



# Walnut Avenue Overpass

Warm Springs Extension, LTSS

By:

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# BART's Role

- BART – Bay Area Rapid Transit
  - Is the San Francisco Bay Area's commuter rail system.
  - It serves Alameda, Contra Costa, San Francisco, and San Mateo counties.
  - It's the 5<sup>th</sup> busiest heavy rail rapid transit system in the US.
  - Serves Approx. 400,000 daily riders.
  - <https://www.bart.gov/>



# BART System Map



# Bart's Goal

- BART (Bay Area Rapid Transit) agency's ultimate goal is to enhance regional mobility, generate new ridership by extending commuter service to multiple regions in the Bay area.
- Extending BART service to San Jose has been a long awaited goal and the Warm Springs Extension is part of this goal.
- Warm Springs Extension Line, Track, Station and Systems Project is a 5.4-mile extension from existing Fremont Station south to a new station in the Warm Springs District of the City of Fremont.

# Walnut Avenue Overpass



- Low vertical clearance
- Fixed rail profile



# Walnut Avenue Overpass



- Two girders
- Slight Skew

# Walnut Avenue Overpass - Innovation

- Cast in Place Post-tensioned Concrete thru girder for rail transit (1<sup>st</sup> in CA)
- Straddles the Hayward Fault at the #2 Abutment (1<sup>st</sup> in CA)
- Supported by two Mechanically Stabilized Earth (MSE) abutments (1<sup>st</sup> in CA)



# Walnut Avenue Overpass - OVERVIEW

- Seismic loading
- Superstructure design
- Substructure (MSE) design



# Seismic Loading



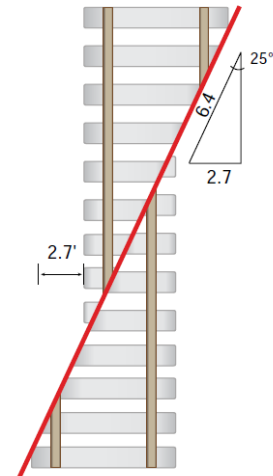
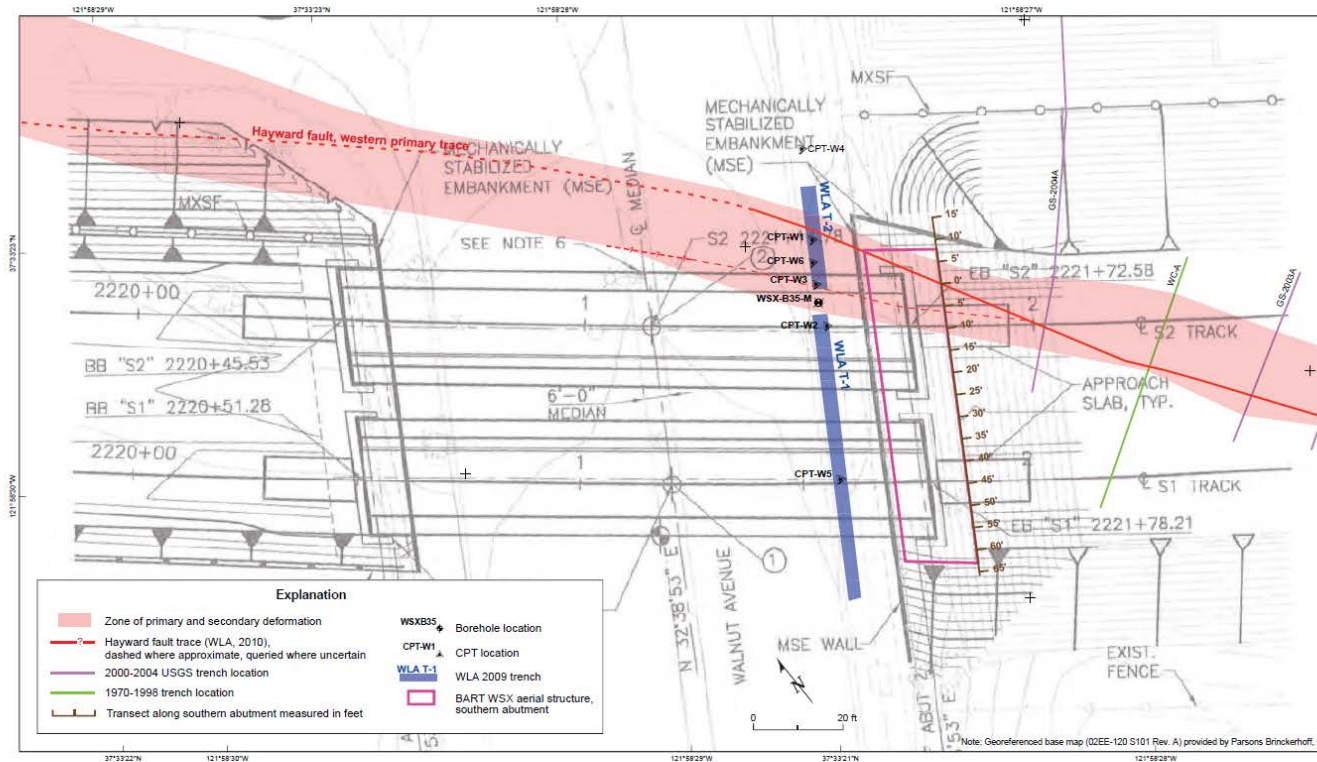
## Hayward Fault Statistics:

- 31% probability of a large (Mw.6.8) earthquake between 2006 to 2035

# Seismic Loading

## USGS Description: Right-Lateral strike-slip offset & East-Side-Down vertical displacement.

Fugro Consultants, Inc.  
Project No. 1968 BART Warm Springs Extension



Approximate scale

D) Median + 1  $\sigma$  displacement  
D<sub>16</sub> = 6.4 feet

Site Map of the BART WSX Overpass Structure along Walnut Avenue

FIGURE 2



# Seismic Loading

## CHALLENGES

- During a fault rupture event we could count on:
  - Horizontal displacements up to 6.4 ft
  - Vertical Displacement up to 1.3 ft (~16 inches)
- Fault Creep: Fault is constantly moving at an approximate rate of 0.2 inches/year.
- BART's design policy:

*“To ensure safety and to provide post-earthquake operability by limiting strains, deflections and damage such that they are capable of being returned to service within a reasonable amount of time....with only minor repairs or shoring.”*

