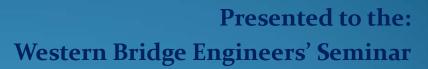
**Technical Presentation** 

# Willow Creek Bridge No. 5 (I-40)

**ADOT Structure No. 1768** 



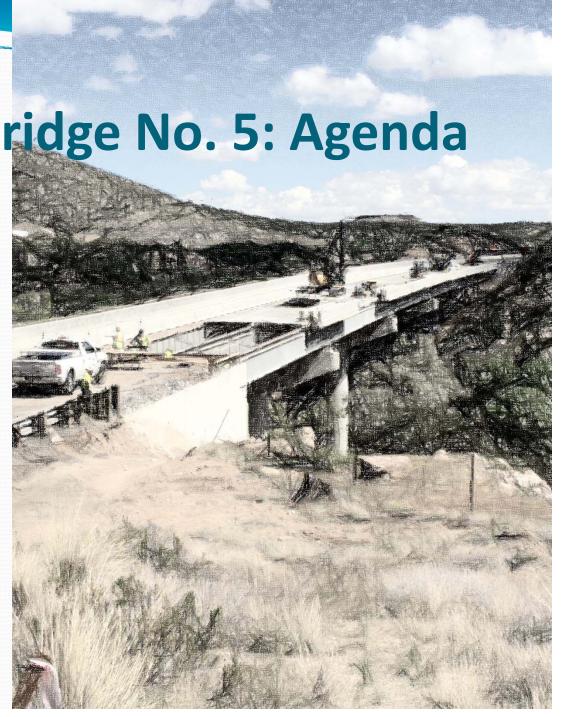
Design Team: Ben C. Ansley, PE Midhat Hassan, PE Christopher A. Labye, PE



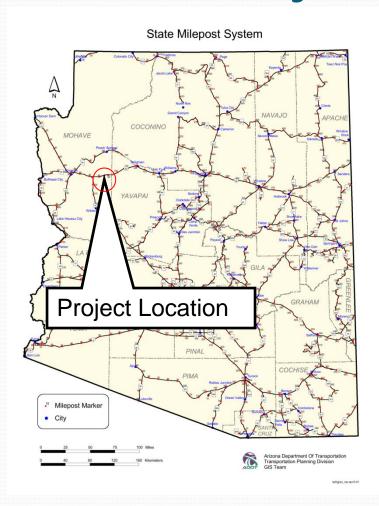


Willow Creek Bridge No. 5: Agenda

- Project Overview
- "The" Problem
- Solution
- Q&A



## **Project Overview**

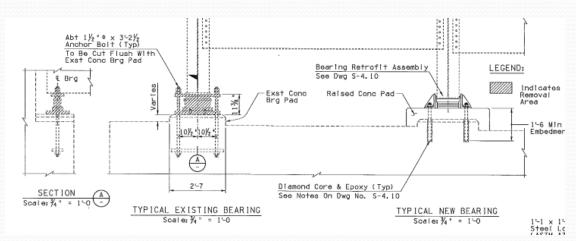


- I-40 Overpasses Mileposts 83.20 to 85.40
- Project Assessment Recommended:
  - Bridge 1 Joint Repair, Girder Repair,
    Abutment Bearing Replacements
  - Bridge 3 Bridge Deck Overlay, Girder
    Repair, Abutment Bearing Replacement
  - Bridge 4 Deck/Barrier/Bearing replacements, Girder Repairs
  - **Bridge 5** Deck/Barrier/Bearing replacements, Girder Repairs

## **Abutment Bearing Replacement**

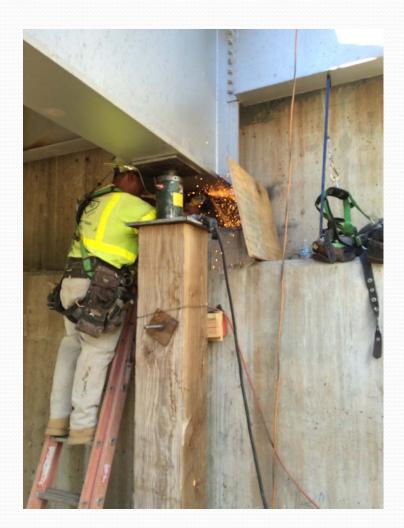


- Rocker bearings replaced with raised concrete pedestals, elastomeric bearing pads, and steel angles to restrain girder movement.
- Work was accomplished with temporary hydraulic jack supports.





