

# Evaluation of Gusset Plate Connections in Steel Truss Bridges

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1 | 06 SEPTEMBER 2017  
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# Presentation Outline

- > Introduction
- > Motivation
- > Scope
- > Existing Methods
- > Study Phases
  - > Development of analytical mode for gusset plate joint
  - > Approximate stress distributions at the critical sections on gusset plates
  - > Procedure to combine and estimate equivalent stress
  - > Application of calculating stresses on gusset plate connections
- > Conclusions

# Introduction

- > Gusset Plate typically connect diagonals, hanger, and chord in steel truss bridges
- > Variety of geometric configurations and load distributions make simple analysis difficult
- > The 2007 I-35W Bridge collapse necessitates load rating and prompt the research on gusset plate joints



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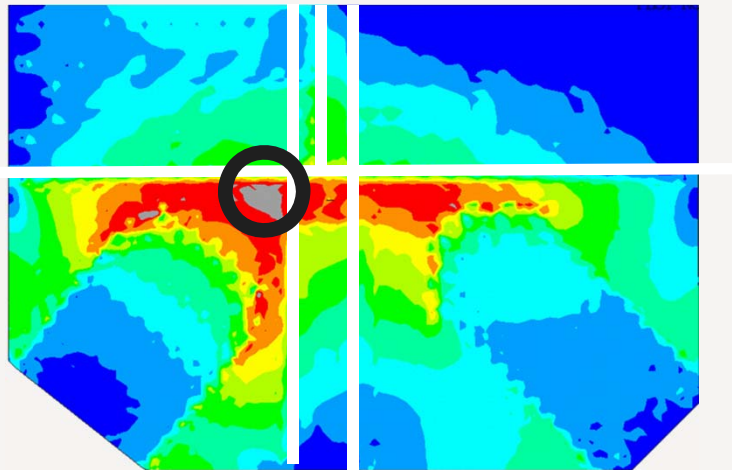
# Motivation

- > The I-35W Bridge collapsed on August 1<sup>st</sup>, 2007
- > Gusset plates U10 and L11 were investigated, and it was found that they had half the required thickness
- > Gusset plate U10 was overstressed and then buckled to cause joint failure



# Presentation Scope

- > Gusset Plate Elastic Stress Distribution
  - > Approximation of stress distribution on the critical sections
  - > Procedure to identify the maximum stress and location on gusset plate connection
  - > Application of this approach

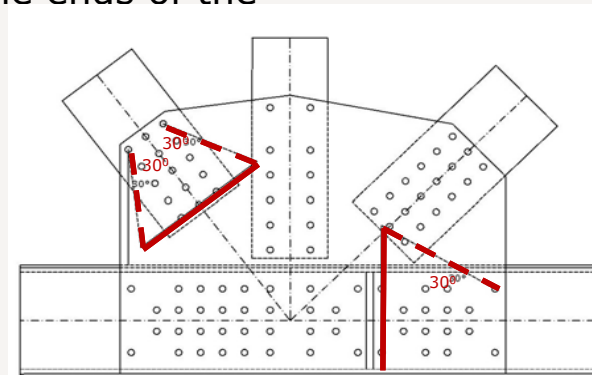
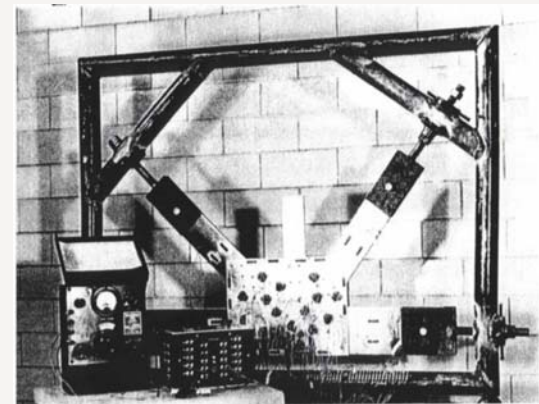


## Existing Methods

- > Only simple approximation is available to estimate gusset plate stress or capacity
  - > The Whitmore Method (maximum stress)
  - > FHWA Guidance
  - > Beam Theory (maximum principal stress)

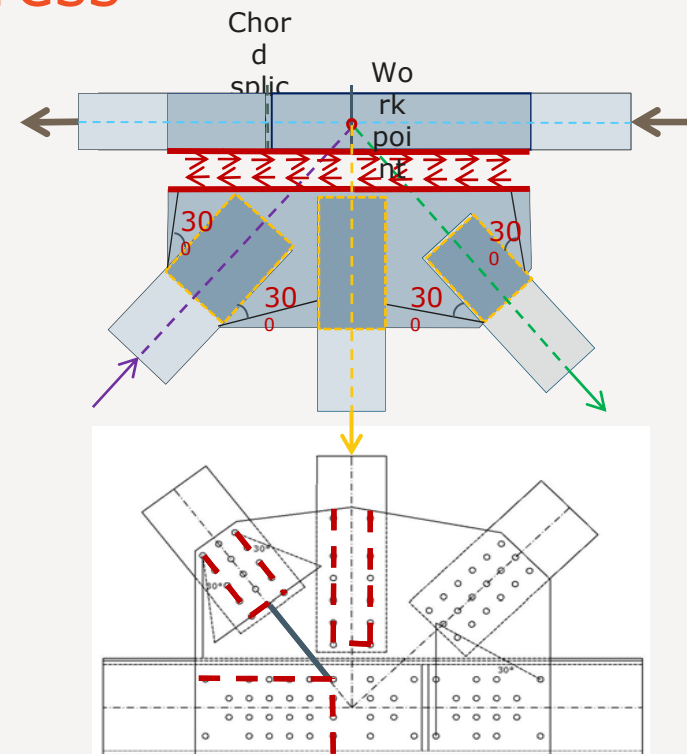
# Literature Review for Elastic Stress

- > Whitmore (1952)
  - > Warren type gusset plate connection was tested
  - > High strength aluminum
  - > Hanger was neglected and **chord was continuous**
  - > **Rectangular bars** were used as truss member
  - > 30° dispersion angle
  - > Maximum principal stresses located at the ends of the diagonals
  - > Elastic behavior
- > Application today (AASHTO, FHWA):
  - > Gross section yield
  - > Net section fracture



# Literature Review for Elastic Stress

- > **FHWA: Guidance and Examples for Bolted and Riveted Gusset Plate in Truss Bridge (2009) and AASHTO MBE (2011)**
  - > Gusset plate shear resistance
    - > Uniform stress  $\Omega=1.0$
    - > Parabolic stress  $\Omega=0.74$
  - > Tensile member
    - > Block shear
    - > Gross yielding, Net section fracture, and using Whitmore's method





# Literature Review for Elastic Stress

## > Beam Theory (Section Method)

Stress along section

$$f_a = \frac{P}{A}$$

$$f_b = \frac{M}{S}$$

$$f_{vave} = \frac{V}{A}$$

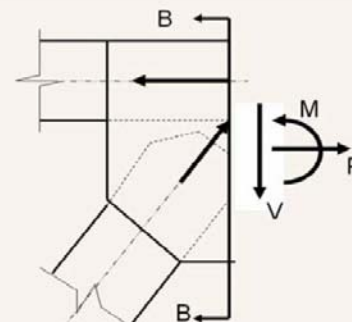
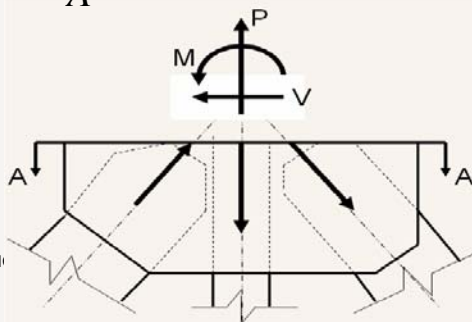
$$f_v = 1.5 \frac{V}{A}$$

