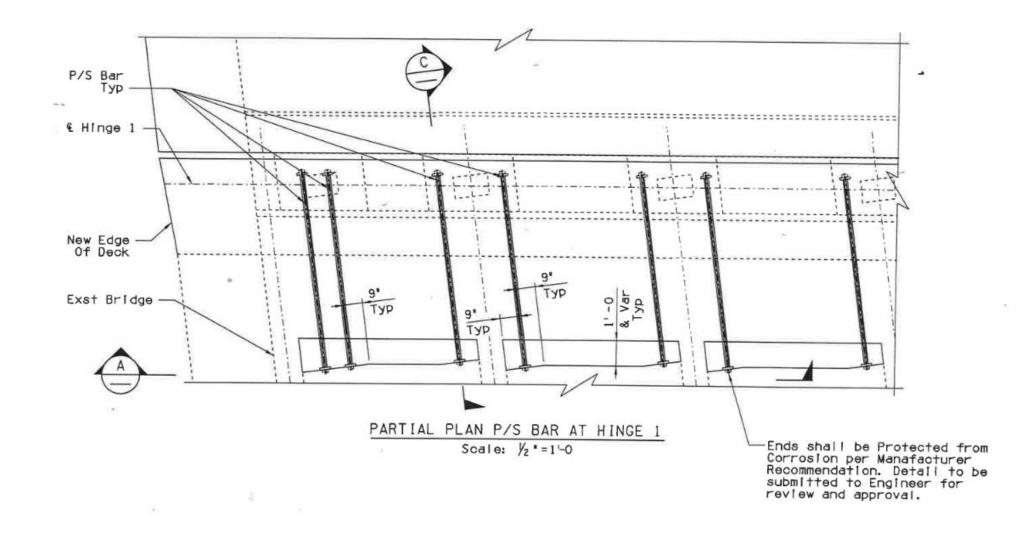




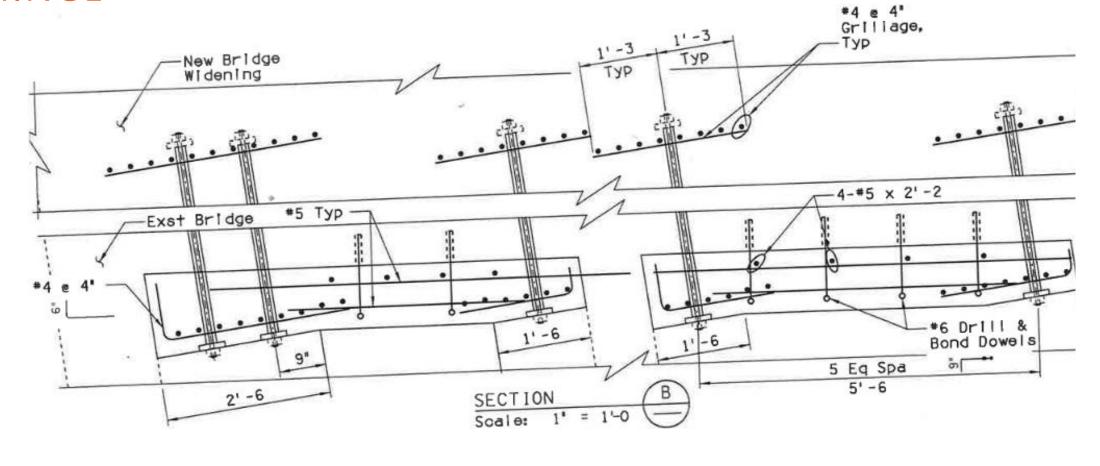
HINGE - Section



HINGE

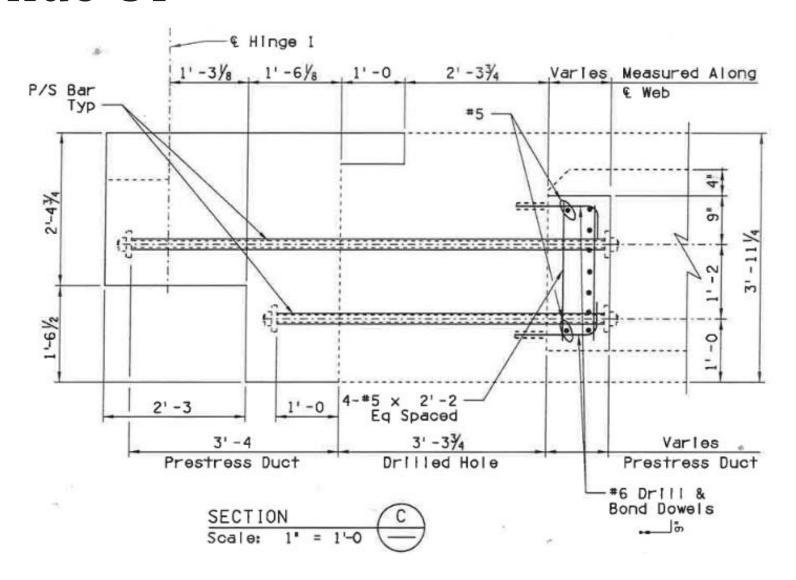


HINGE





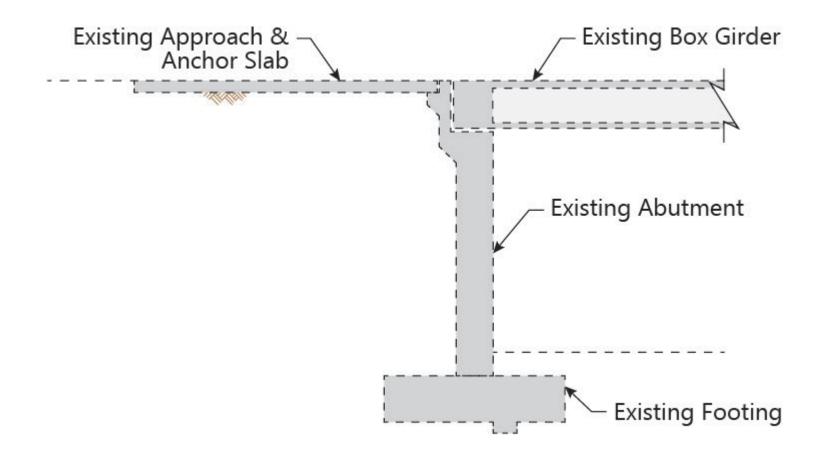
HINGE



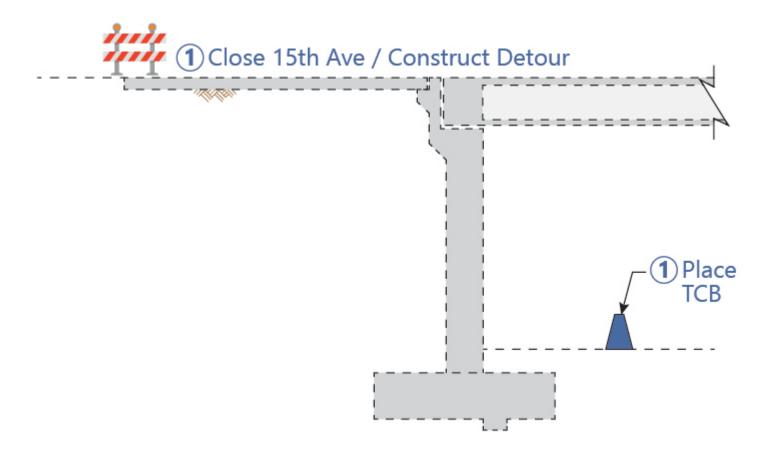
ltem	
Prestress Bar	Type 2 Deformed
Size	1 3/8" Diameter
Area Bar	1.58 sq inch
fpu	150 ksi
Jacking	0.70 fpu
Pjack Force	5309 kips per Frame