# **Abutment Bearing Replacement**



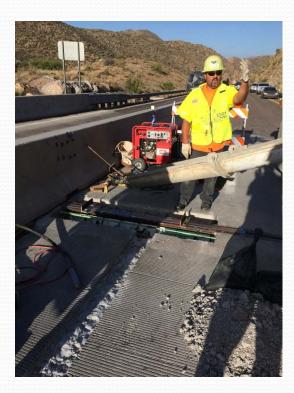


# Bridge No. 1 (Str # 1592)

• Transverse Joint Repair



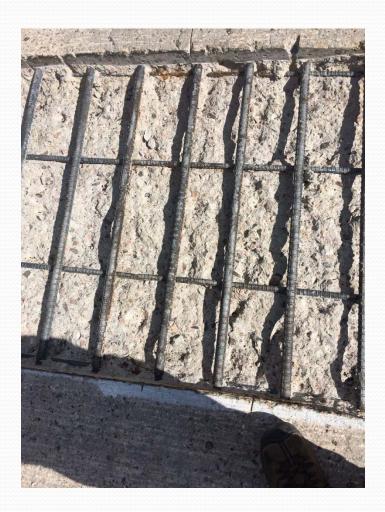




# Bridge No. 3 (Str # 1594)

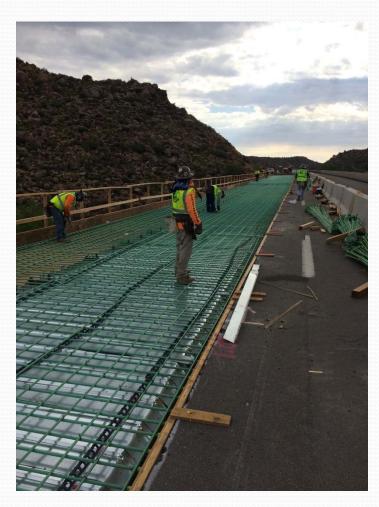
Bridge Deck Overlay





# Bridge No. 4 (Str # 1595)

Deck Replacement



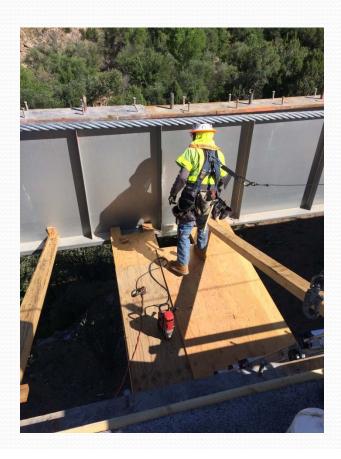


## Bridge No. 4 (Str # 1595)

- Girder Repairs -
  - New Bearing Stiffeners
  - Repair Longitudinal Stiffeners



New Bottom Flange Cover Plate



## **Project Constraints**



- Accelerated Design Schedule
  - 11 months from Project Kick-off to PS&E Submittal
- Limited Construction Window
  - TCB allowed April 30 October 15

 Environmental impacts kept to a minimum. Limited time to obtain clearances during design.

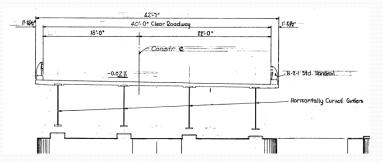
## **Project Constraints**



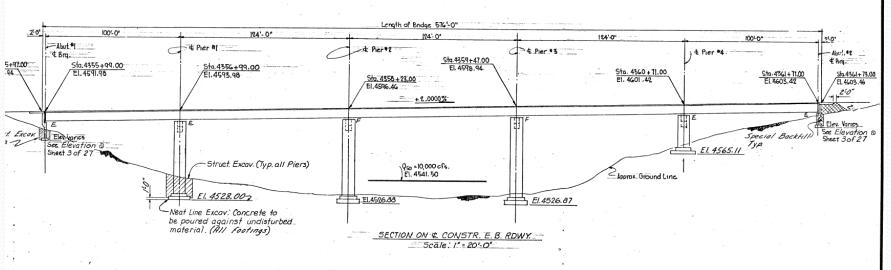
- No cross-overs allowed (7 mile footprint)
- Limited Traffic Openings