Principal stress at points of section are

$$R = \sqrt{\left(\frac{f_a + f_b}{2}\right)^2 + f_v^2}$$

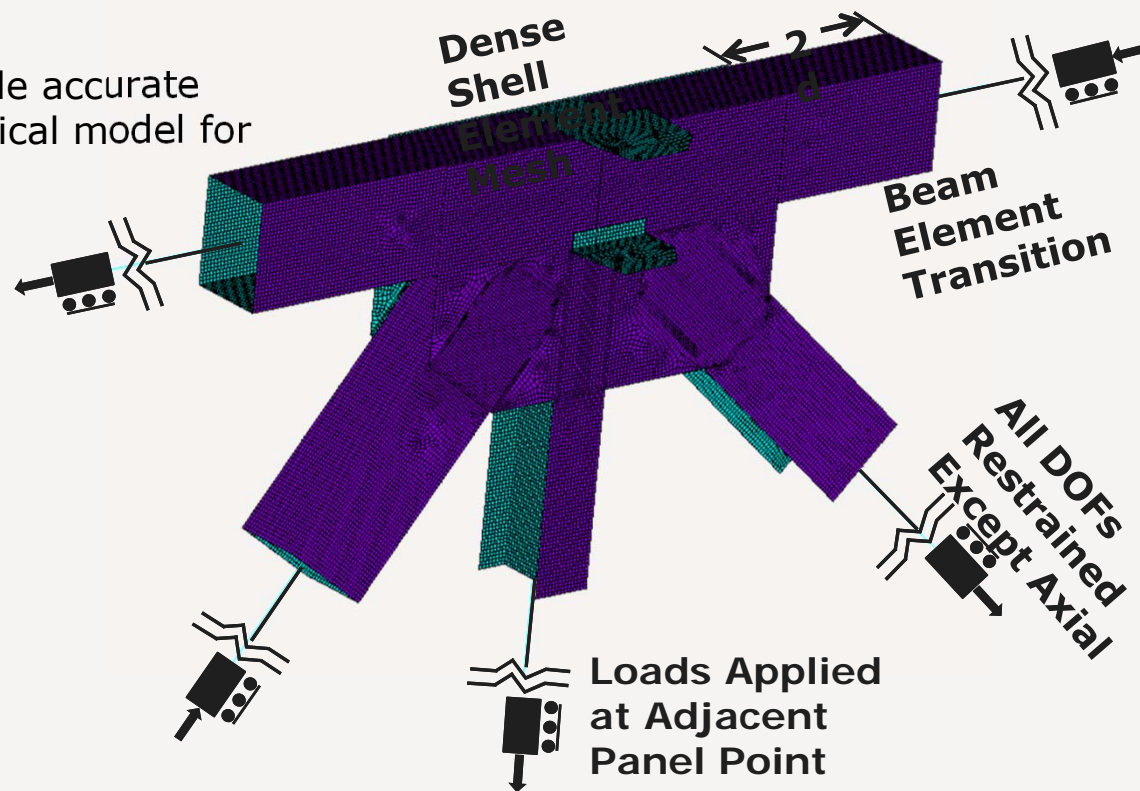
$$f_{tension} = \frac{f_a + f_b}{2} - R$$

$$f_{compression} = \frac{f_a + f_b}{2} + R$$



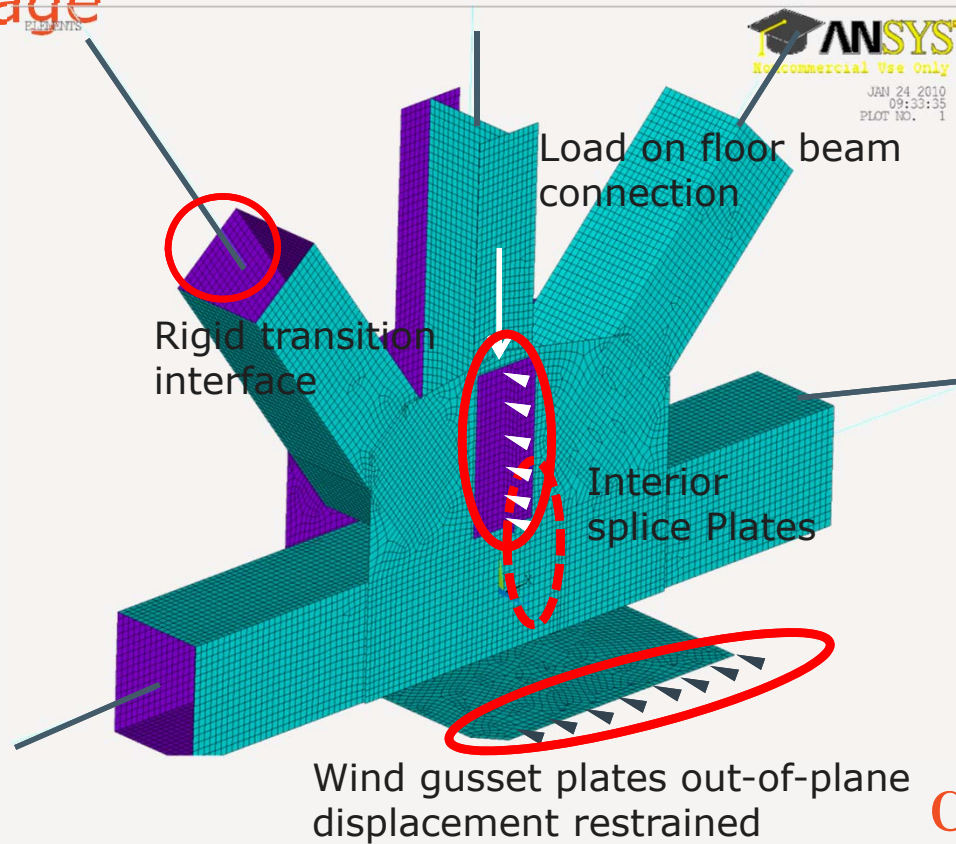
# FE Modeling of Joint Subassemblage

- > Development of reliable accurate and easily-built analytical model for parametric study



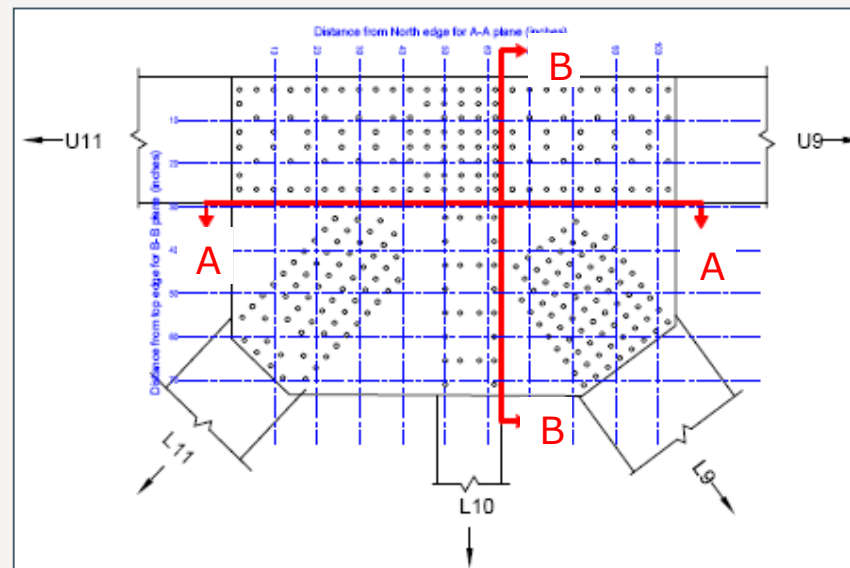
# FE Joint Subassemblage

- > Rigid transition interface
- > Floor beam connection
- > Wind gusset plate
- > Chord splice plates
- > Rivet modeling
- > 0.5 in element edge size