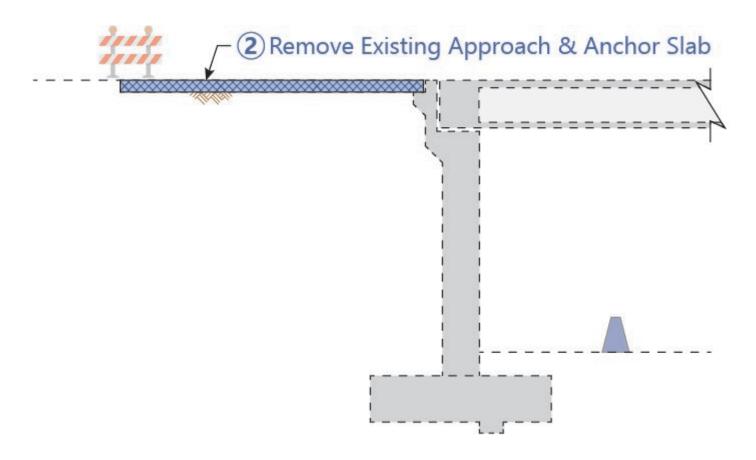


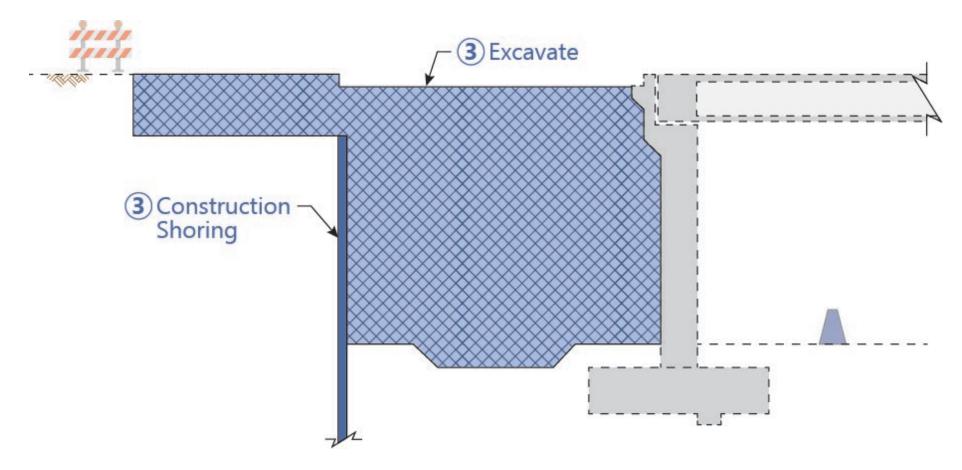
EXISTING



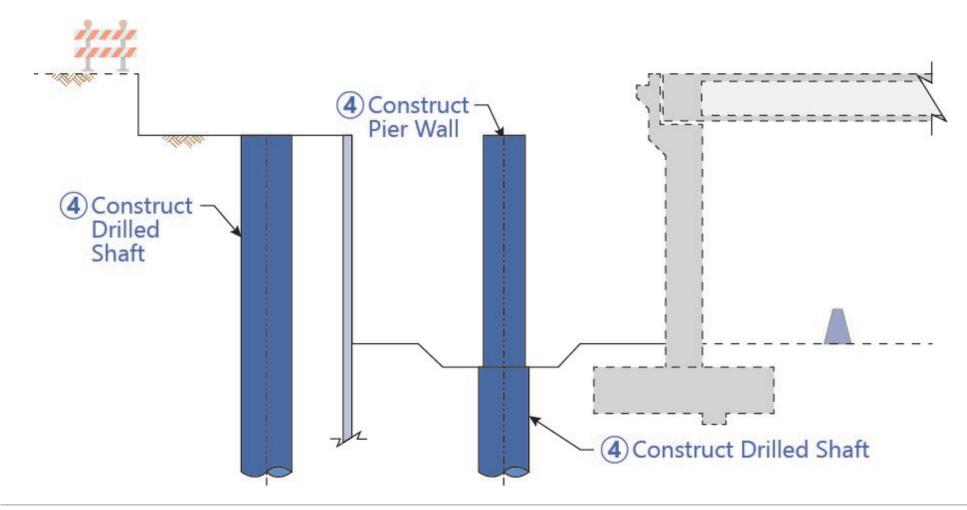






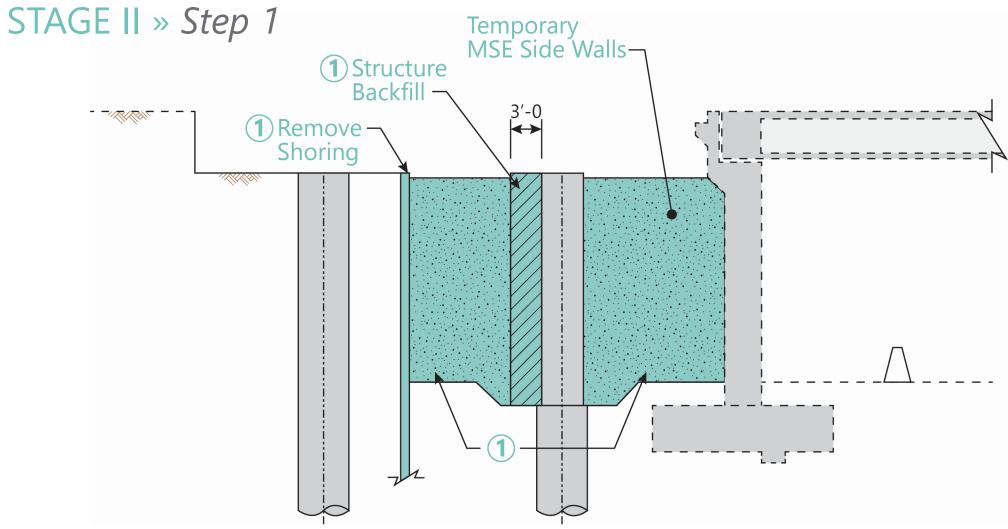




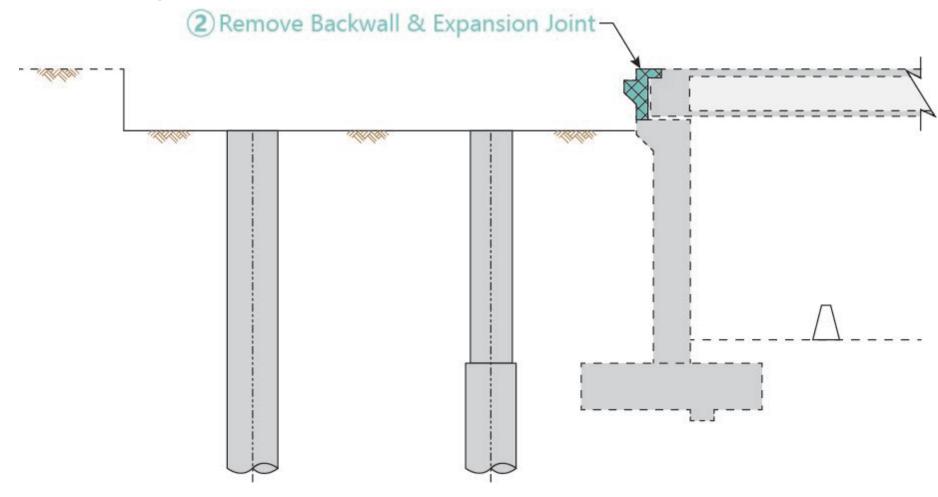


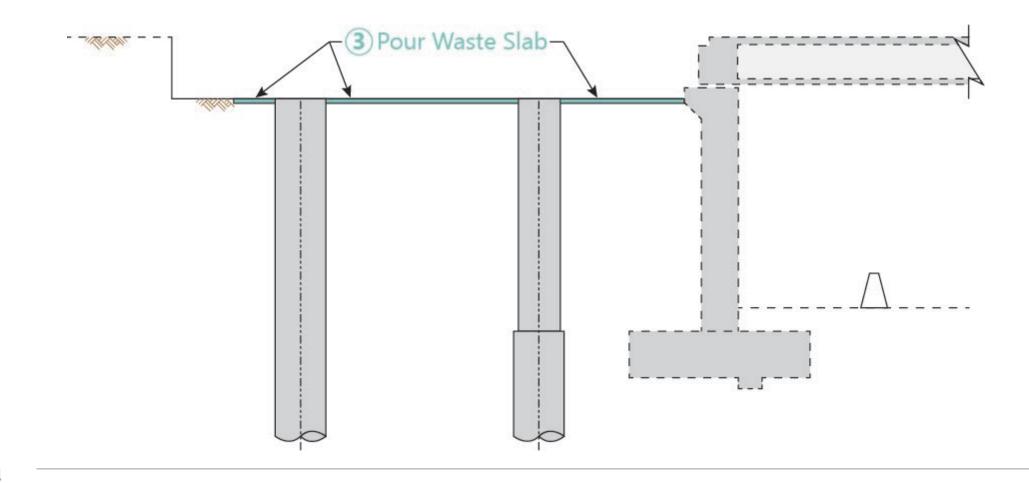


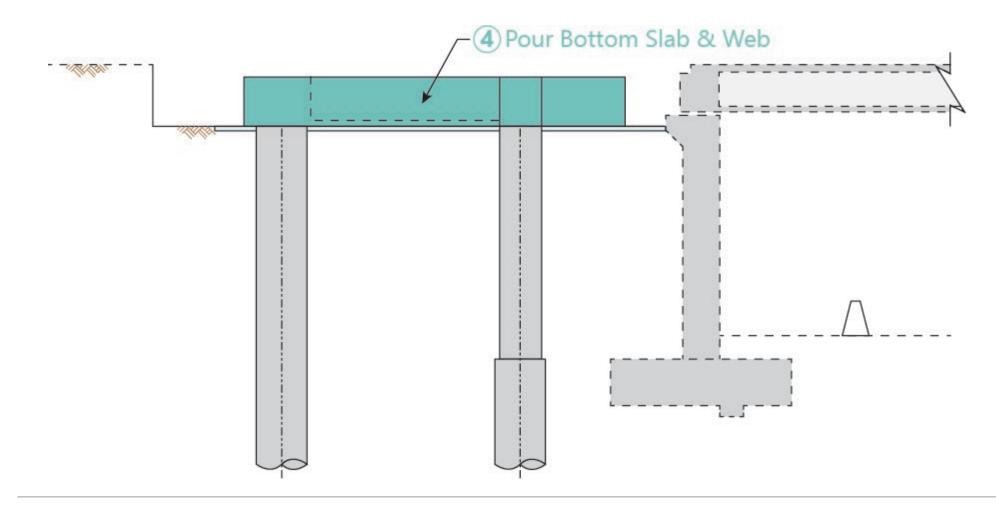
15th Avenue UP

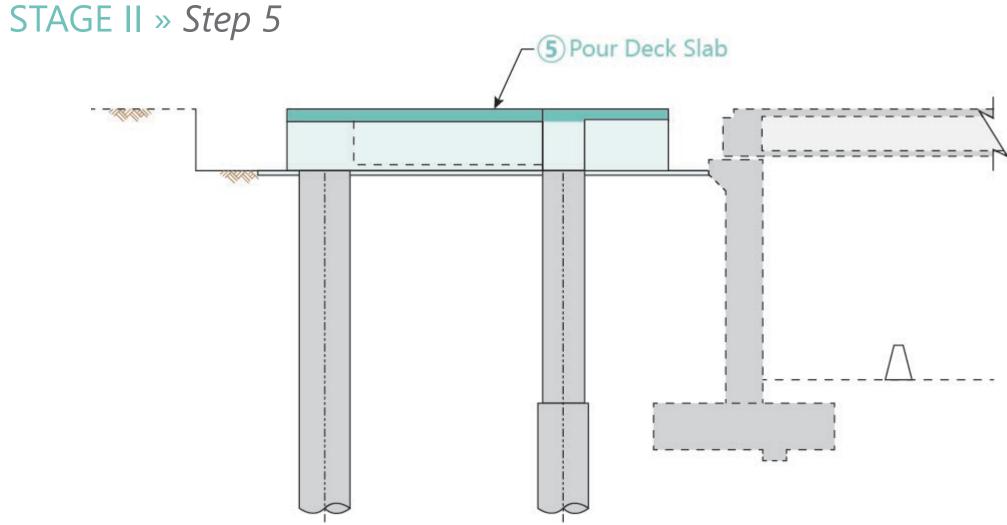


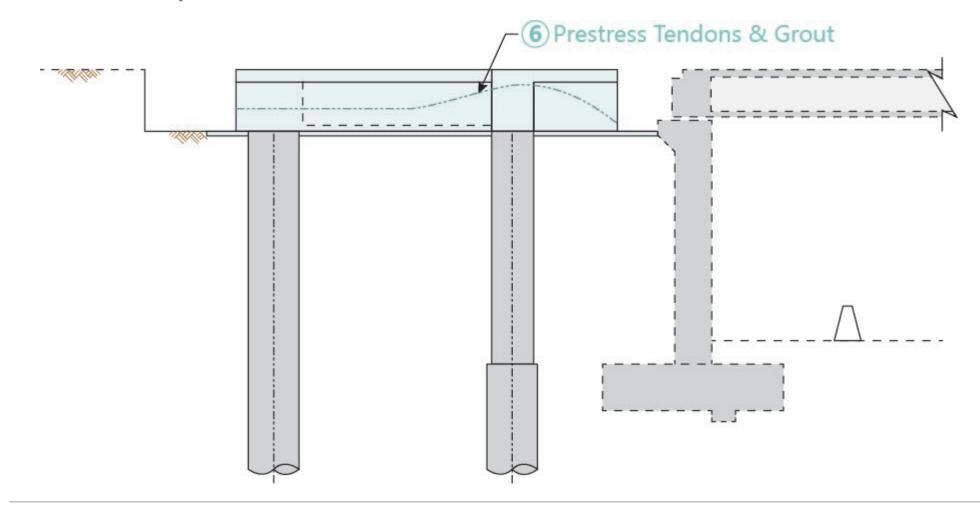