## Bridge No. 5 (Str # 1768)

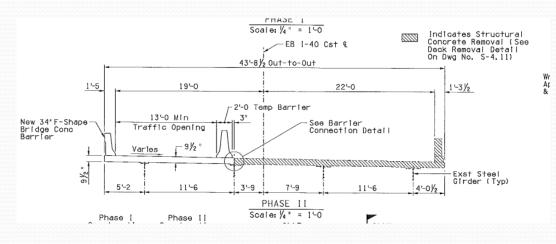


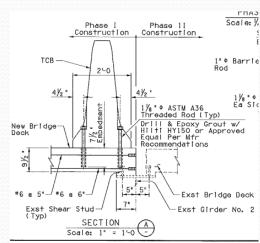
• Willow Creek Bridge No. 5 consisted of a 5-span steel girder bridge with a composite cast-in-place concrete deck built in the late 1960's. The concrete deck was experiencing severe corrosion due to deicing salt usage.



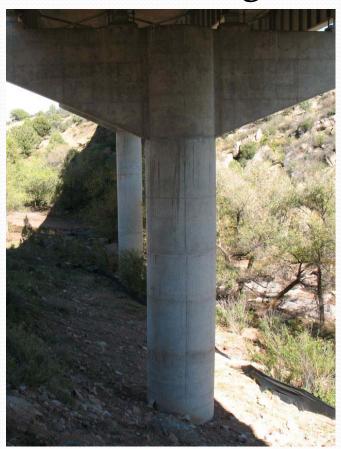
## Bridge No. 5 (Str # 1768)

• Cross-overs were not allowed due to the length of the project and the accompanying safety concerns to the travelling public, so phased deck removal/replacement was required, sometimes with as little as 3" clearance from the TCB to a deck construction joint on Willow Creek Bridge No. 5 (and as low as ½" on Willow Creek Bridge No. 4!).





• Existing pier columns on Bridge No.5:



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  - Single-column piers, large cantilevers (16'-9" pier overhangs)

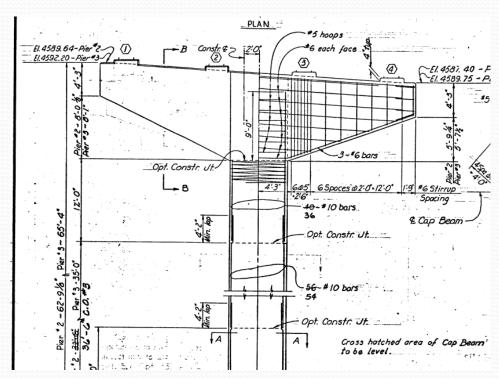


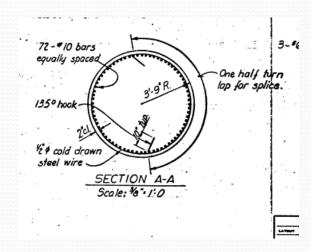
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  - Maximum height of 65'-4" (top of pier to bottom of footing) with a diameter of 7'-6"

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  - 2500 psi concrete
  - Grade 40 steel

- Existing pier columns on Bridge No.5:
  - Not conducive to phased deck replacement!!





• Tiedowns into creek...