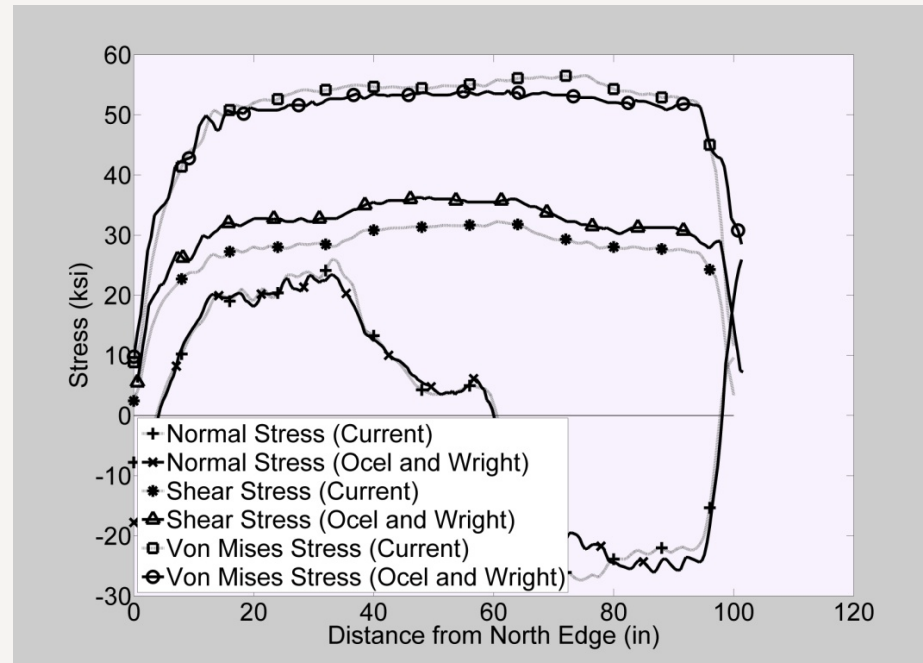
## Comparison with FHWA Results

- > Examined shear, normal, and Von Mises stresses along A-A and B-B sections and compare with Ocel and Wrights results (FHWA investigation report)



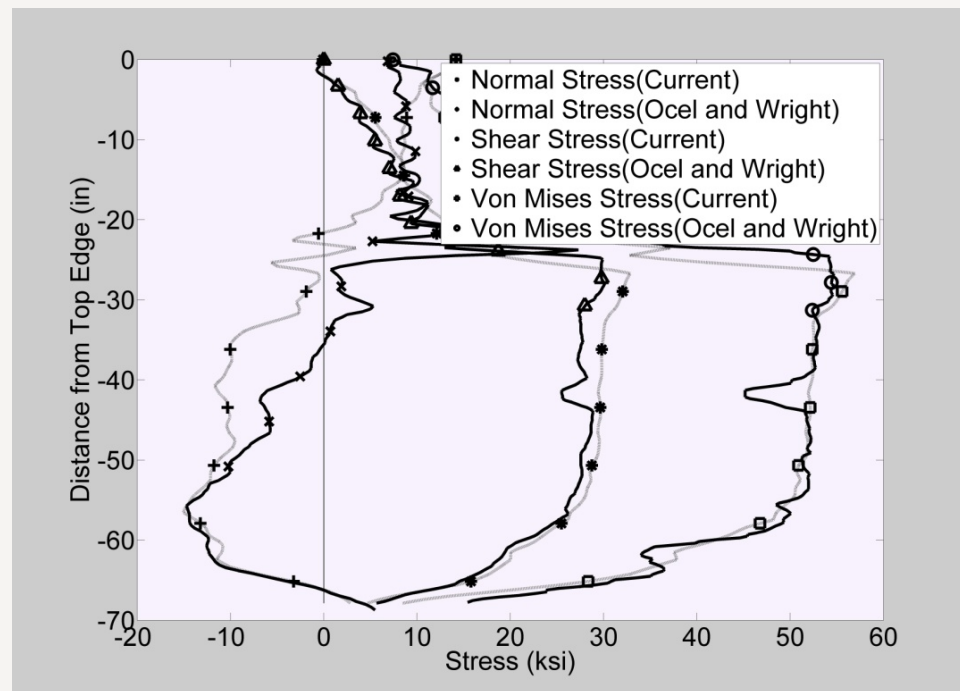
# Comparison with FHWA Results

> Along A-A section

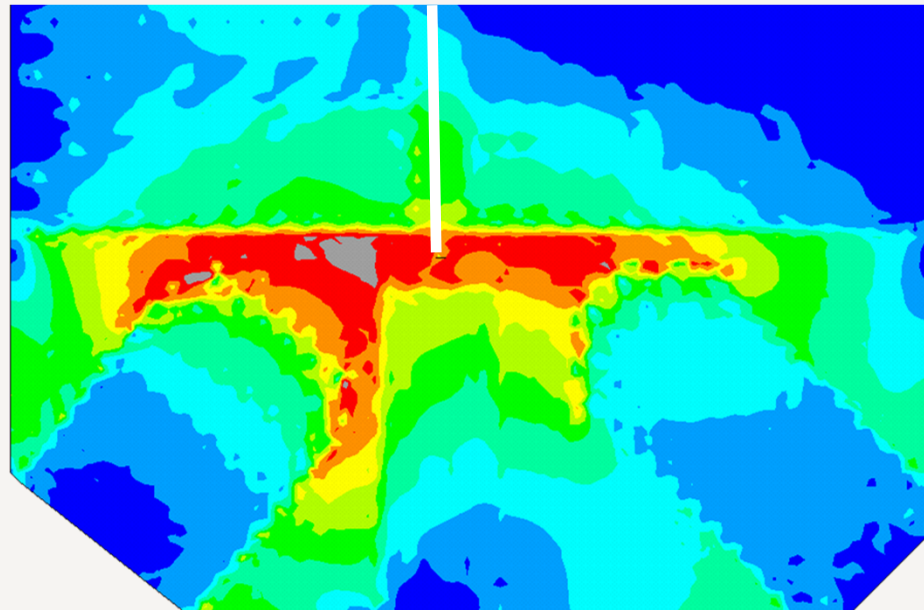


## Comparison with FHWA Results

> Along B-B section

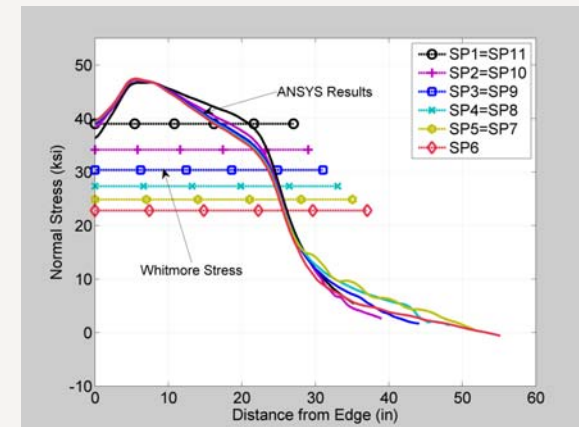
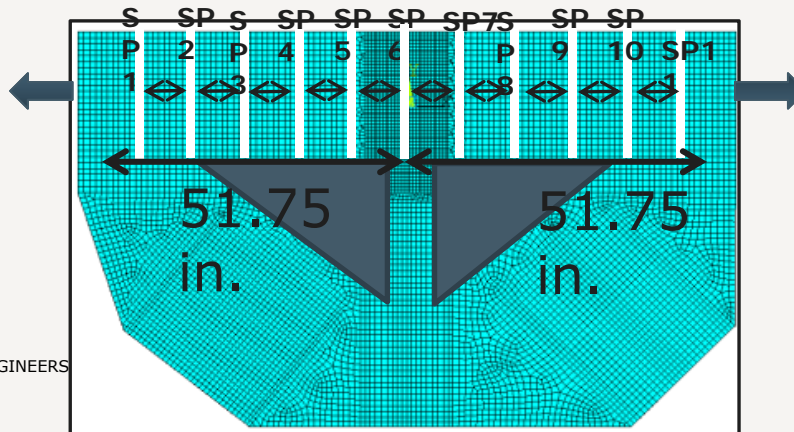