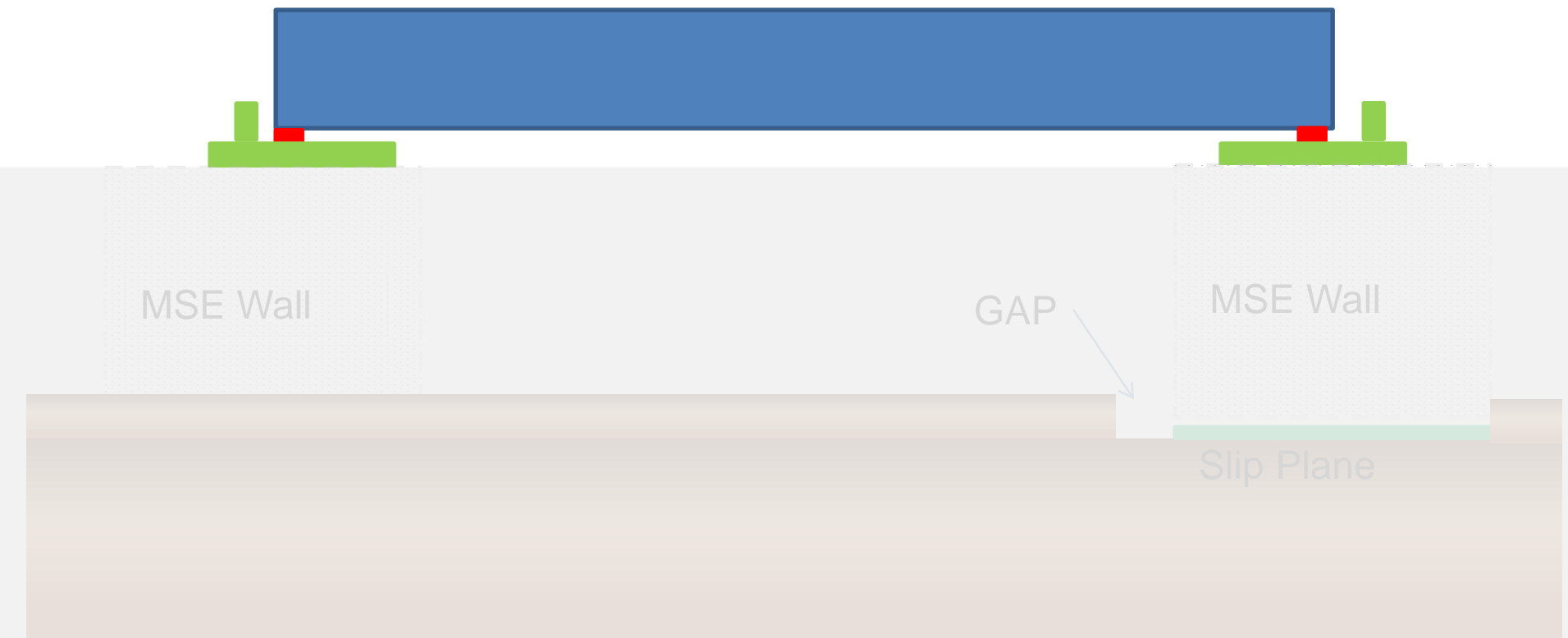
# Seismic Loading - Design

## How to design away our challenge

- How to simplify the analysis
  - Highly predictable Design
- Minimize the number of variables within the design of the bridges components
  - Load Path, Load Path, Load Path.



# Superstructure - Design





# Superstructure - Design

## APPROACH/IMPLEMENTATION TO MITIGATE FAULT RUPTURE EFFECTS

- **SUPERSTRUCTURE**

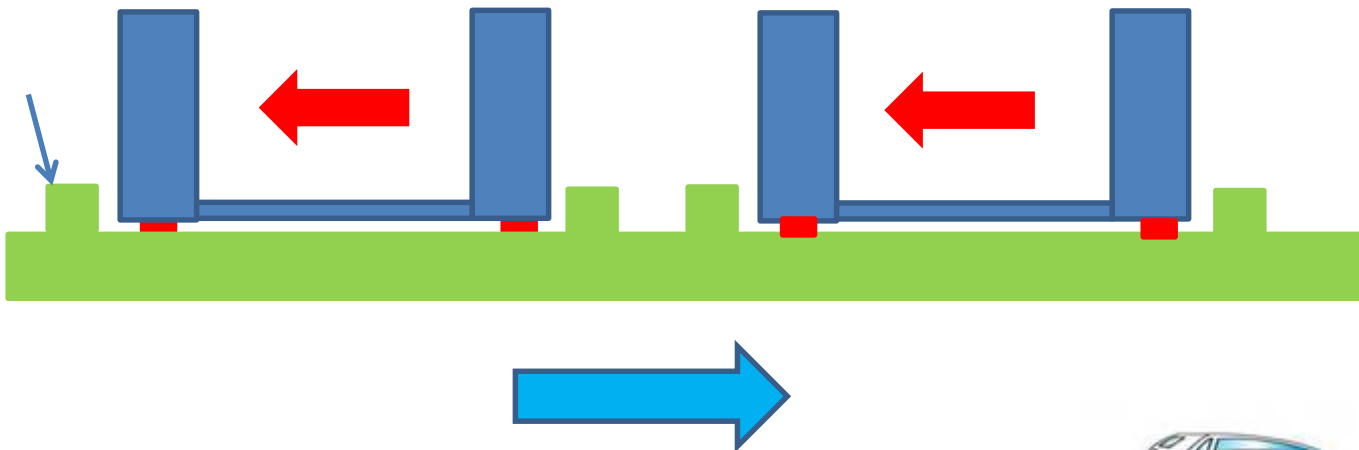
- Anticipated the loss of one bearing at south abutment. Girders are designed for resulting:
  - Shear
  - Torsion
- The girders are supported on wide-seat abutment spread footings.
  - Transverse Post Tensioned spread footing
- Shallow foundations preferred over deep foundations
  - Minimizes unpredictable behavior