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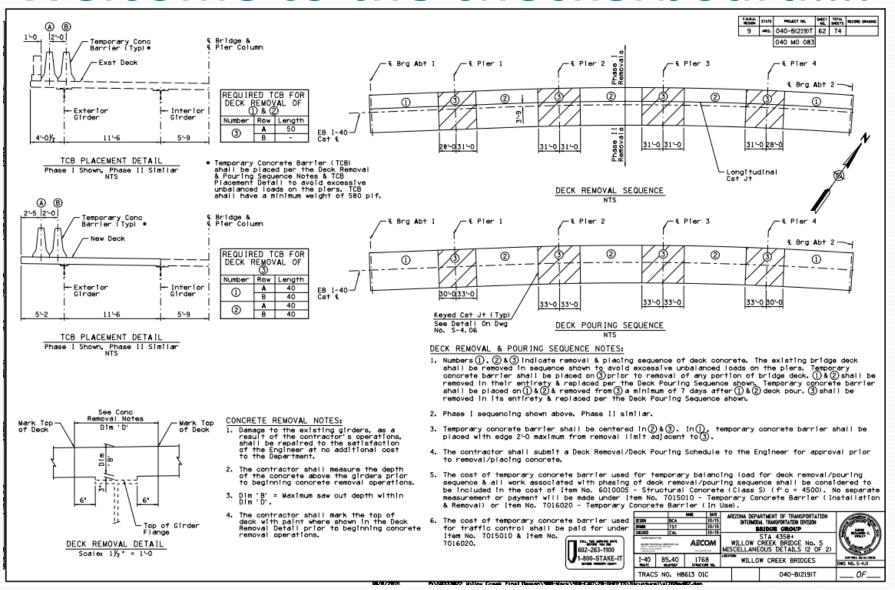
- Tiedowns into creek... (wanted to limit environmental impacts/construction activity in the creek – height of columns and exposure to potential wind/stream forces also led to stability concerns)
- Add counterbalance weights to pier cap... (not feasible due to the weight needed as well as insufficient access from above, and wanted to limit environmental impacts to the creek)

• In a traditional deck sequence placement...

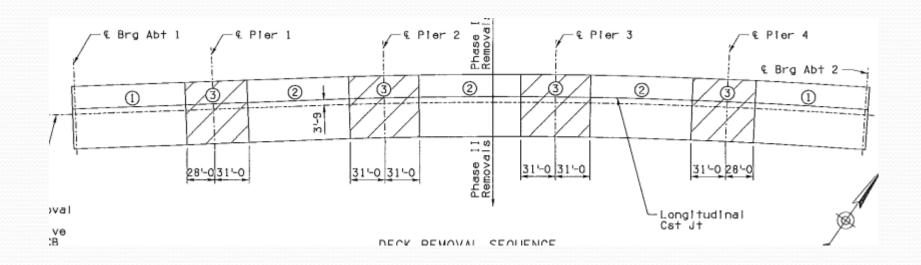
 In a traditional deck sequence placement...we only pour positive moment portions of a deck prior to negative moment portions to avoid continuous cracking that would otherwise occur in the previously poured, cured portions of the deck.

• In a traditional deck sequence placement...we only pour positive moment portions of a deck prior to negative moment portions to avoid continuous cracking that would otherwise occur in the previously poured, cured portions of the deck.

So what if we used the existing deck and remove it in a similar manner to how it was placed to provide the requisite counterbalance?



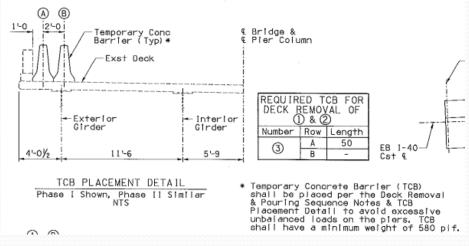
- The checkerboard utilized the following concept to provide the required counterbalance:
  - Portions of the deck (positive moment areas up to contraflexure points) were removed...



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  - Portions of the deck (positive moment areas up to contraflexure points) were removed; however, this load was not QUITE enough to provide the requisite counterbalance. To make up for the extra load, rows of additional TCBs were placed on the negative moment