## Critical Section: Chord Splices



# Study of Stresses at the Chord Splice

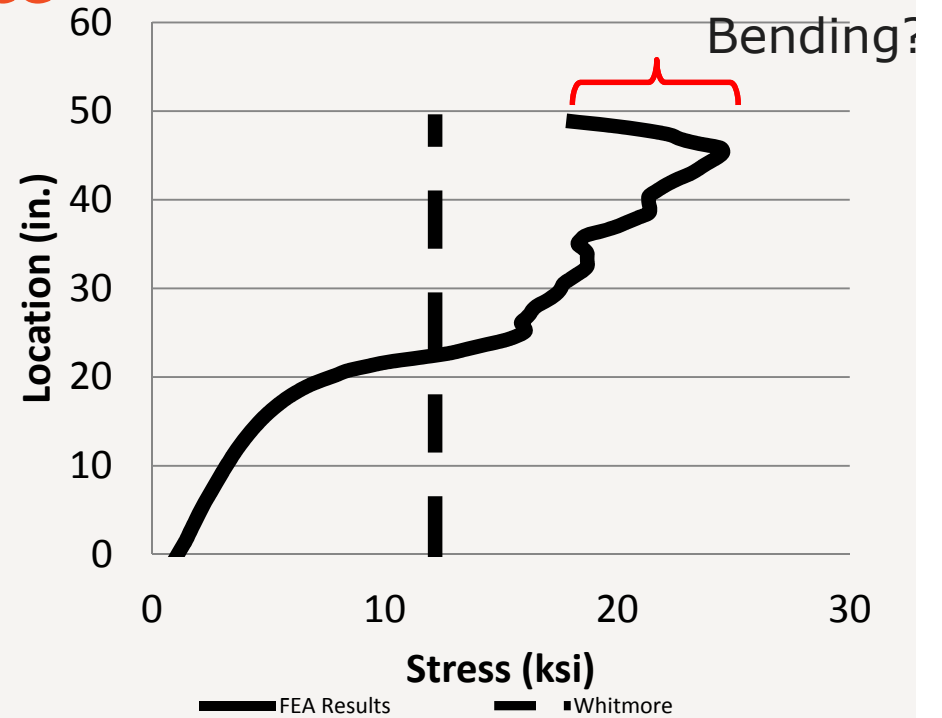
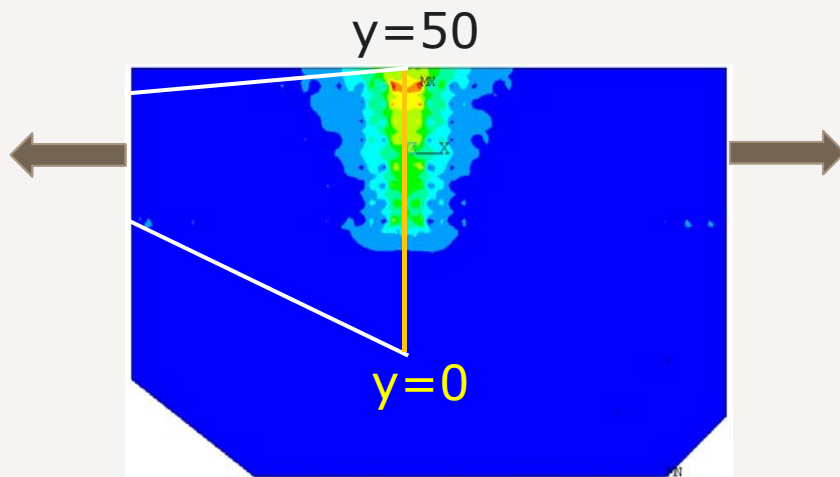
- > Lack of verification for the Whitmore's method applied to the chord splice
- > Parameters
  - > Only chords are loaded with equal and opposite forces
  - > Various splice locations considered (7.5 in increments)
- > The Whitmore method underestimates the maximum stress
- > FEA results shows the stress distribution does not vary with splice location





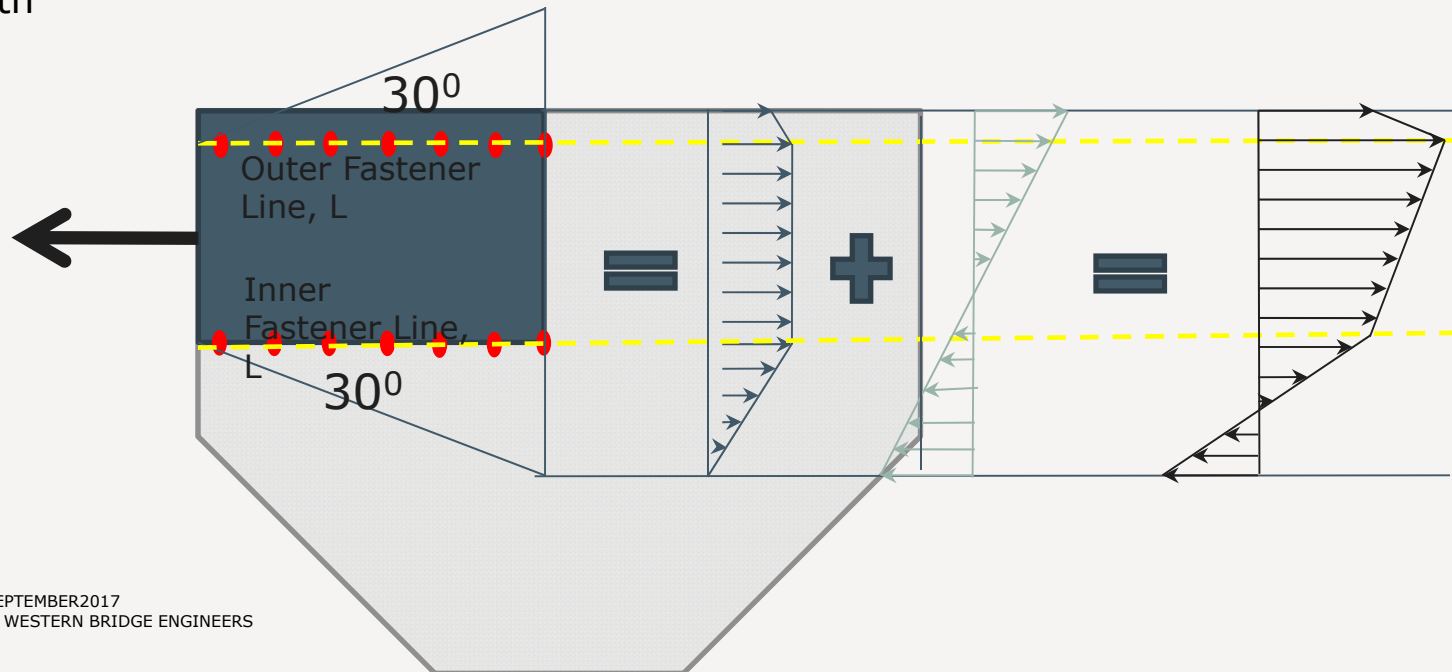
# Normal Stress at Chord Splice

- > Anti-symmetric geometry



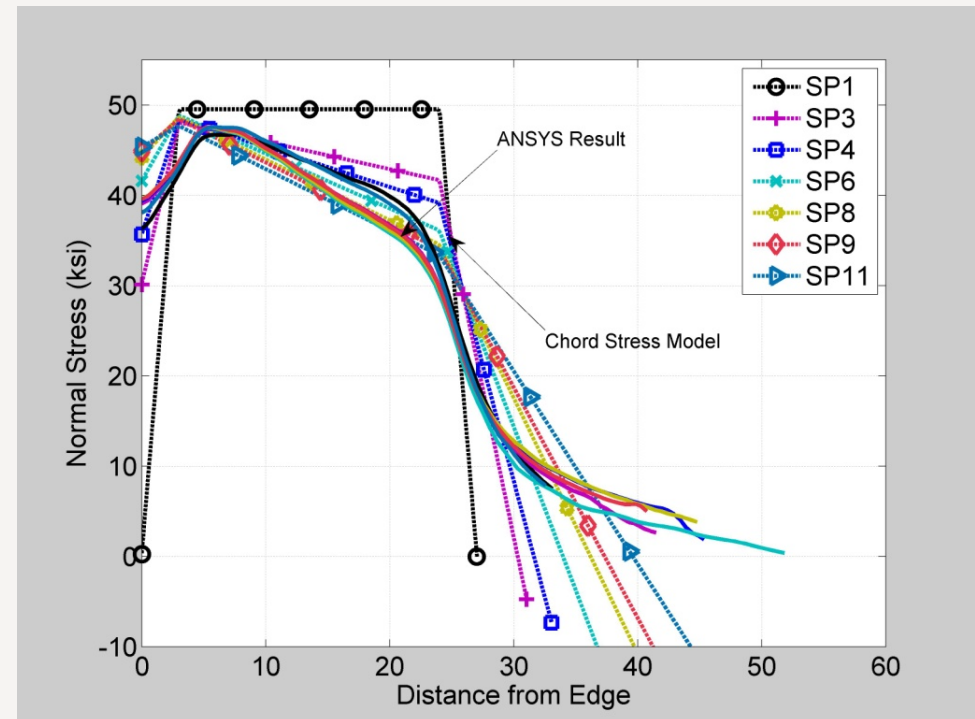
## Idealized Approximate Stress Model

- > Based on FEA results: Trapezoidal stress distribution + bending effect is used to describe the behavior at the end of chord instead of just using uniform stress over the Whitmore width