# Superstructure - Design

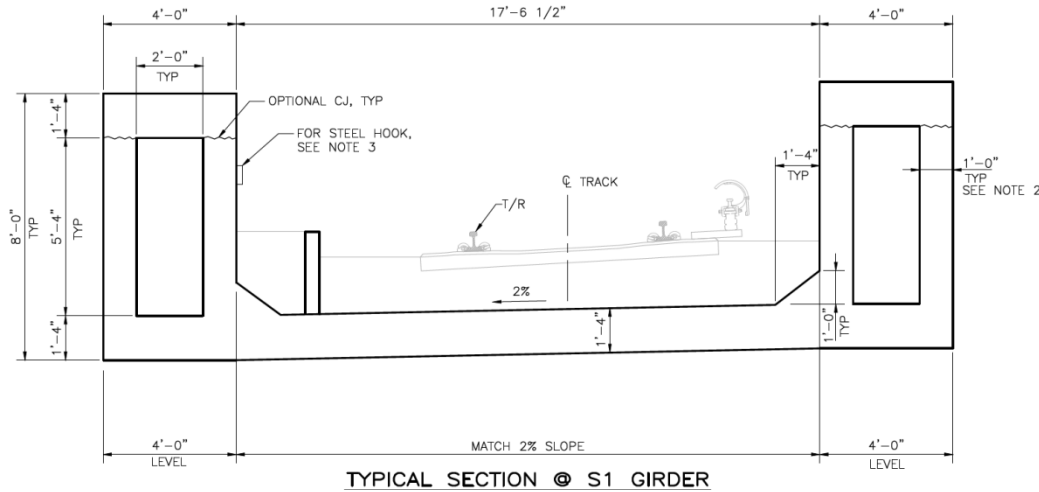
- Longitudinal



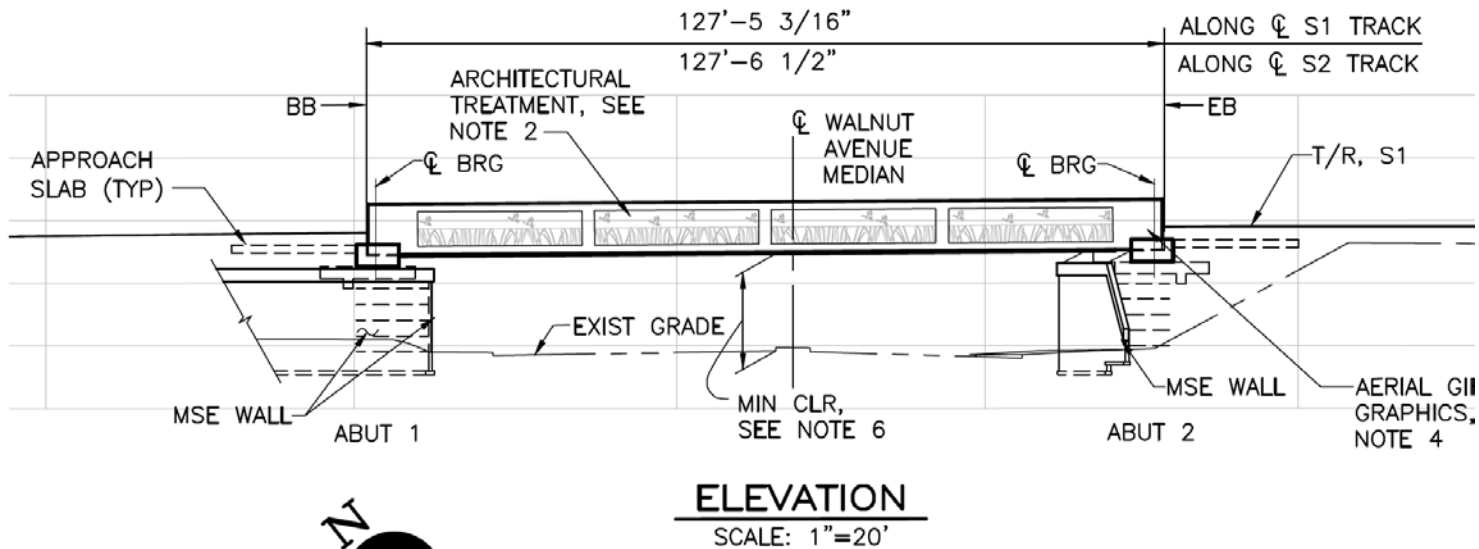
- Transverse



# Superstructure - Design

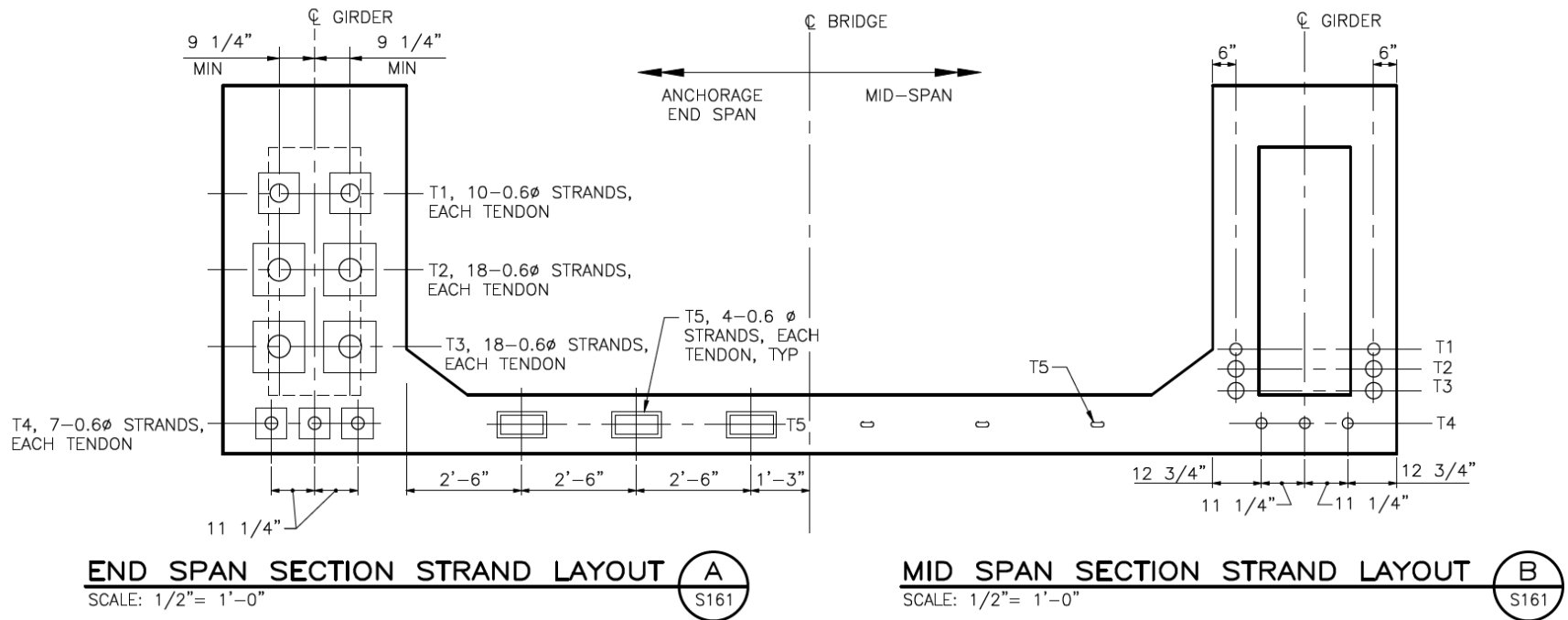


Girder Depth: 8'-0"  
 Span length: 127'-6"  
 $P_{jack}$ : 10,888 kips  
 Clear distance (interior face to face): 17'-6 1/2"  
 Slab thickness: 1'-4"  
 Designed for Zero tension



# Superstructure - Design

## Post Tensioning



**END SPAN SECTION STRAND LAYOUT (A)**  
SCALE: 1/2" = 1'-0"

**MID SPAN SECTION STRAND LAYOUT (B)**  
SCALE: 1/2" = 1'-0"

### PRESTRESSING TABLE

	X <sub>1</sub> (in)	X <sub>2</sub> (in)	X <sub>3</sub> (in)	JACKING FORCE PER STRAND (KIPS)	NOTES
T1	68.50"	37.11"	26.61"	43.94	10-0.6ϕ STRANDS EACH TENDON (TOTAL 4)
T2	48.72"	29.04"	22.47"	43.94	18-0.6ϕ STRANDS EACH TENDON (TOTAL 4)
T3	28.72"	19.72"	16.72"	43.94	18-0.6ϕ STRANDS EACH TENDON (TOTAL 4)
T4	8.36"	8.36"	8.36"	43.94	7-0.6ϕ STRANDS EACH TENDON (TOTAL 6)
T5	8"	8"	8"	43.94	4-0.6ϕ STRANDS EACH TENDON (TOTAL 6)