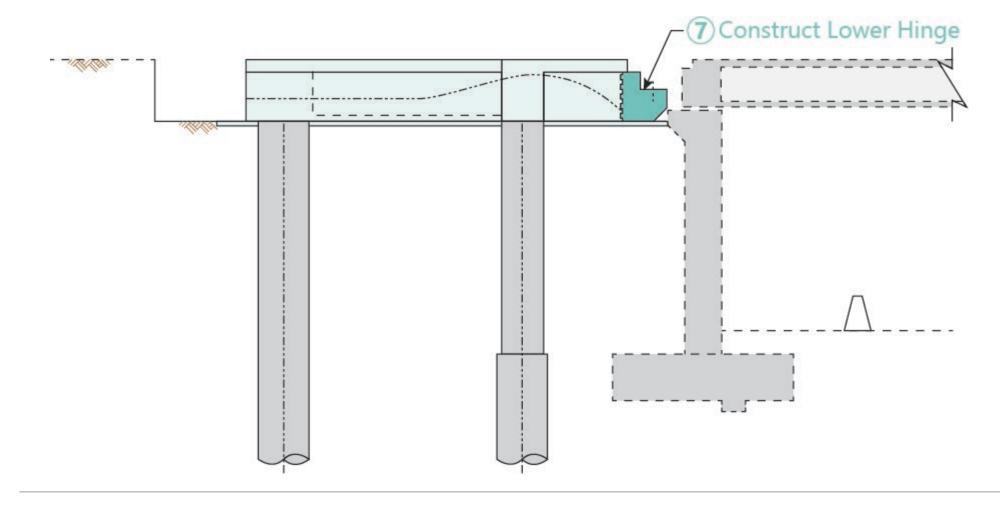


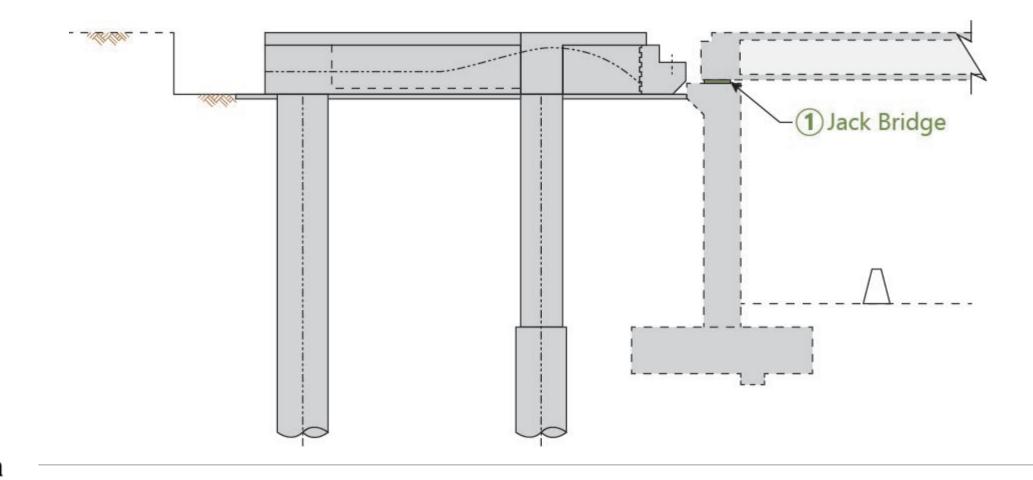


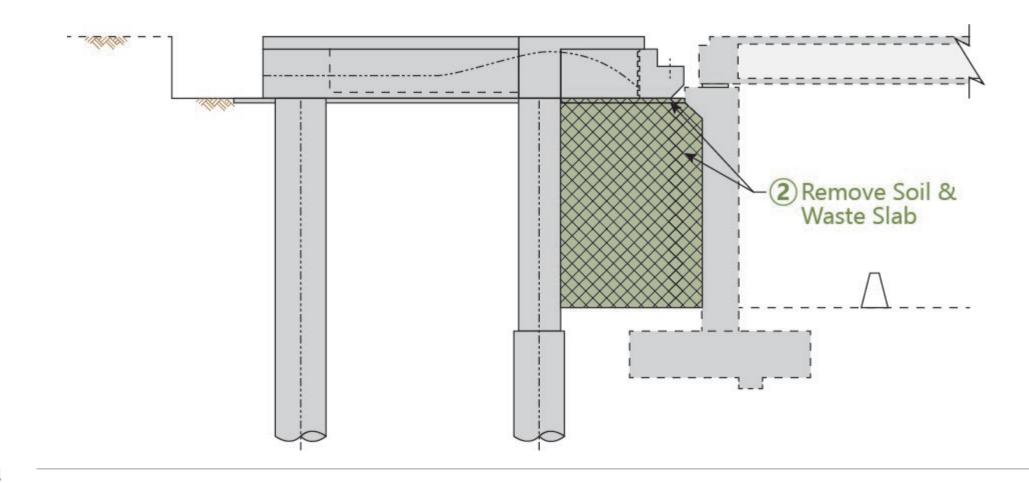


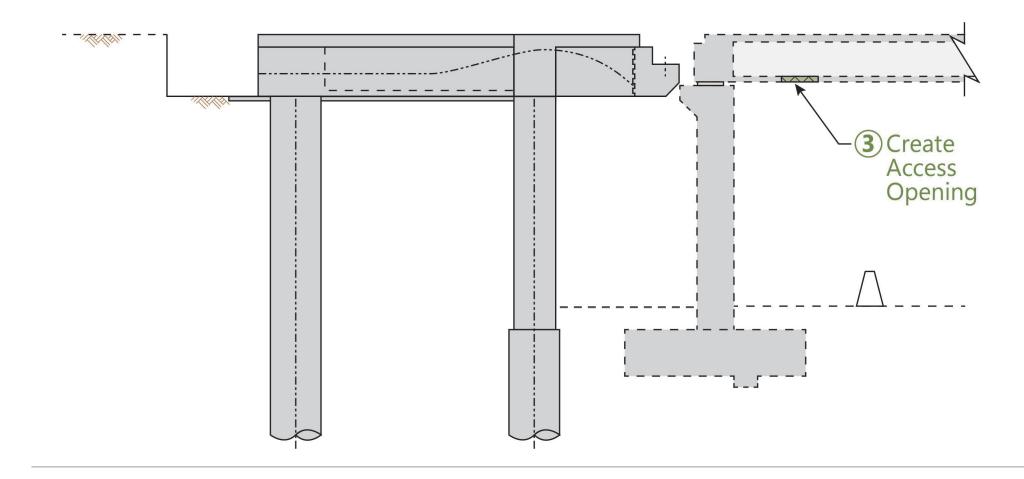




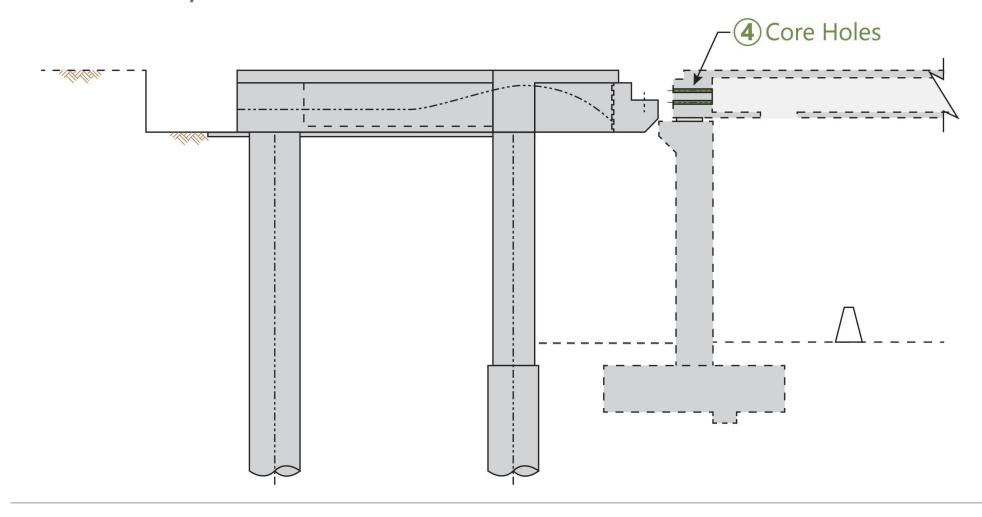


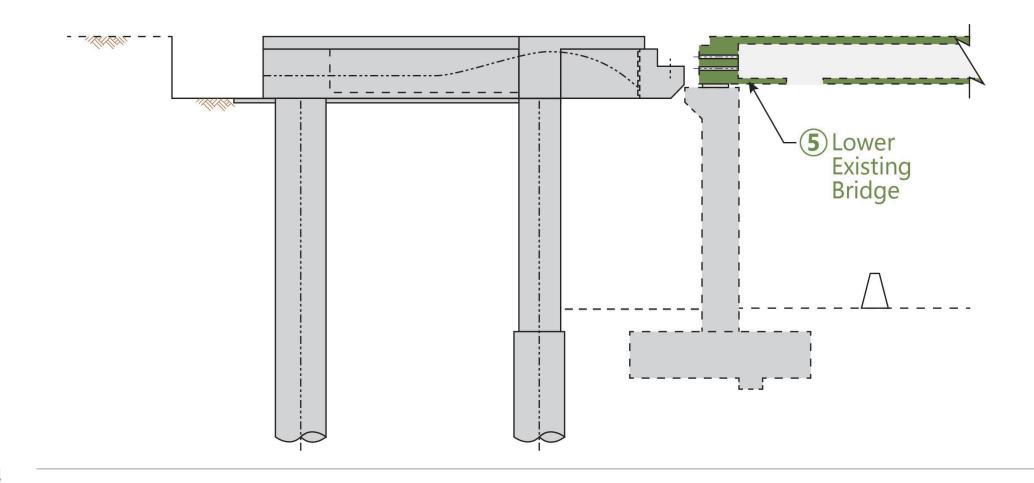


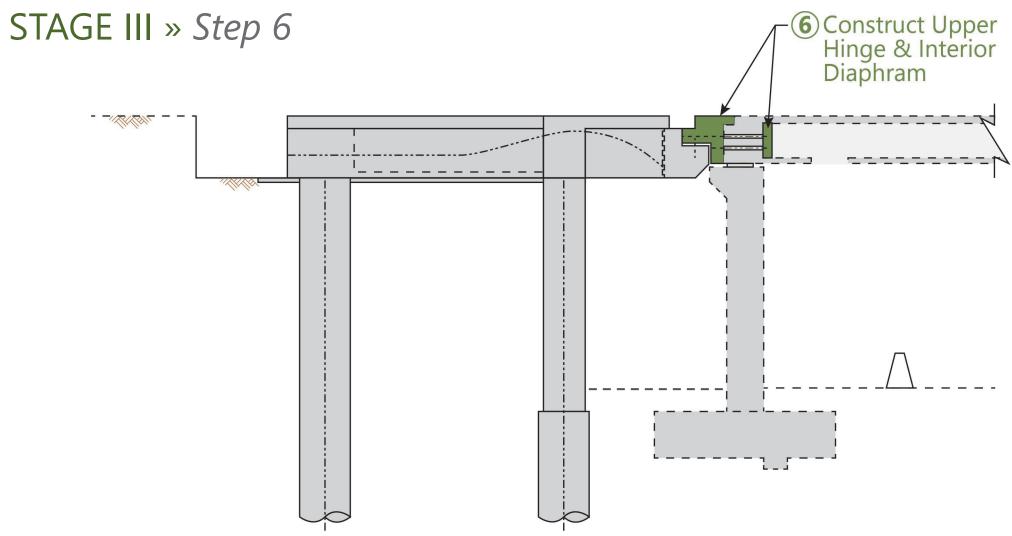


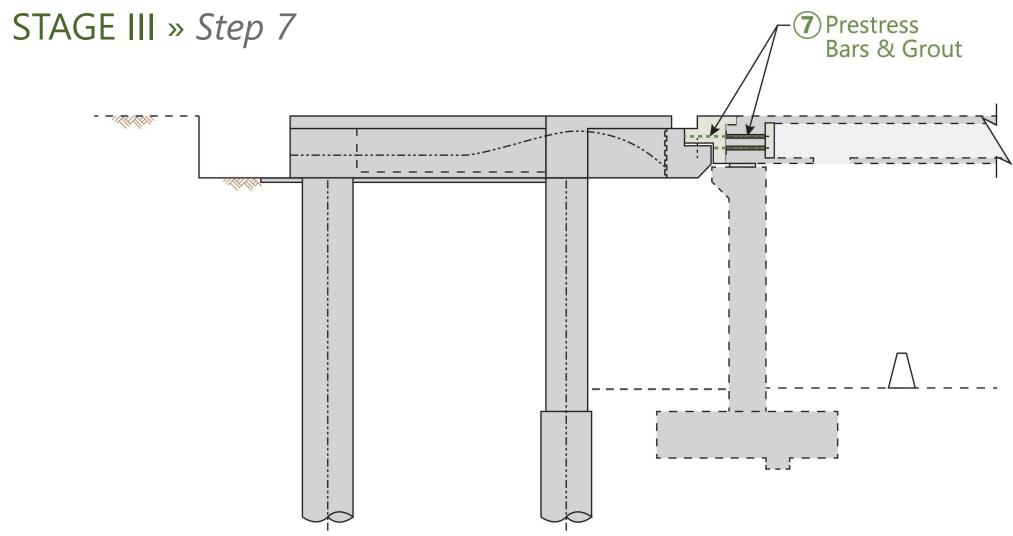


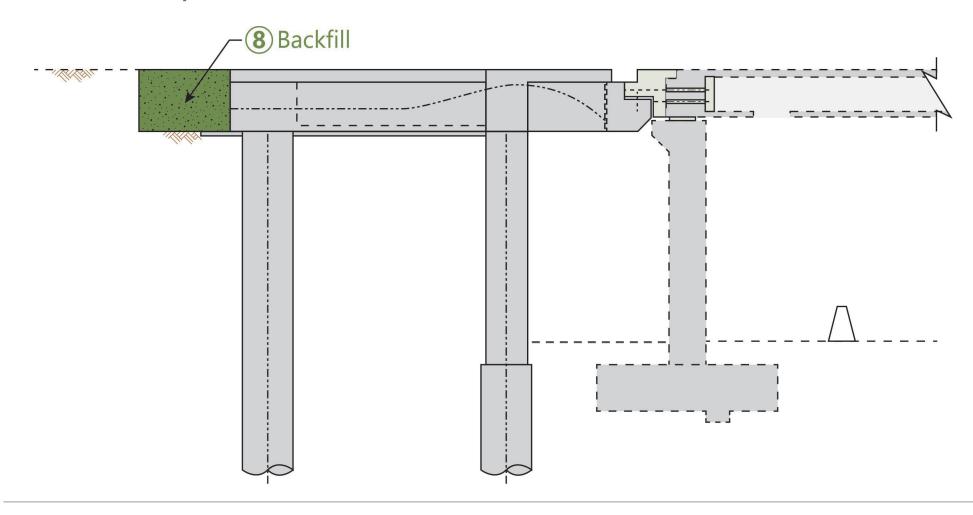


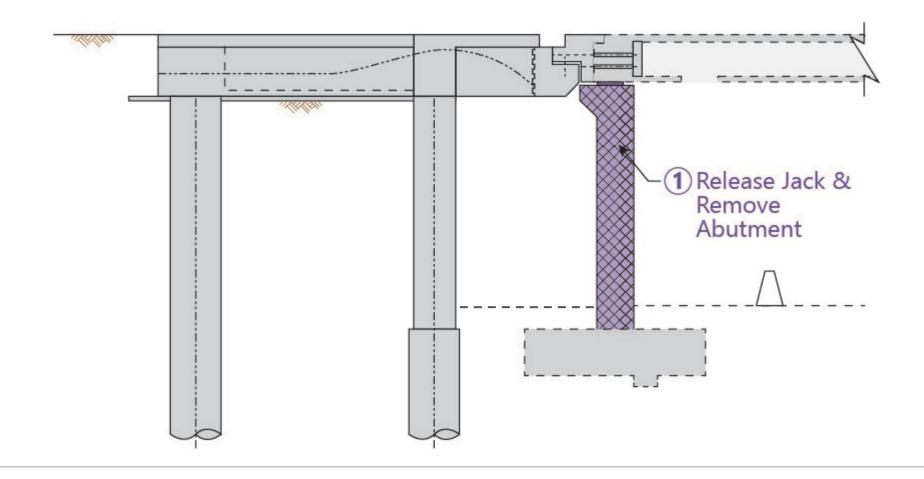