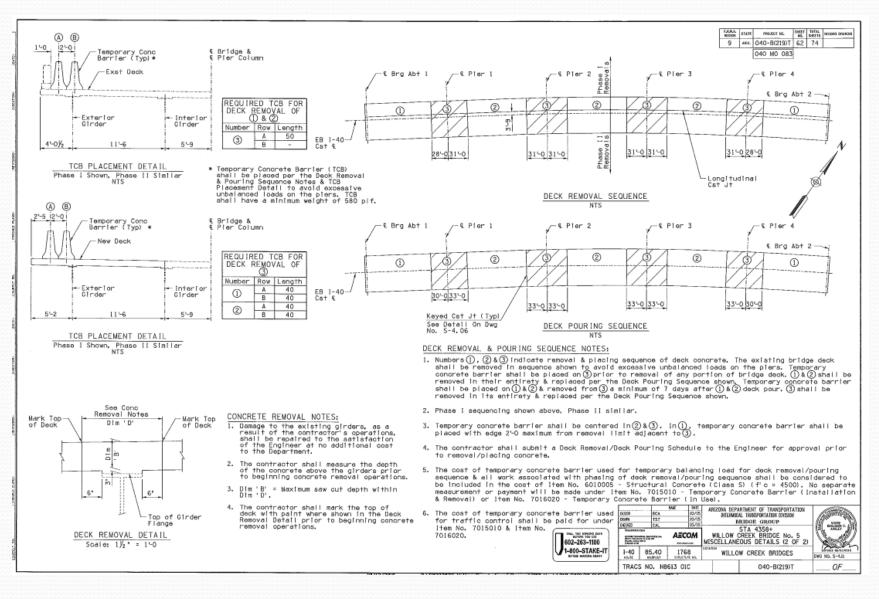
portions of the deck.



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  - Portions of the deck (positive moment areas up to contraflexure points) were removed; however, this load was not QUITE enough to provide the requisite counterbalance. To make up for the extra load, rows of additional TCBs were placed on the negative moment portions of the deck.
  - Once the new positive deck portions were poured and cured, TCBs were shifted to the cured sections while negative moment sections were removed then poured.

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  - Portions of the deck (positive moment areas up to contraflexure points) were removed; however, this load was not QUITE enough to provide the requisite counterbalance. To make up for the extra load, rows of additional TCBs were placed on the negative moment portions of the deck.
  - Once the new positive deck portions were poured and cured, TCBs were shifted to the cured sections while negative moment sections were removed then poured.
  - Lengths of TCBs required for counterbalance were computed depending on the deck sequence.
  - The same process was repeated during Phase II.



• What about debris in the wash?



What about debris in the wash?



- What about debris in the wash?
- The contractor prevented debris from entering the wash successfully by placing plywood between bottom girder flanges.
- Overhangs were removed gradually using a bucket to catch debris off to the side as it was removed as shown in the next slide.









## **Special Design Notes**

There was some initial concern about variable girder deflections utilizing this construction methodology. Girder deflections were anticipated to be minimal. However, the following precautions were taken:

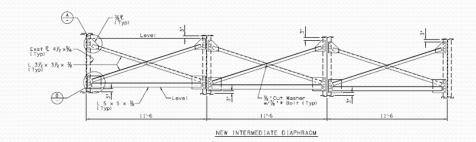
- Contractor was requested to provide extensive survey shots along deck edges and girder centerlines before deck removals and after deck removals. Top of girder elevations were also shot.
- This extensive survey enabled us to confirm our minimal girder deflection assumptions because of the stiffness provided by remaining portions of the deck (longitudinally and transversely) as well as cross-bracing.

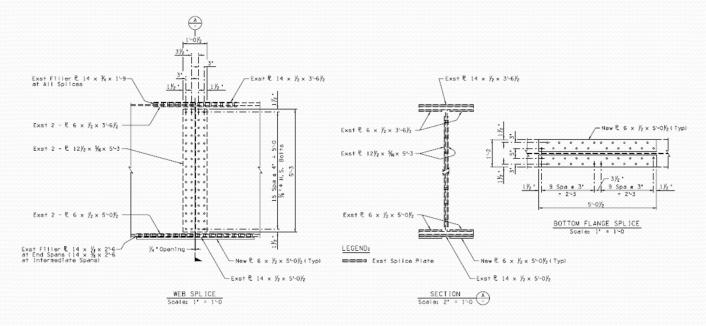
## Special Design Notes (cont'd)

- Also, this extensive survey allowed us to better accommodate and tie into existing superelevation transitions to minimize any awkward roadway-bridge transitions experienced by the traveling public.
- This survey also helped us to confirm the magnitude of the new build-ups on the girders.

## Bridge No. 5 (Str # 1768)

- Girder Repairs -
  - Bottom Flange Splice Plate Repair
  - New Bottom Flange Cover Plate
  - New Intermediate Cross Bracing





# Questions?