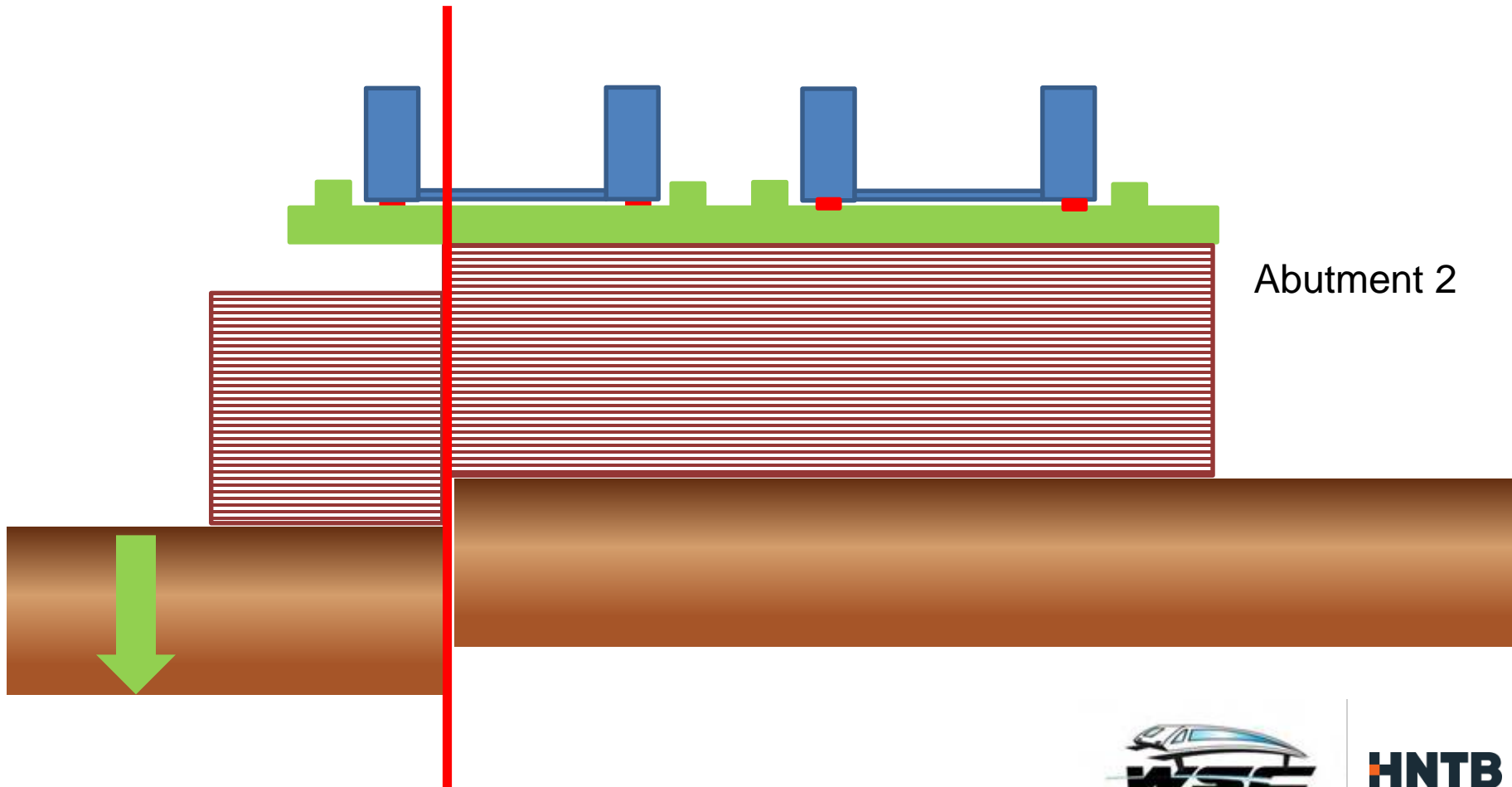
## **FAULT RUPTURE**

- Girder Design
- Spread Footing Model



# Superstructure – Vertical Fault Rupture

- ABUTMENT 2 – Vertical Rupture - Displacement = 1.3 ft (16 inches)



# Superstructure – Vertical Fault Rupture

## Torsion

Blodgett Graphic – Torsional Response. Balancing on opposite corners

2.10-12 / Load & Stress Analysis

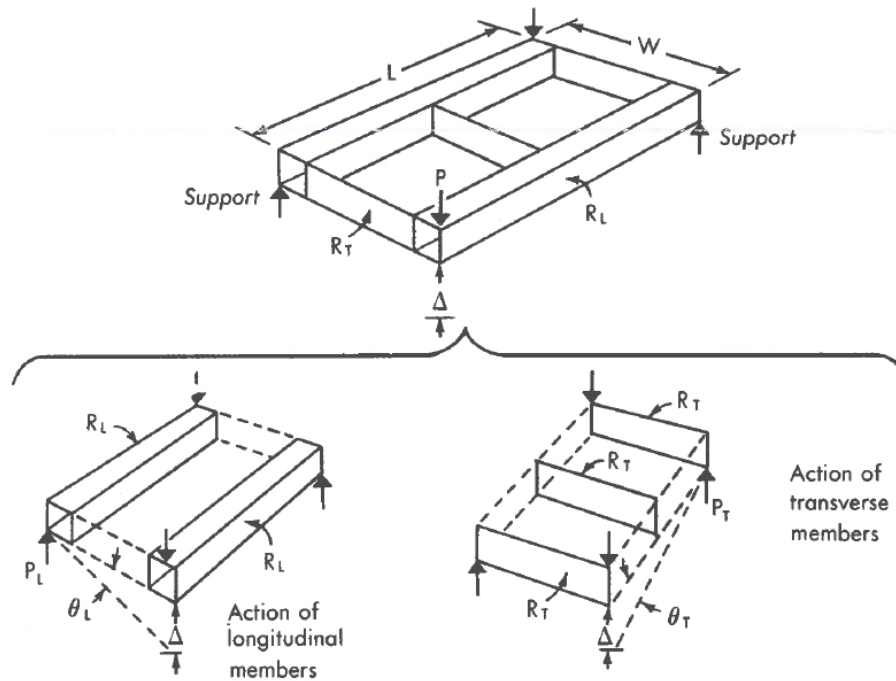
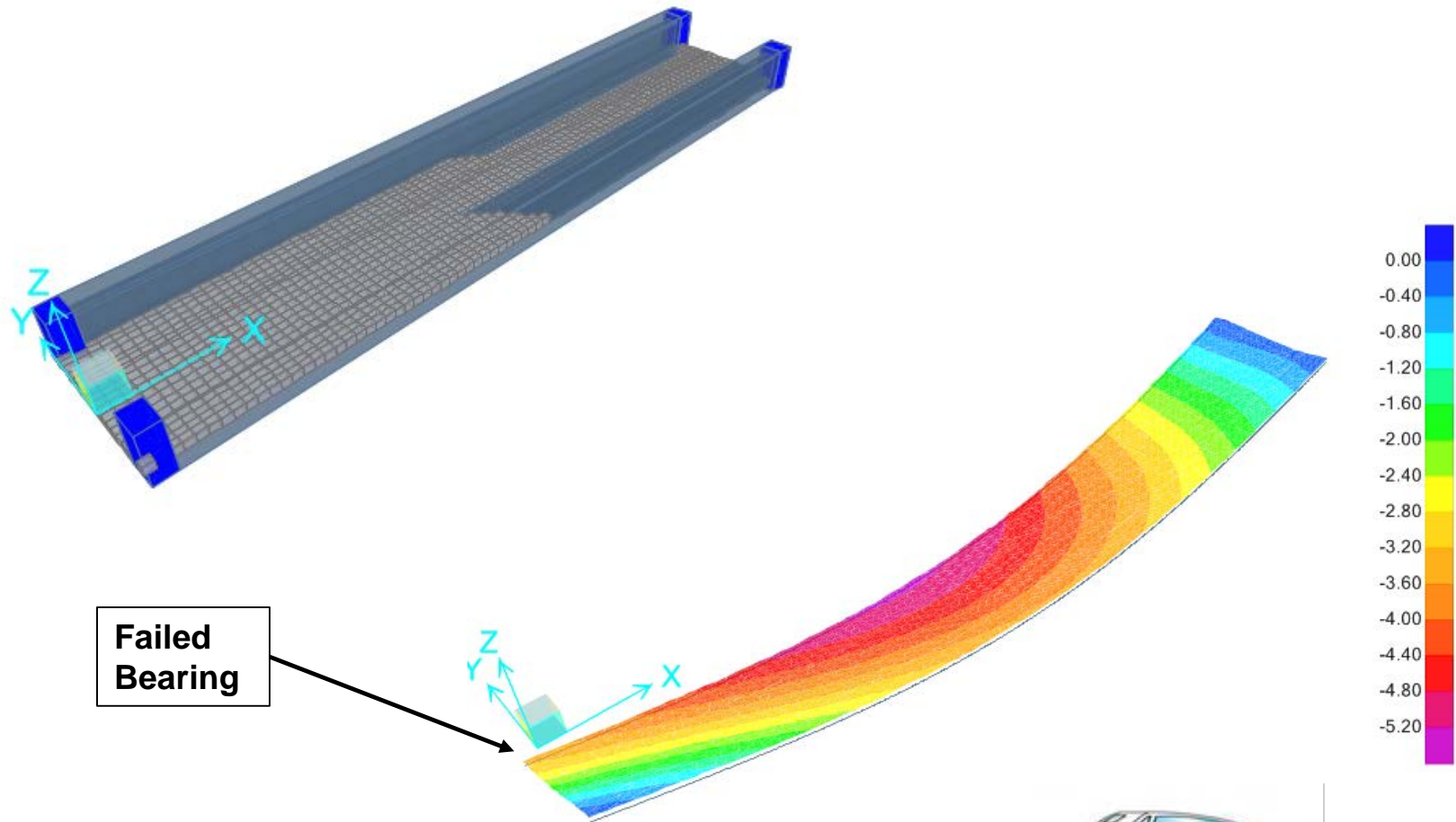