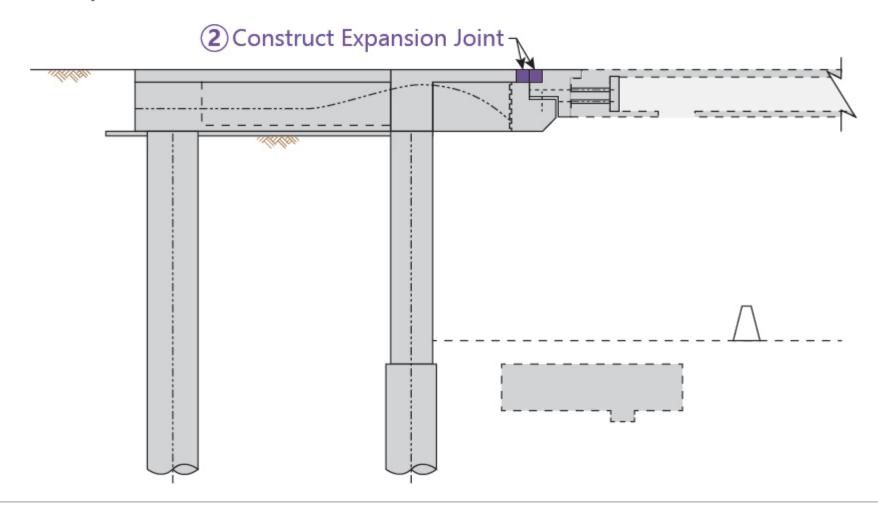


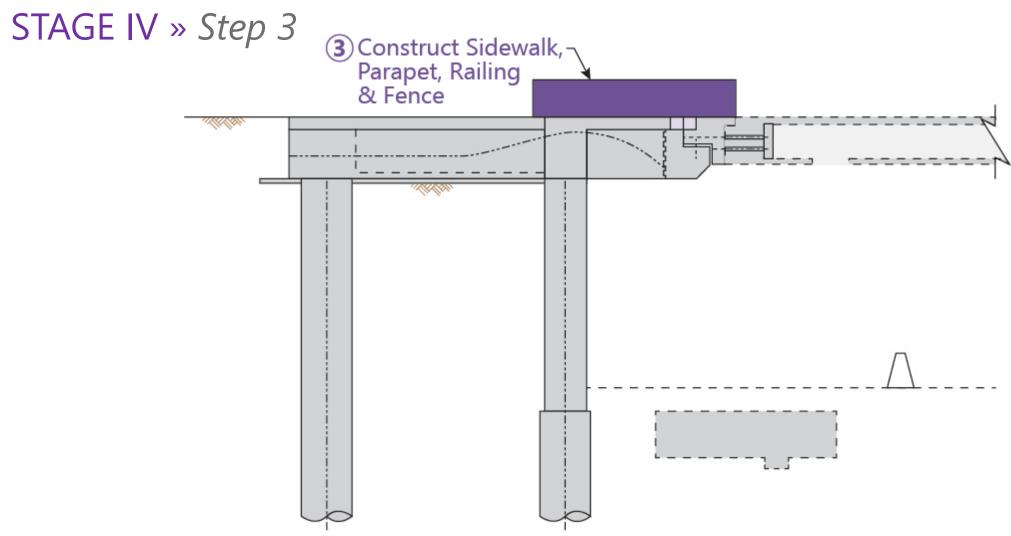


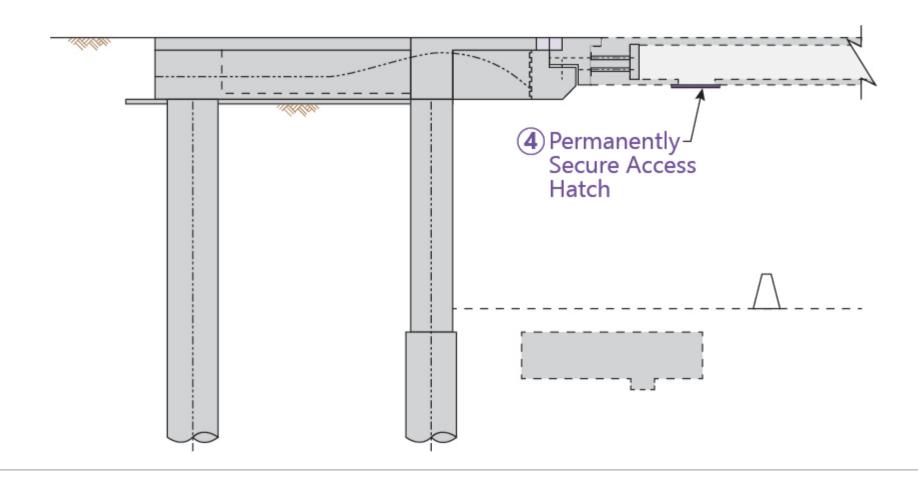












- -FHWA Field Inspection
- Request to Review Plans and Calculations
- Questioned Lack of Strut-and-Tie Analysis
- Performed Analysis

-RESULT:

- No changes to any rebar size
- No changes to any location
- FHWA Questioned lack of Supplemental Reinforcement



5.8.2.5.3a – General

The limiting compressive stress at the node face, fcu, shall be taken as:

fcu = m v f'c

v = concrete efficiency factor:

- 0.45 for structures that do not contain crack control reinforcement
- As shown in Table 5.8.2.5.3a-1

5.8.6.3 – Crack Control Reinforcement
Structures which have been designed using the efficiency factor of Table 5.8.2.5.3a-1 shall contain orthogonal grids of bonded reinforcement.