# Verification of the Trapezoidal Stress Distribution Model

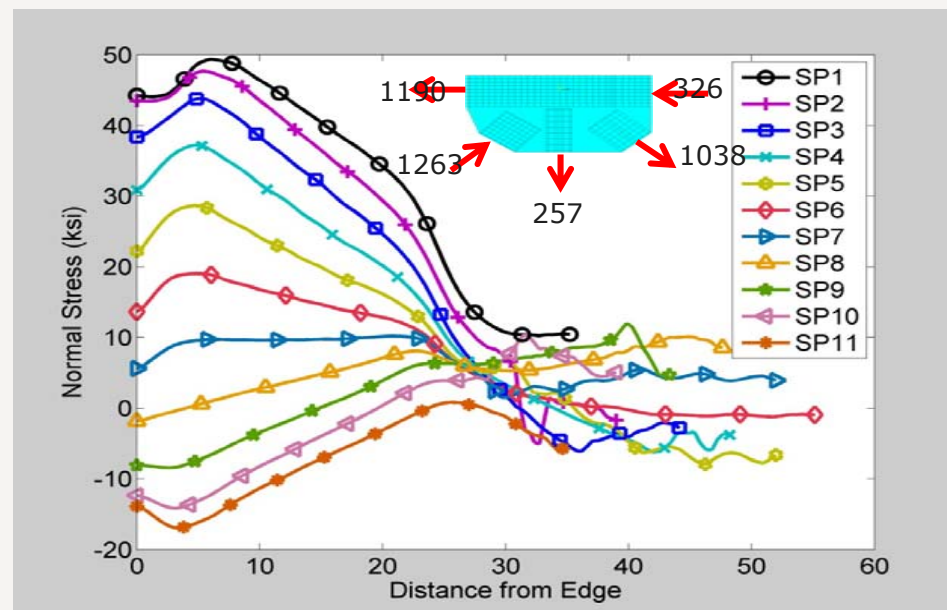
- > Parameters Studied:
  - > Gusset plate thickness
  - > Deformable fastener
  - > Web and flange splice plates
  - > Ratio chord connection length to connection width (L/W)
- > Conclusion:
  - > Works well for only chords loaded



# Stress Distribution at the Chord Splice with Full Loads

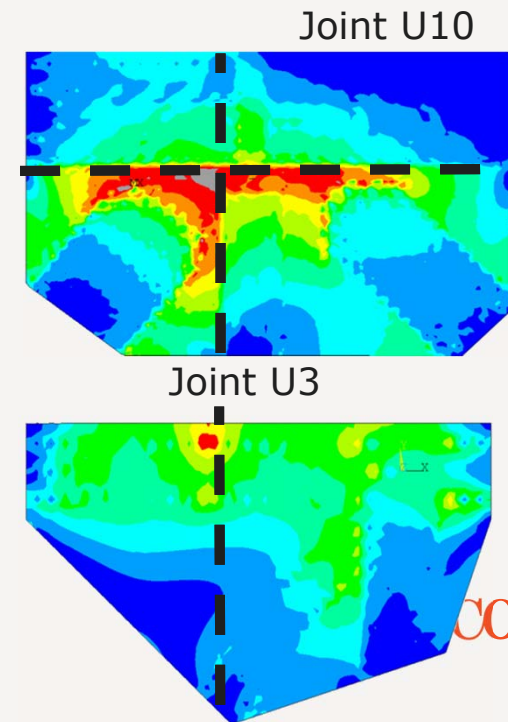
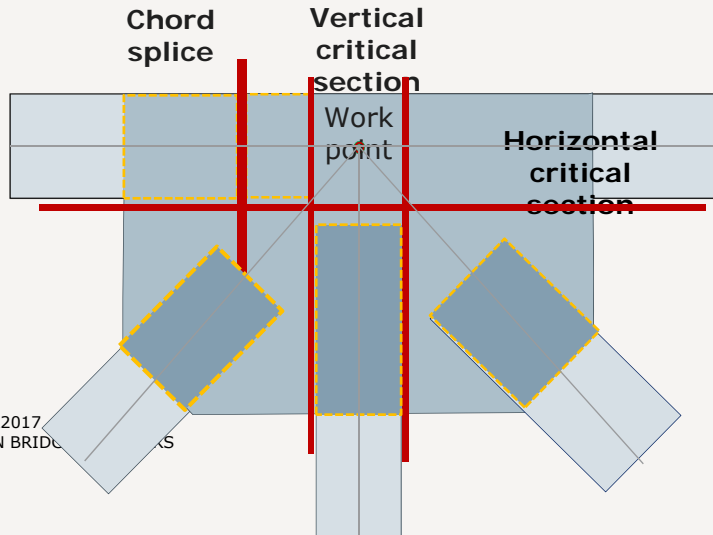
- Stress at chord splice affected other forces

**Need to consider stresses due to loads in other truss members**



# Identification of the Critical Section

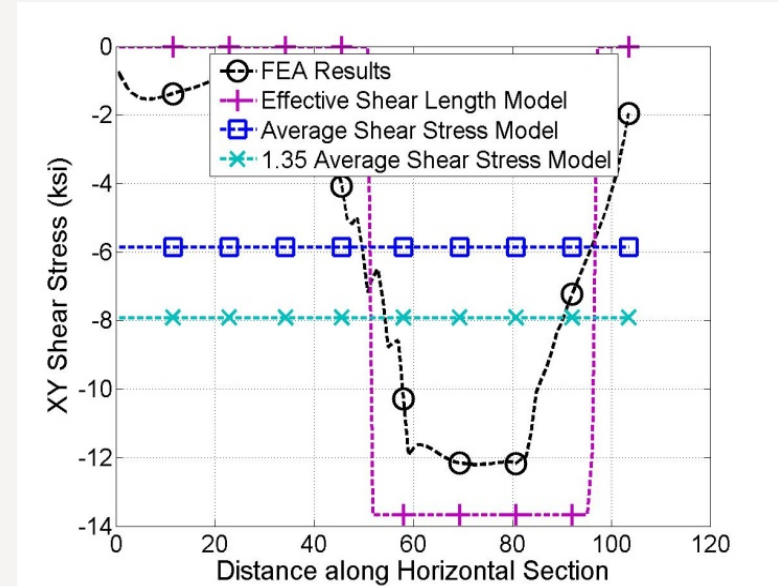
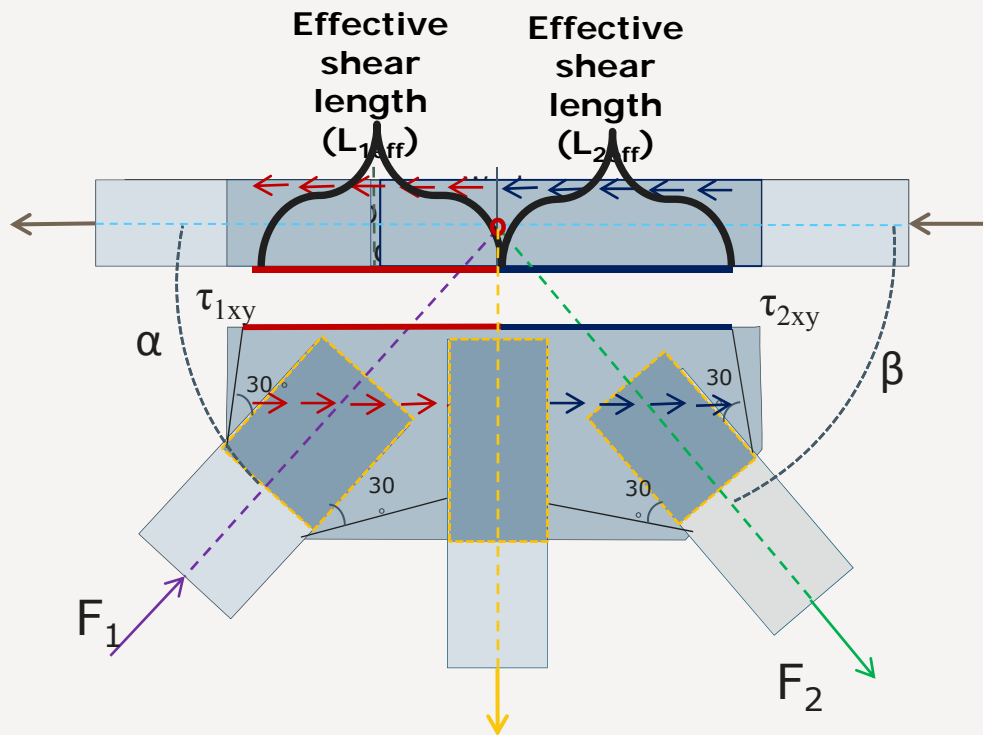
- > Easiest to consider effect of all elements by looking at stress distribution on critical sections
- > Critical sections are found at
  - > Horizontal section between chord and diagonals
  - > Vertical section between hanger and diagonals
  - > Vertical section at chord splice