FIGURE 20

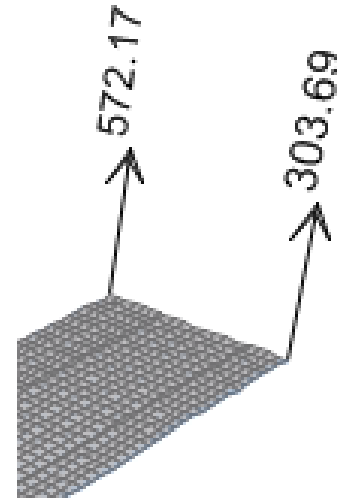
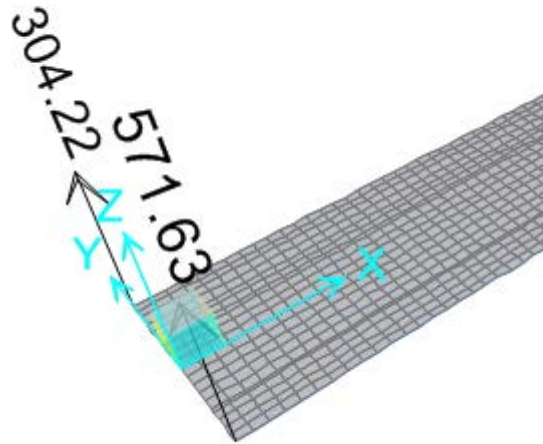
# Superstructure – Vertical Fault Rupture

Bearing failure – superstructure deflection



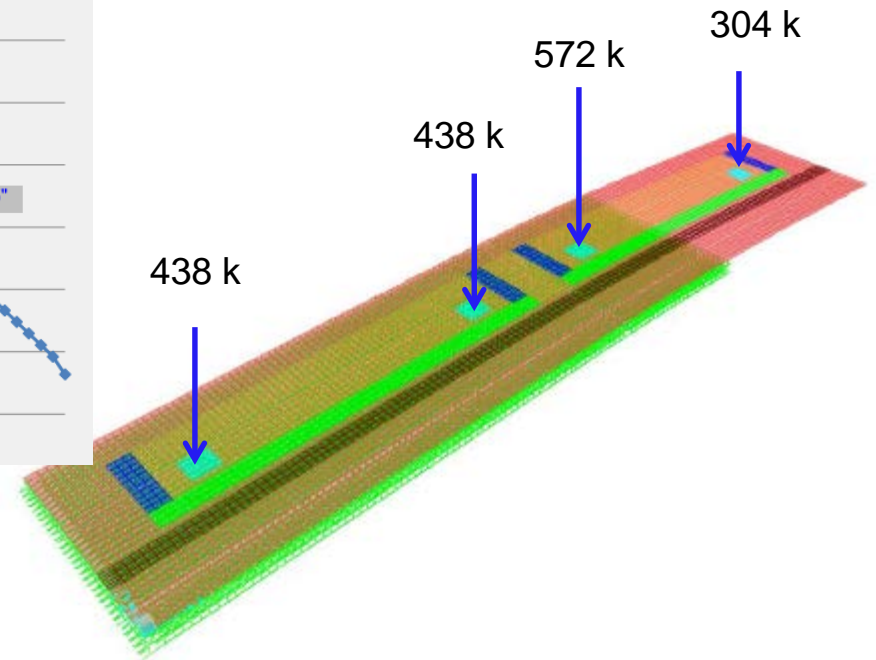
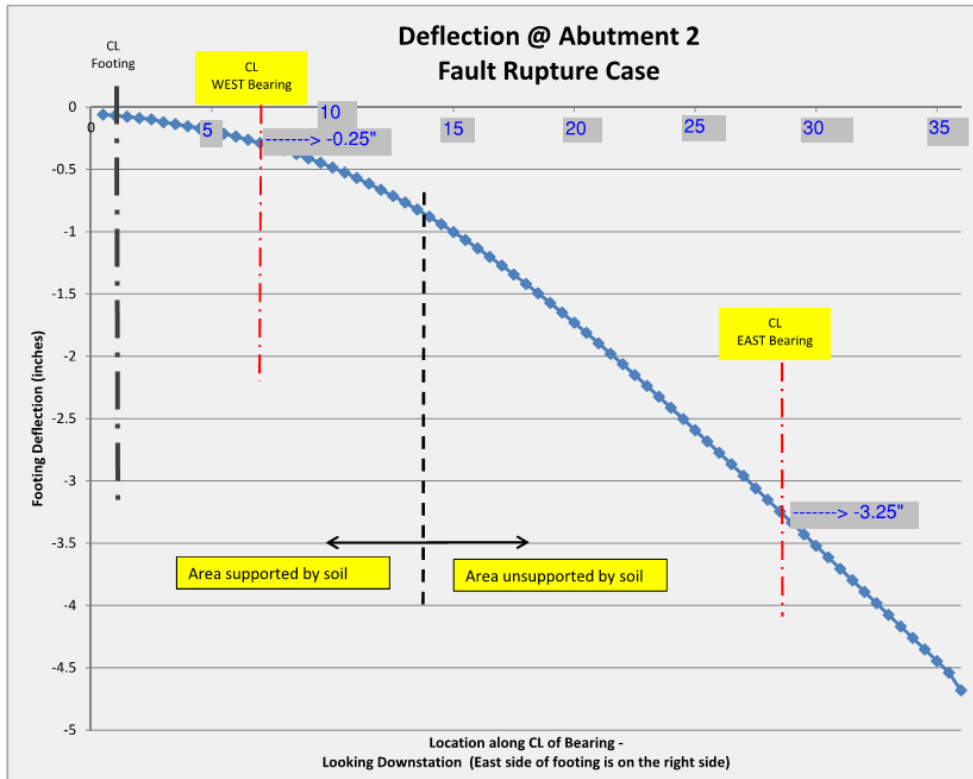
# Superstructure – Vertical Fault Rupture