Av = 0.003* bw*sh

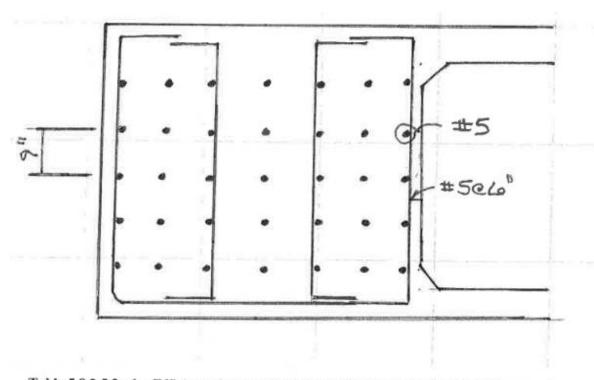


Table 5.8.2.5.3a-1—Efficiency Factors for Nodes with Crack Control Reinforcement

	Node Type		
Face			
	CCC	CCT	CTT
Bearing Face			
Back Face	0.85	0.70	
	$0.85 - \frac{f_c'}{20 \text{ ksi}}$	$0.85 - \frac{f_c'}{20 \text{ ksi}}$	$0.85 - \frac{f_c'}{20 \text{ ksi}}$
Strut-to-Node Interface	$0.45 \le \nu \le 0.65$	$0.45 \le v \le 0.65$	$0.45 \le v \le 0.65$

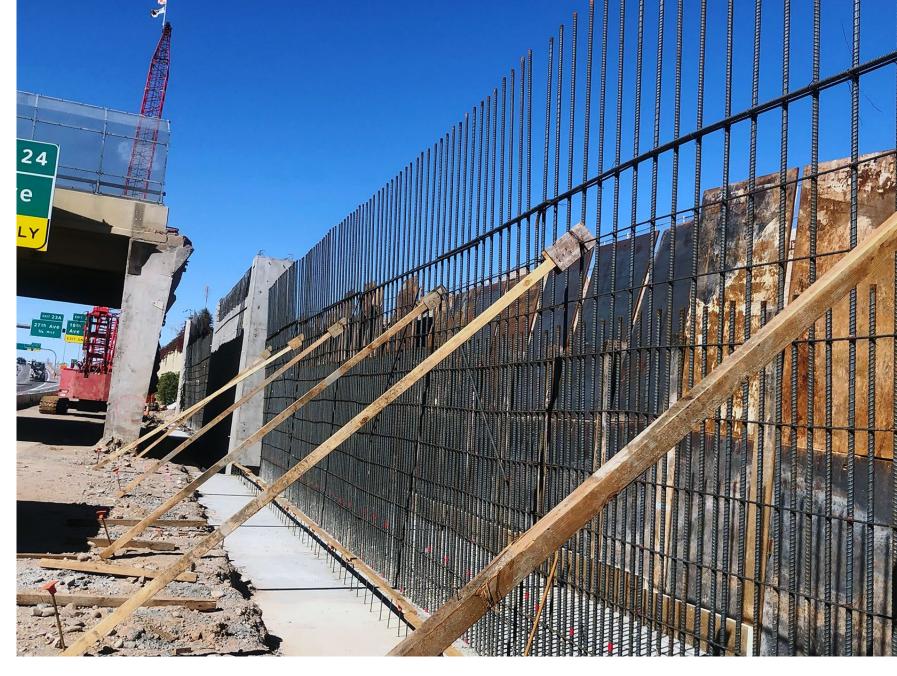
- ADOT Policy Status
- -Proposal to Study When/Where Analysis Method Used