## Developing Approximate Stress Distribution at Critical Sections

- > Develop loading profiles that highlight one truss member's impact on the stress components at each critical section
- > Observe the stress distributions at the critical sections from FEA of U10 for the different loading profiles and chord splice locations
- > Develop approximate distributions based on those observations
- > Quantify how well the approximations matched FEA results using the error percentage and correlation coefficient
- > Combine the stresses using Von Mises at critical locations to determine the maximum gusset plate stress and its location
- > Compare results with FEA results for several gusset plate joints

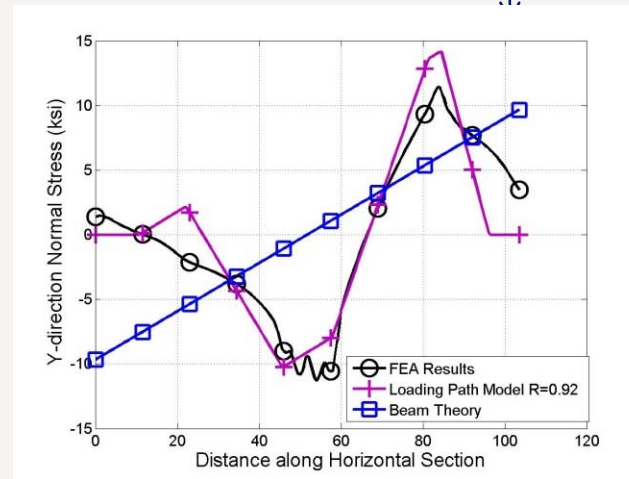
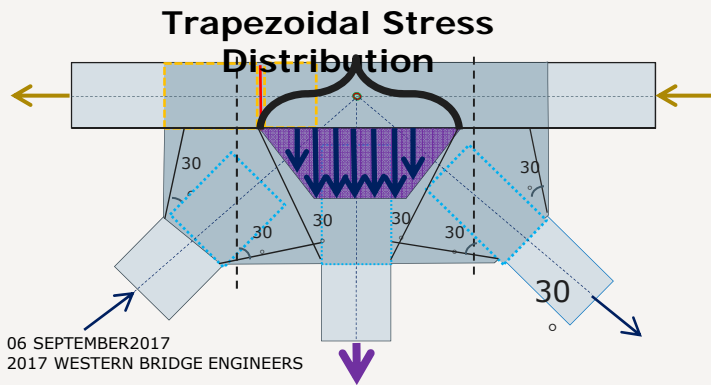
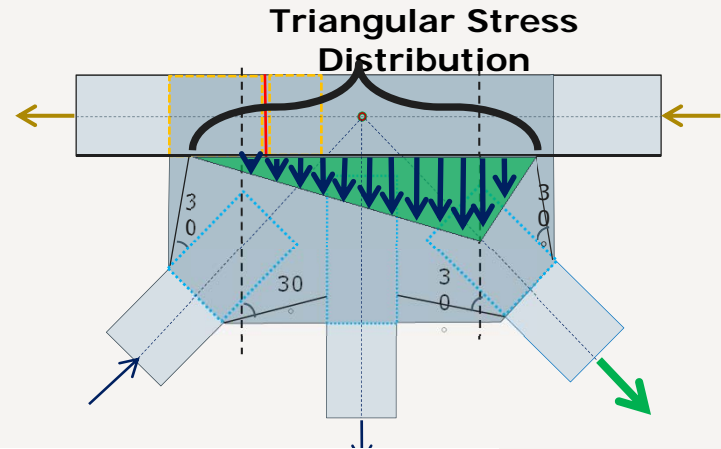
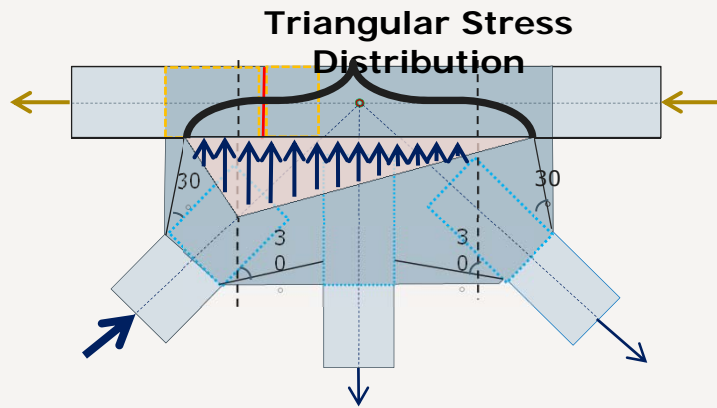
# Stress at Horizontal Section



$$\tau_{1xy} = \frac{F_1 \cos \alpha}{2L_{1eff}t} \quad \tau_{2xy} = \frac{F_2 \cos \beta}{2L_{2eff}t}$$

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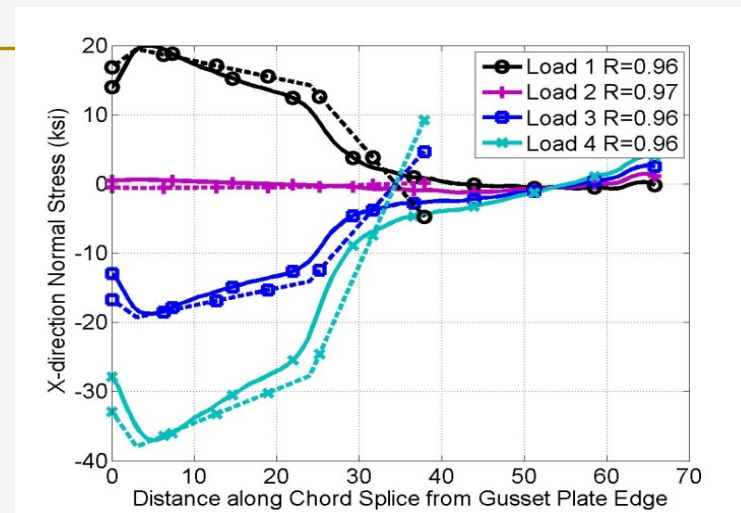
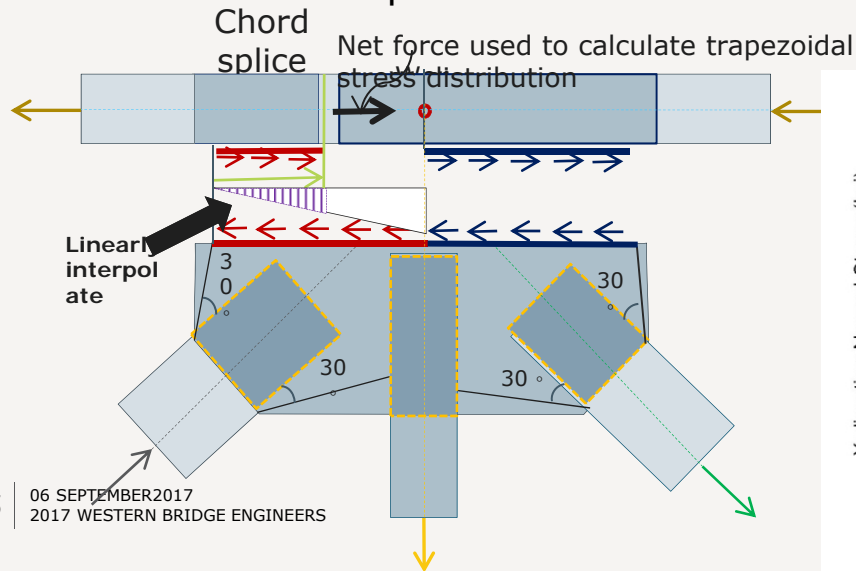
# Stress at Horizontal Section