Bearing failure – end reactions



# Superstructure – Vertical Fault Rupture

## Bearing failure – abutment footing deflection

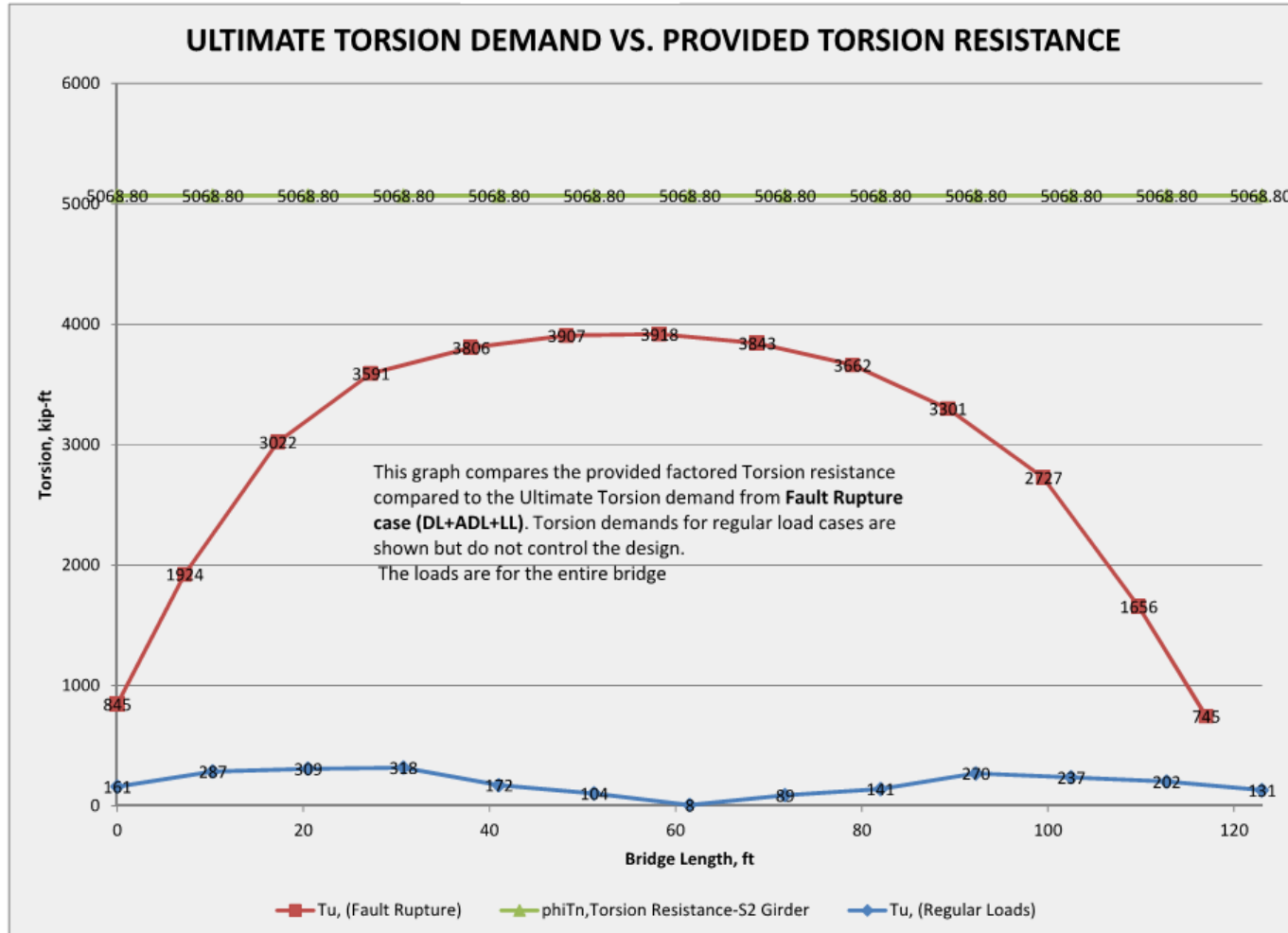




# Superstructure – Vertical Fault Rupture

## Girder Torsion Design

- Threshold =  $T_{cr} / 4 = 2000$  kip-ft



# Superstructure – Vertical Fault Rupture

## TRANSVERSE DESIGN

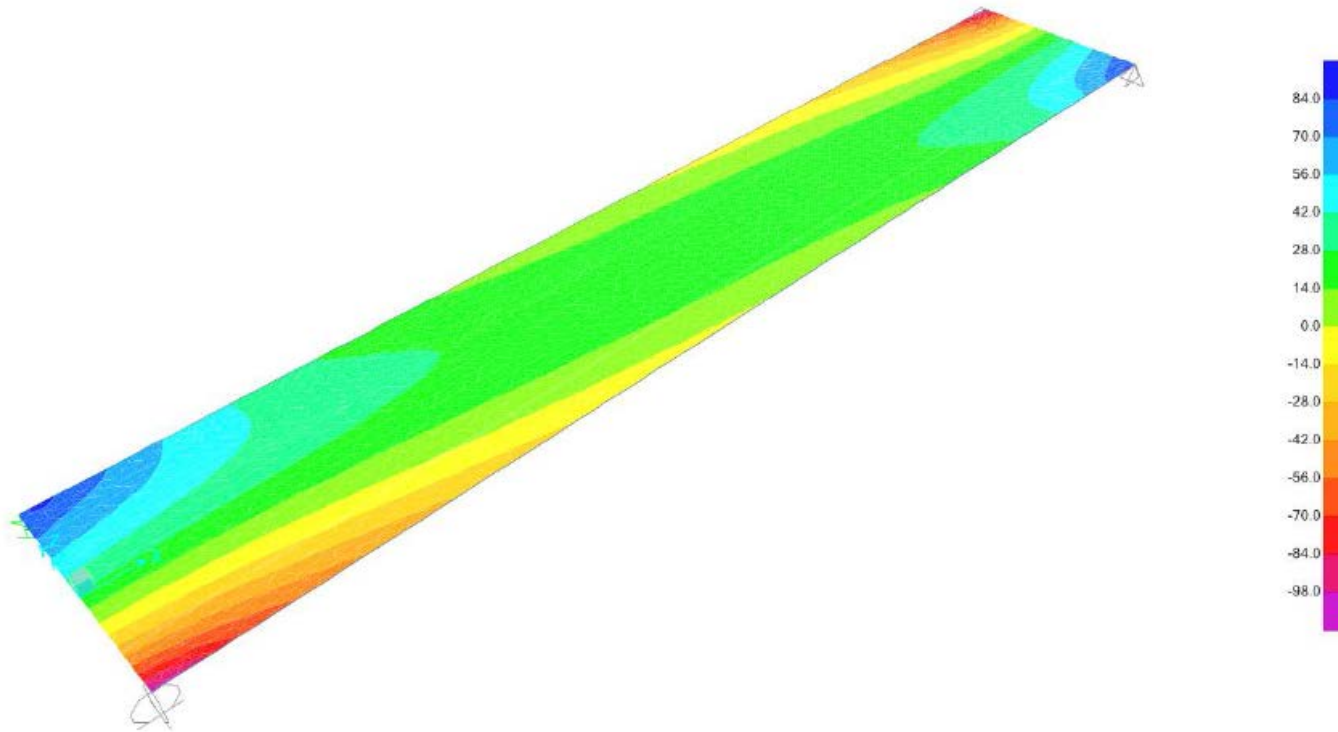
- Deck design

## COMBINED Longitudinal and Transverse

- Girder Webs

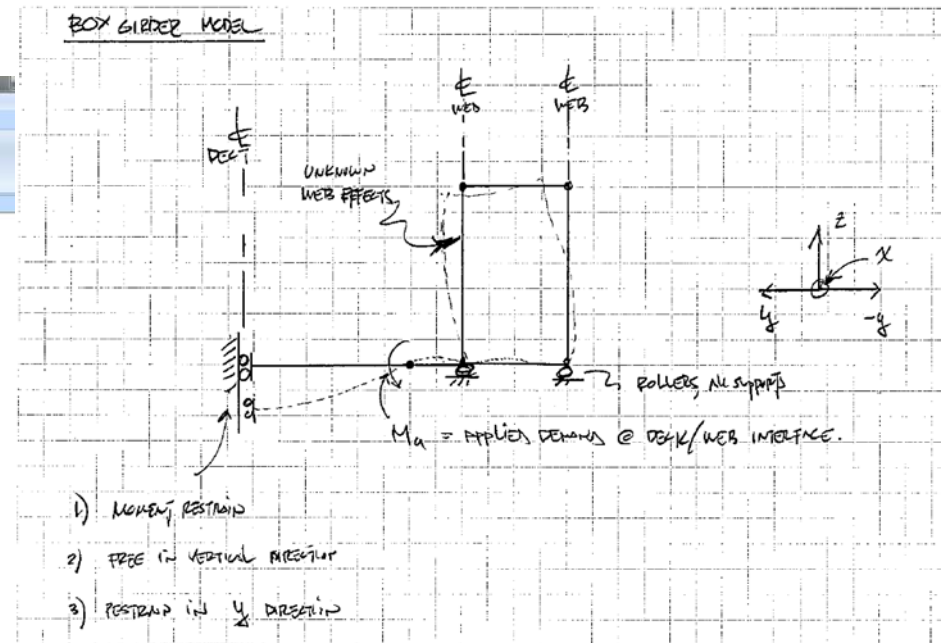
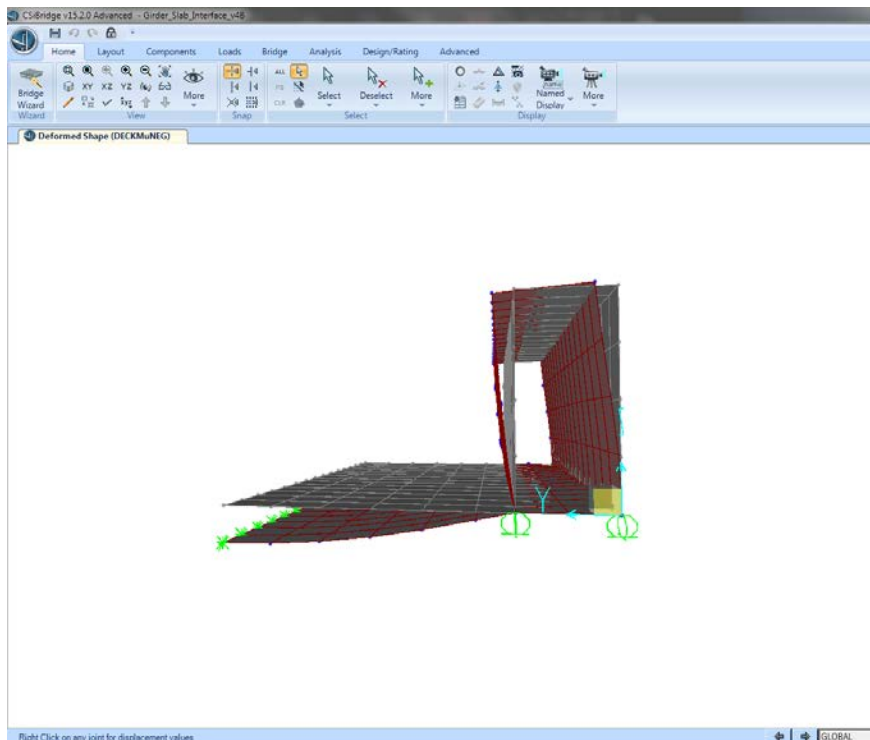