SHORING & APPROACH WALLS

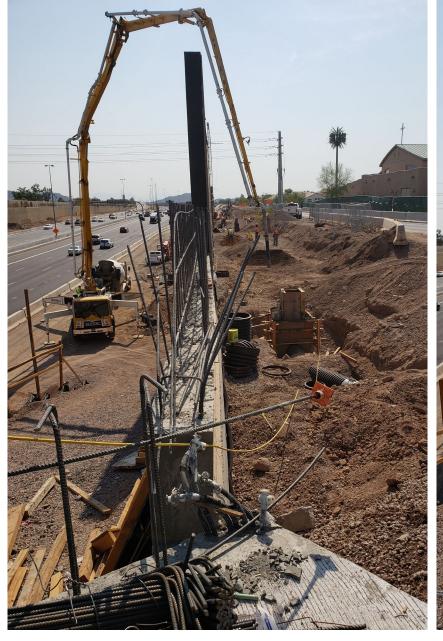


RETAINING WALL



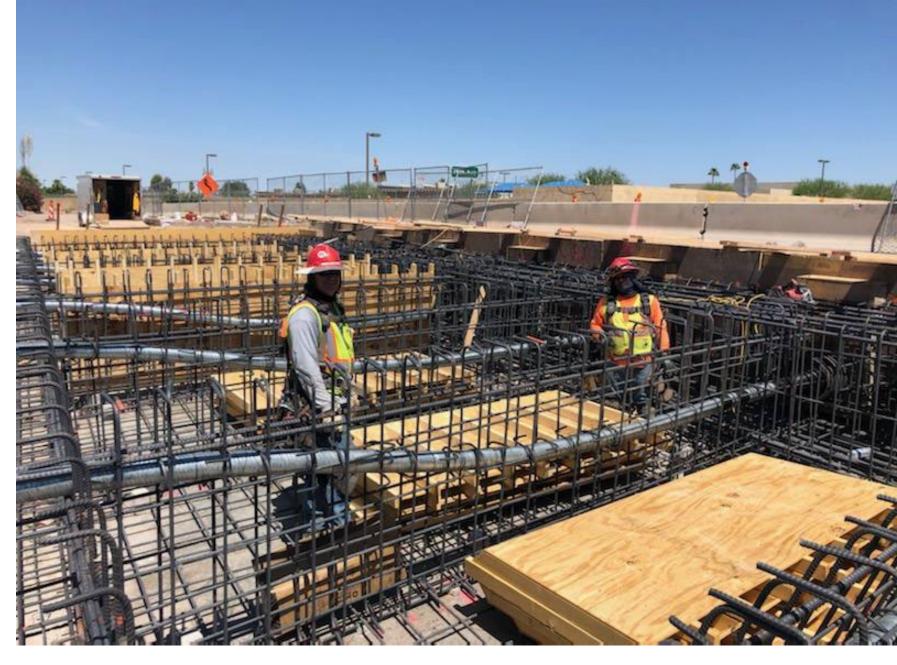


WALLS

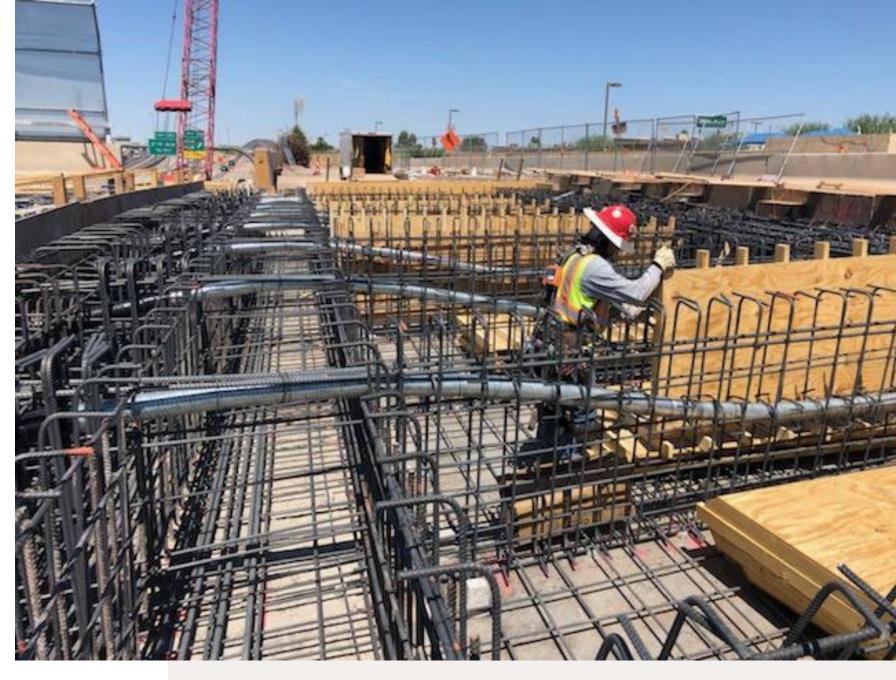




BACK SPAN



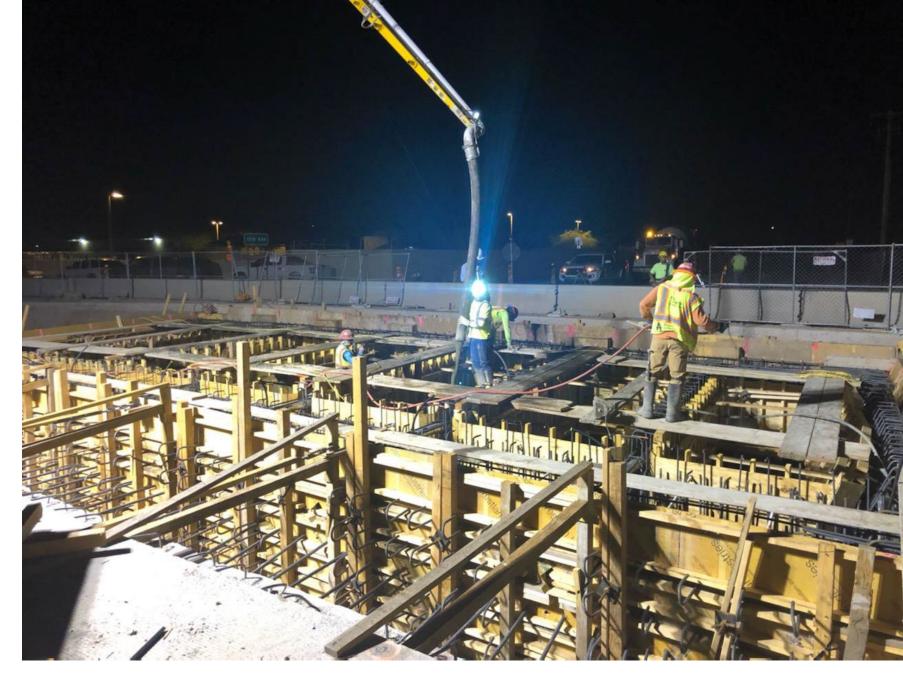
WEB FORMS



END SPAN
ABUTMENT
CAP
ROOM TO
PULL

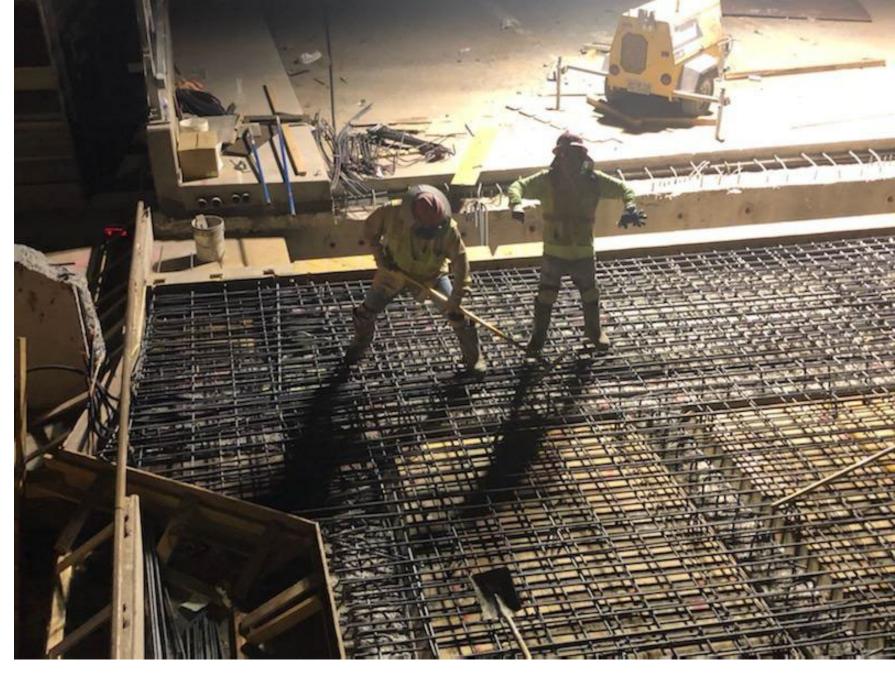


BOTOM SLAB & WEB POUR



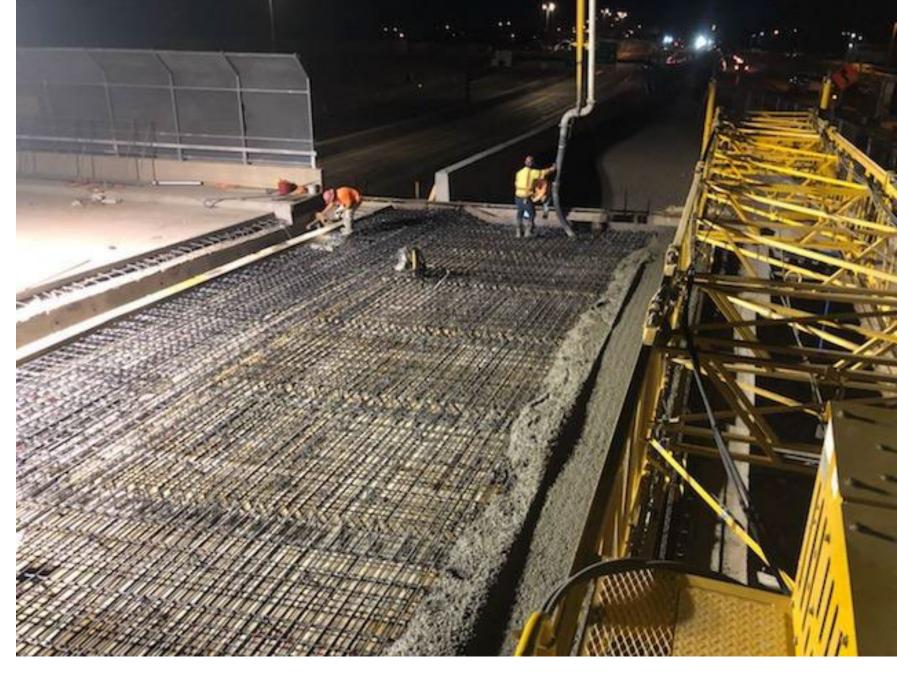


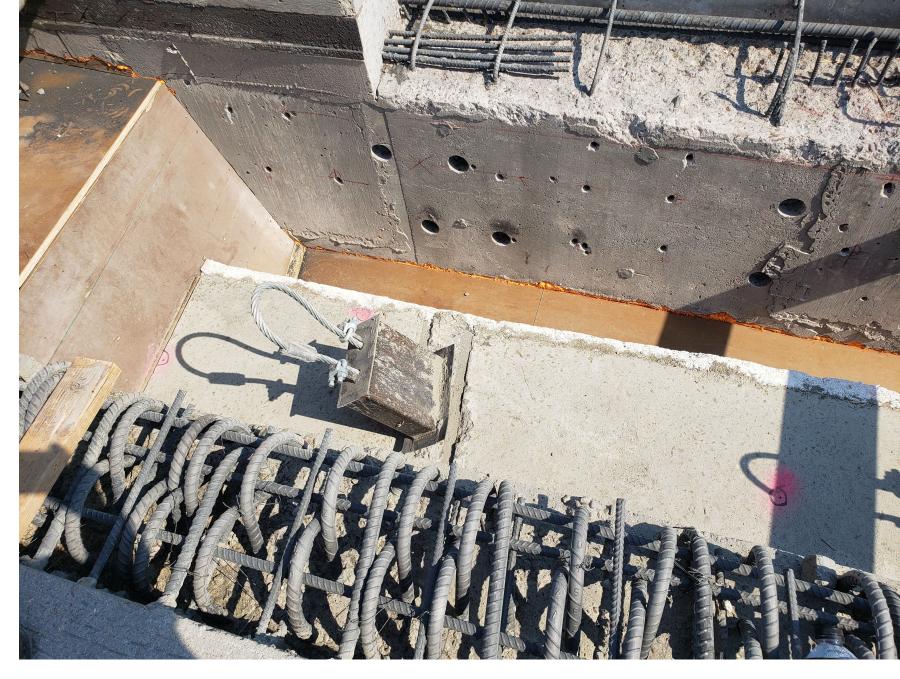
DECK REINFORCING



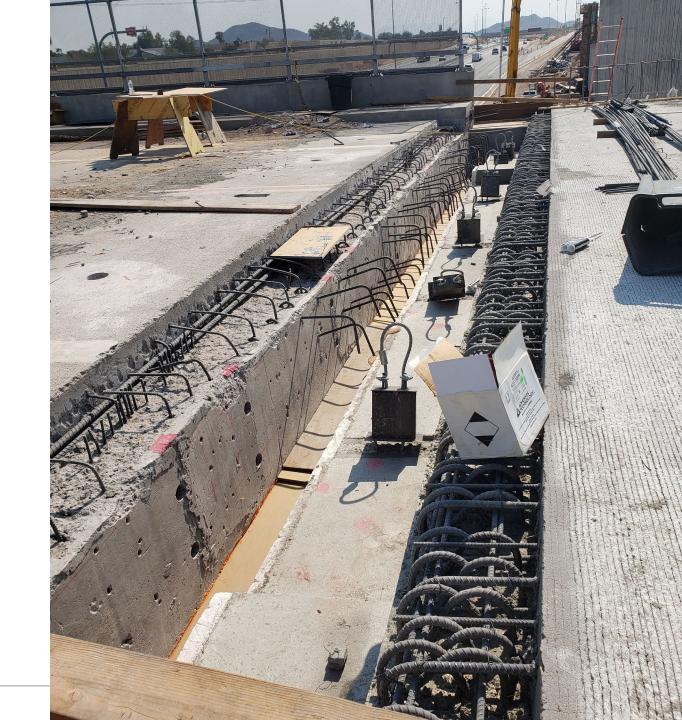


DECK POUR