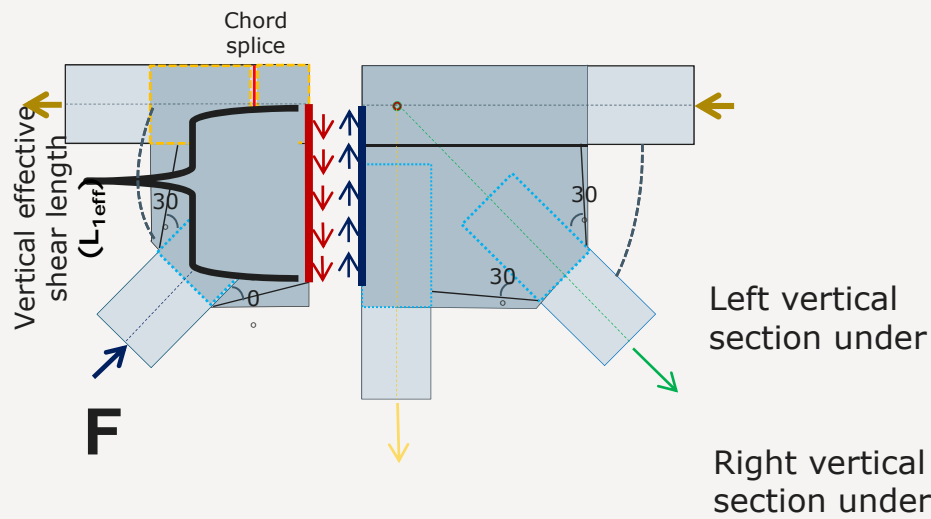


# Stress at Chord Splice

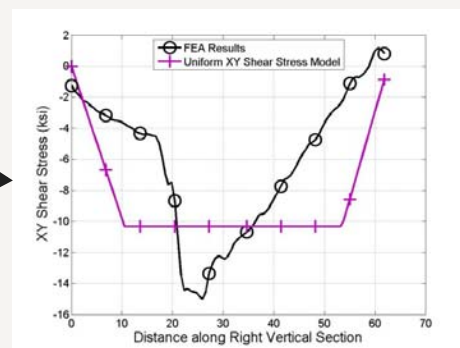
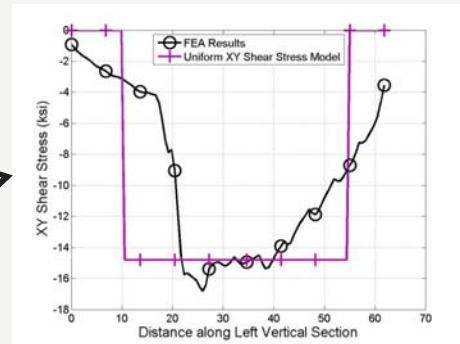
- > Use net force between chord and horizontal component of the diagonal
  - > Linearly interpolate amount of diagonal force based on chord splice location
  - > Based on assumption of constant shear stress



# Stress at Vertical Section

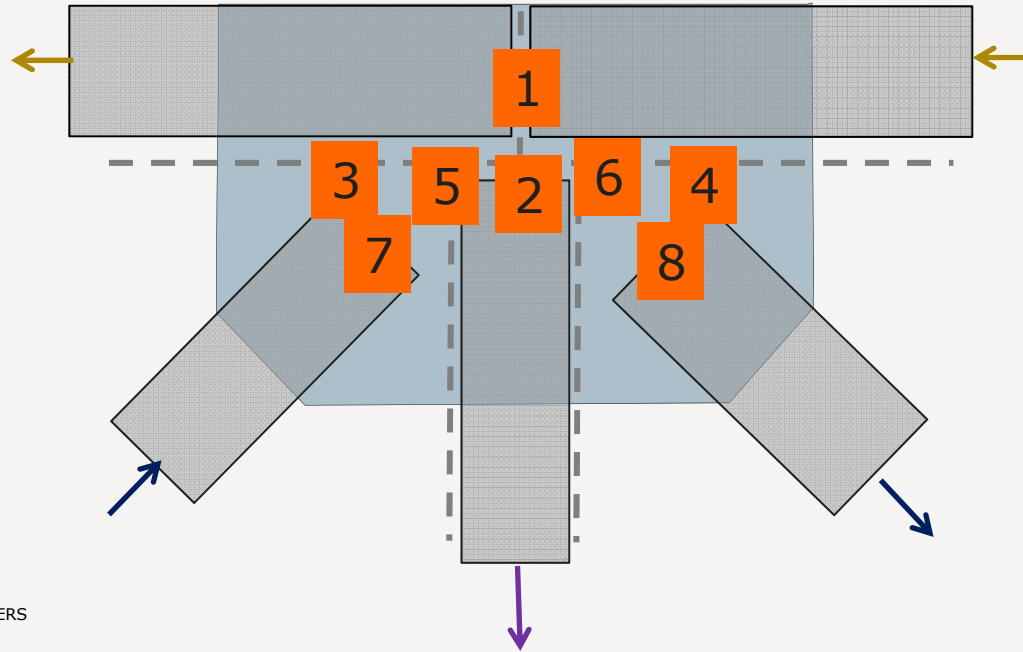


$$\tau_{xy} = \frac{F \sin \alpha}{2L_{eff} t}$$



# Stress Superposition

- > Von Mises stress calculated at locations 1 through 6 using the approximate distributions
- > Whitmore stress calculated at locations 2, 7 and 8



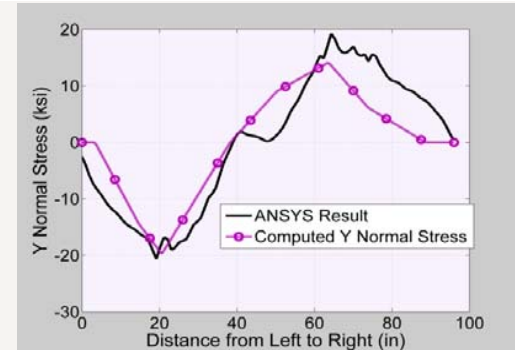
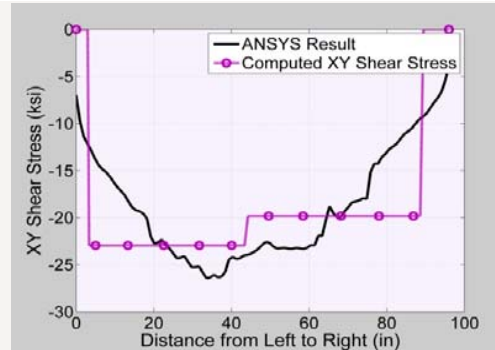
# Example- Joint U10

Von Mises Stress (ksi)

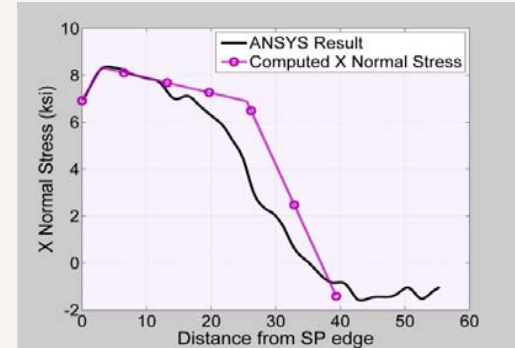
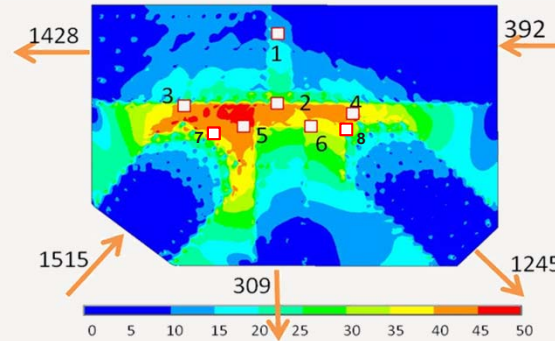
	Predicted	FEA
1	8.3	10-15
2	40.3	40-45
3	44.4	40-45
4	37.1	35-40
<b>5</b>	<b>46.6</b>	<b>45-50</b>
6	36.5	35-40
7	26.1	35-40
8	20.7	30-35