# Superstructure – Vertical Fault Rupture

- Loss of one bearing (fault rupture)
  - $(Mu)_{negative} = (Mu)_{positive} = 83 \text{ kip-ft/ft}$

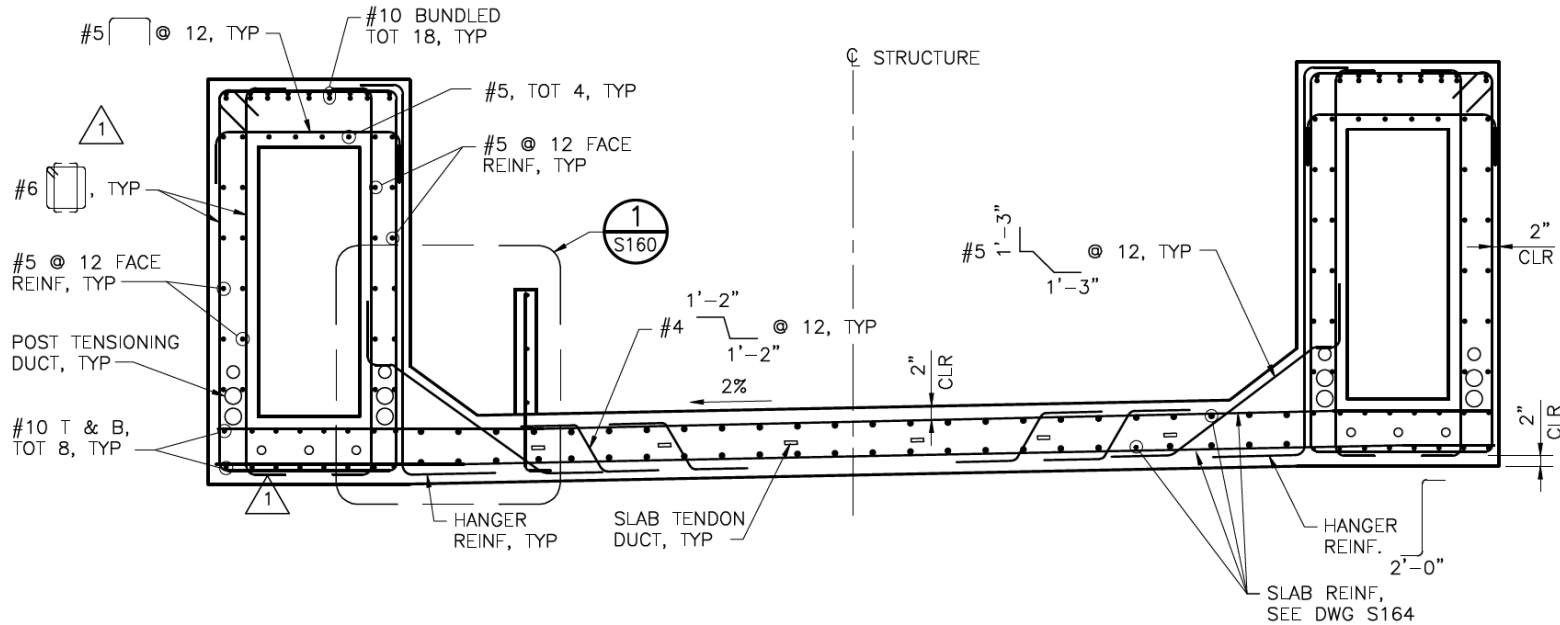


# Girder Mild Reinforcement

- Web design – combine longitudinal and transverse effects for the maximum of:
  - 100% transverse + 50% longitudinal
  - 50% transverse + 100% longitudinal



# Girder Mild Reinforcement



## TYPICAL REINFORCEMENT

SCALE: 1/2"=1'-0"

- Web Stirrups = #6@9
- Deck Reinforcement = #8@6 (trans) top & bott
- Vertical EQ Reinforcement = 18-#10 (long) per box