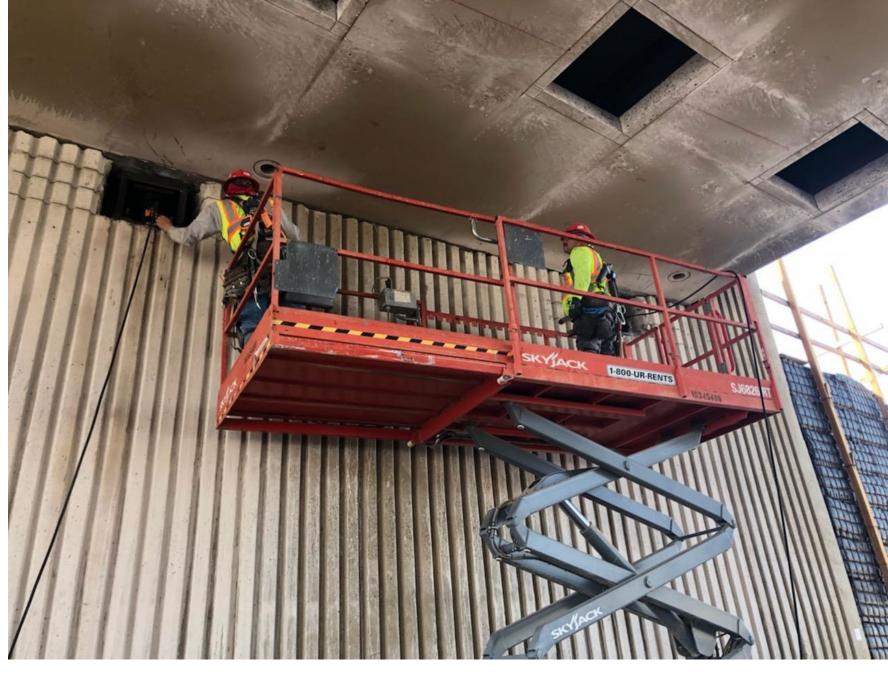




HINGE



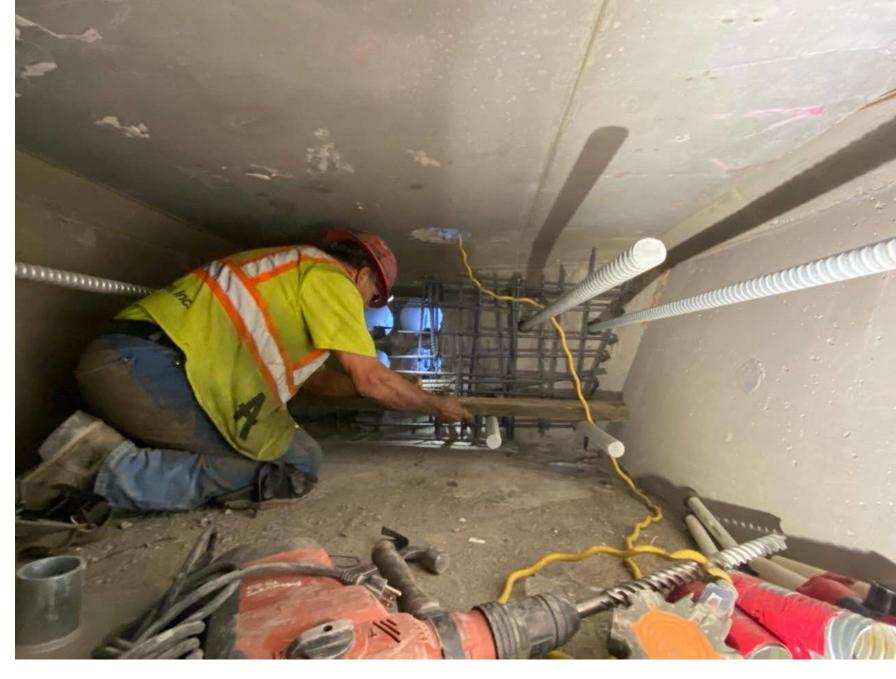
ACCESS HOLES
JACKING



JACKING
ANALYSIS
EXISTING
ABUTMENT CAP



EXISTING CELL

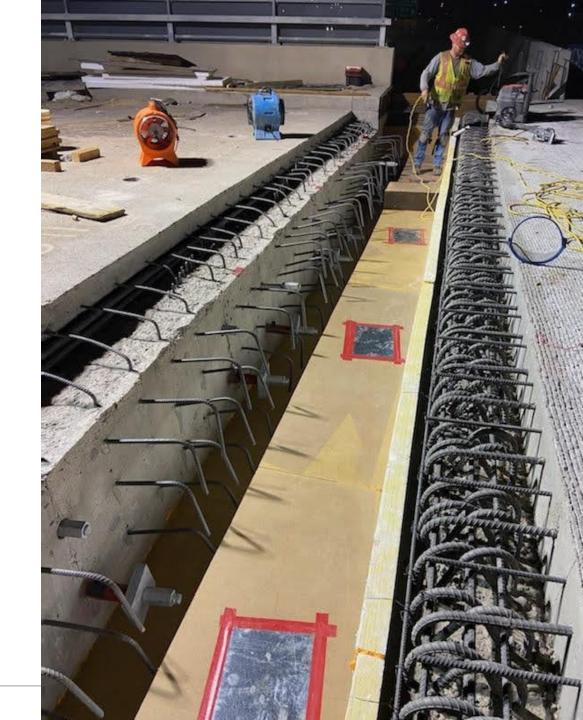


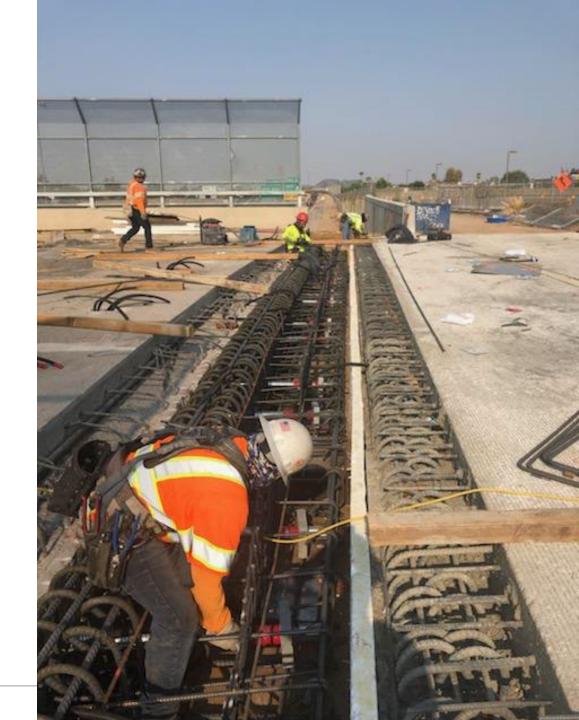
INSIDE CELL



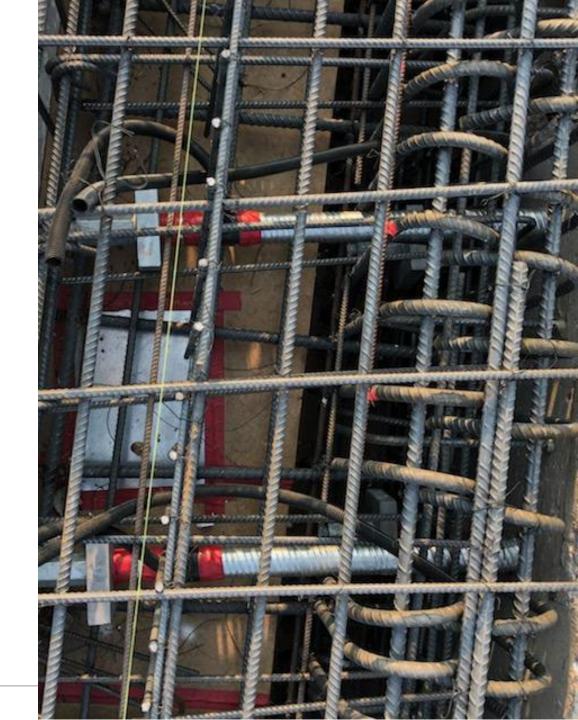


HINGE DETAILS





P/S BAR DUCTS



PRESTRESS JACKING



ABUTMENT REMOVAL



LOOKING NORTH



EXPANSION JOINT



LOOKING SOUTH



LOOKING SOUTH



TYLin

2023 Western Bridge Engineer's Seminar

Questions?