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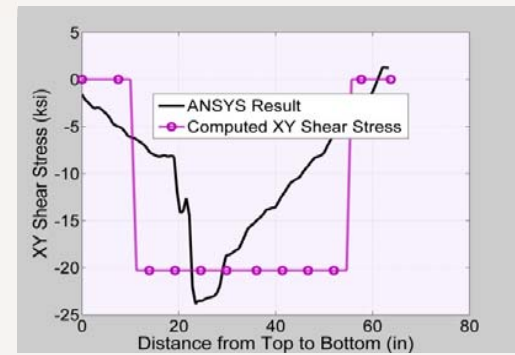
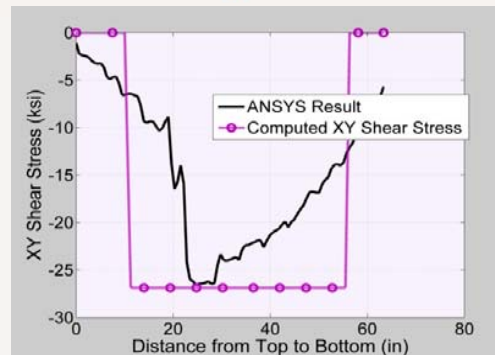
Horizontal section



Chord Splice



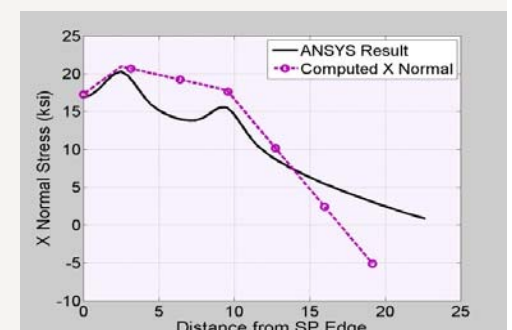
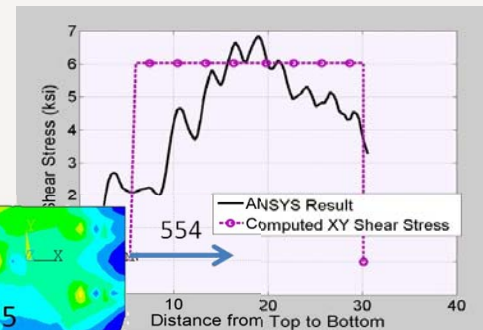
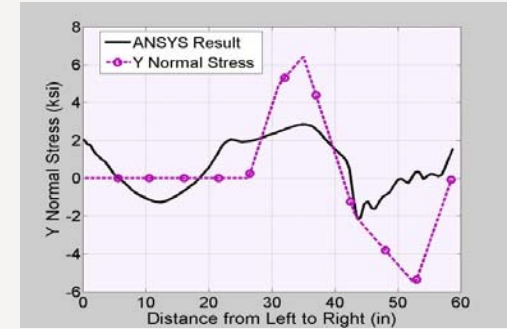
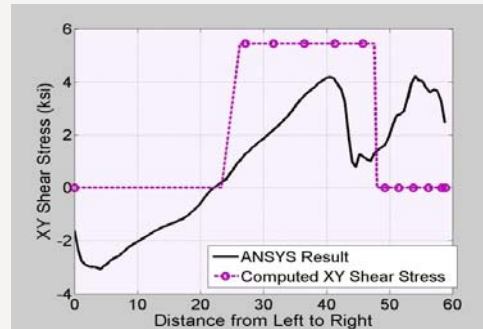
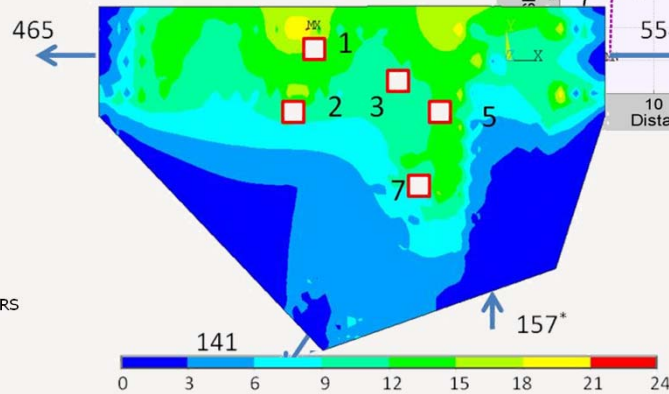
Vertical section



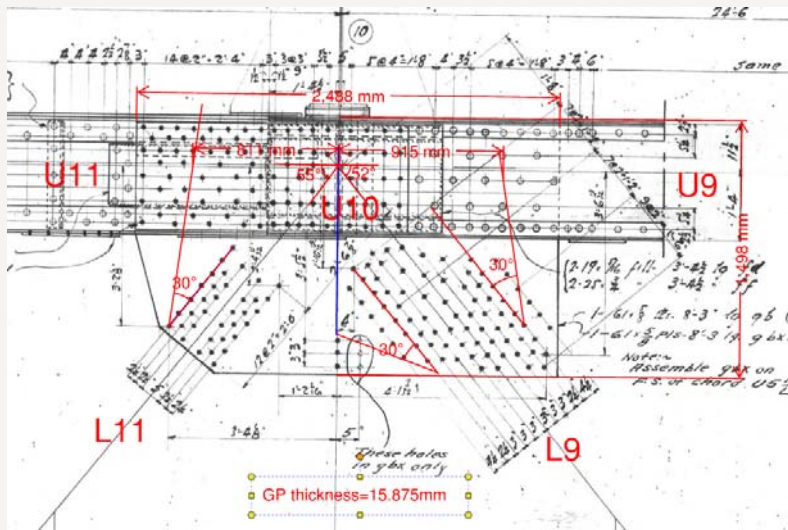
# Example-Joint U3

Von Mises Stress (ksi)

	Predicted	FEA
1	20.9	18-21
2	17.7	15-18
3	11.4	9-12
5	10.5	12-15
7	6.6	9-12



# Application of Calculating Stress



- > According to FHWA procedure
  - > LLFC=0.81 for gross shear yielding along horizontal section
  - > LLFC=0.41 for gross shear yielding along vertical section
- > Horizontal shear force data
  - > U9: Zero force member
  - > U11: D1=1902kN, D2=173kN, D3=301kN, PED=138kN, LL=850kN
  - > L9: D1=1099kN, D2=101kN, D3=176kN, PED=80kN, LL=495kN
  - > L11: D1=799kN, D2=72kN, D3=126kN, PED=65kN, LL=394kN

## Application of Calculating Stress

- > Vertical shear forces
  - > L9: D1=1347kN, D2=123kN, D3=215kN, PED=99kN, LL=607kN
  - > L11: D1=978kN, D2=88kN, D3=154kN, PED=79kN, LL=483kN
- > Load factors:
  - >  $\alpha_{D1} = 1.1, \alpha_{D2} = 1.2, \alpha_{D3} = 1.5, \alpha_{ped} = 1.25, \alpha_{LL} = 1.7$
- > Stresses calculated on the horizontal section
  - > L9:  $\tau_{xy} = 174\text{MPa}, \sigma_y = 213\text{MPa}$  VM=337MPa (@221MPa yield stress)
- > Stresses calculated on the vertical section
  - > L9:  $\tau_{xy} = 180\text{MPa}, \sigma_x = 312\text{MPa}$  VM=412MPa (@221MPa yield stress)
- > Useful to develop gusset plate strengthening strategy

# Conclusions

- > Individual stress distribution
  - > The shear stress distribution was found to be better represented using individual effective shear lengths for each diagonal
  - > The trapezoidal approximate stress distribution at the chord splice better estimates stress than Whitmore's method
- > The maximum stress
  - > The proposed evaluation procedure is able to estimate the magnitude and location of the maximum stress on the gusset plate

The method of calculating stresses on gusset plate provides a useful tool for designing gusset plate connection strengthening.



# Thank You !

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# Loading Profiles

Load Profile	Member Forces Varied	Critical Section Stress Studied
I (4 cases)	U9	Chord Splice
II (5 cases)	L9, L11	Chord Splice and Horizontal Section
III (7 cases)	U9/L9	Chord Splice and Horizontal Section
VI (5 cases)	L10, L11, U11	Vertical Section