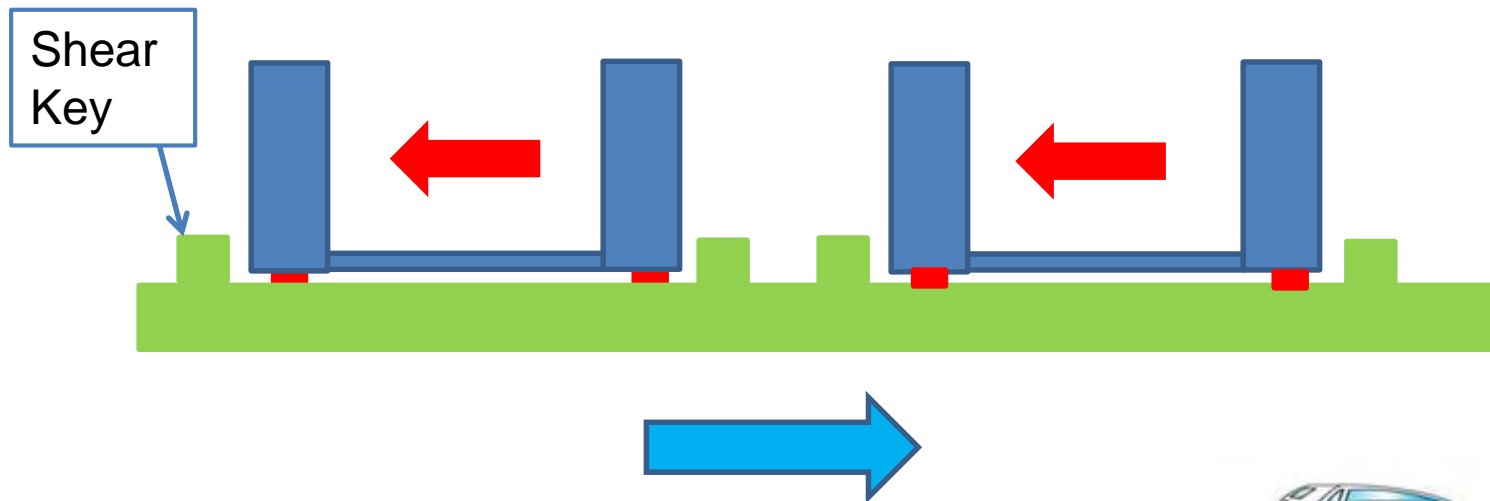


# Seismic Design – shear keys

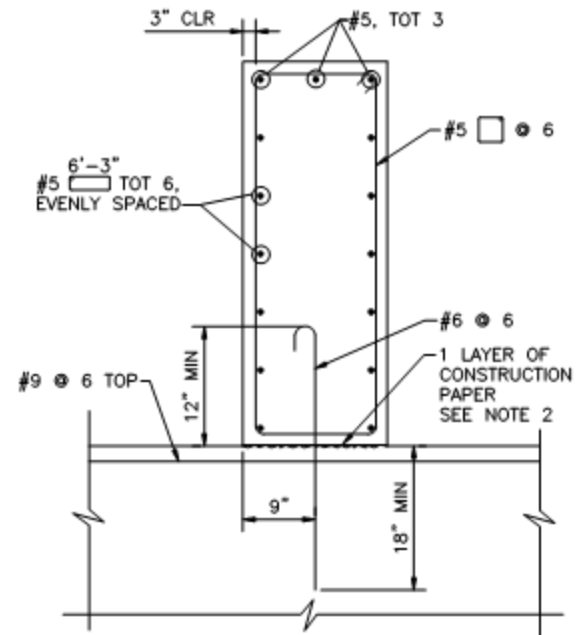
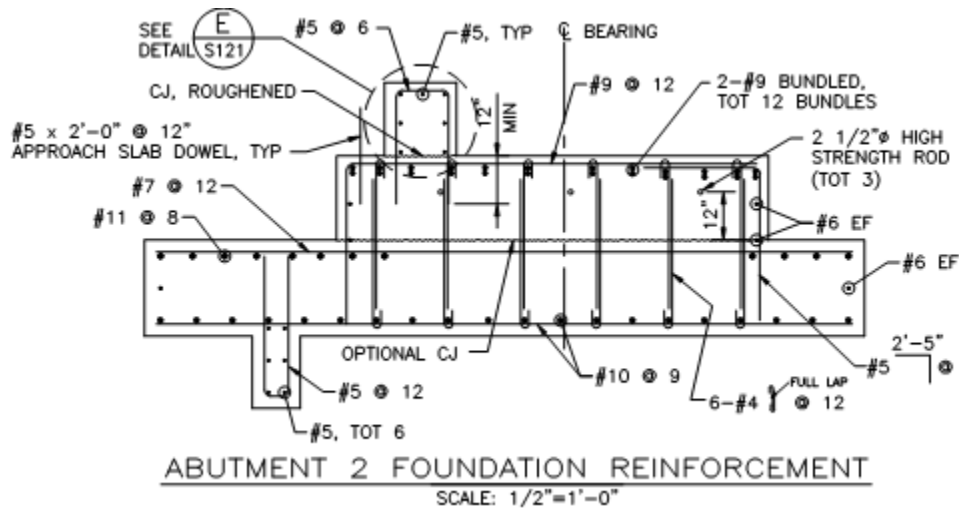
- Longitudinal – designed to remain intact



- Transverse – designed to fuse



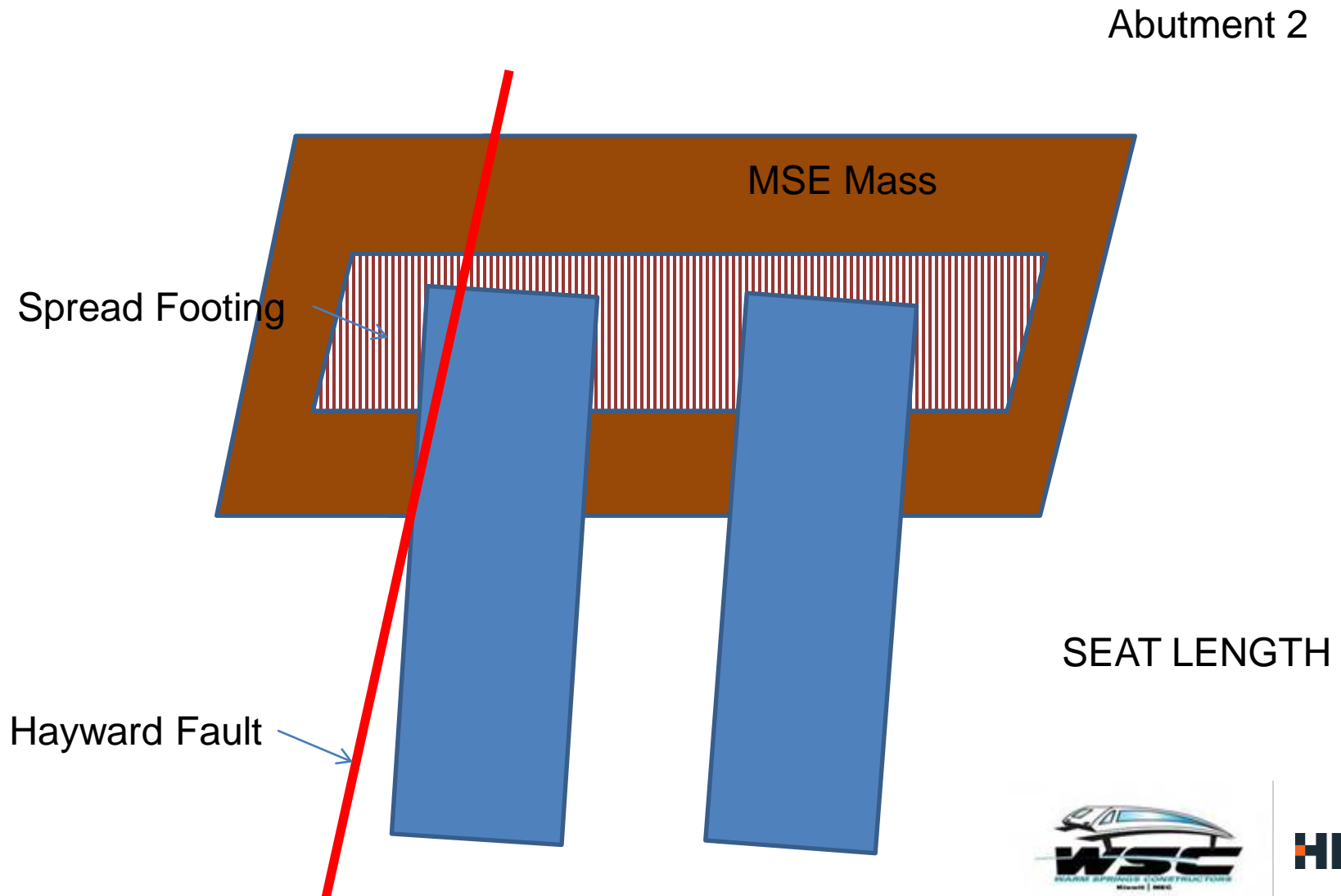
# Footing Reinforcement



- 3-2.5" dia High Strength PT rods
- Transverse Shear Key – detailed to fuse

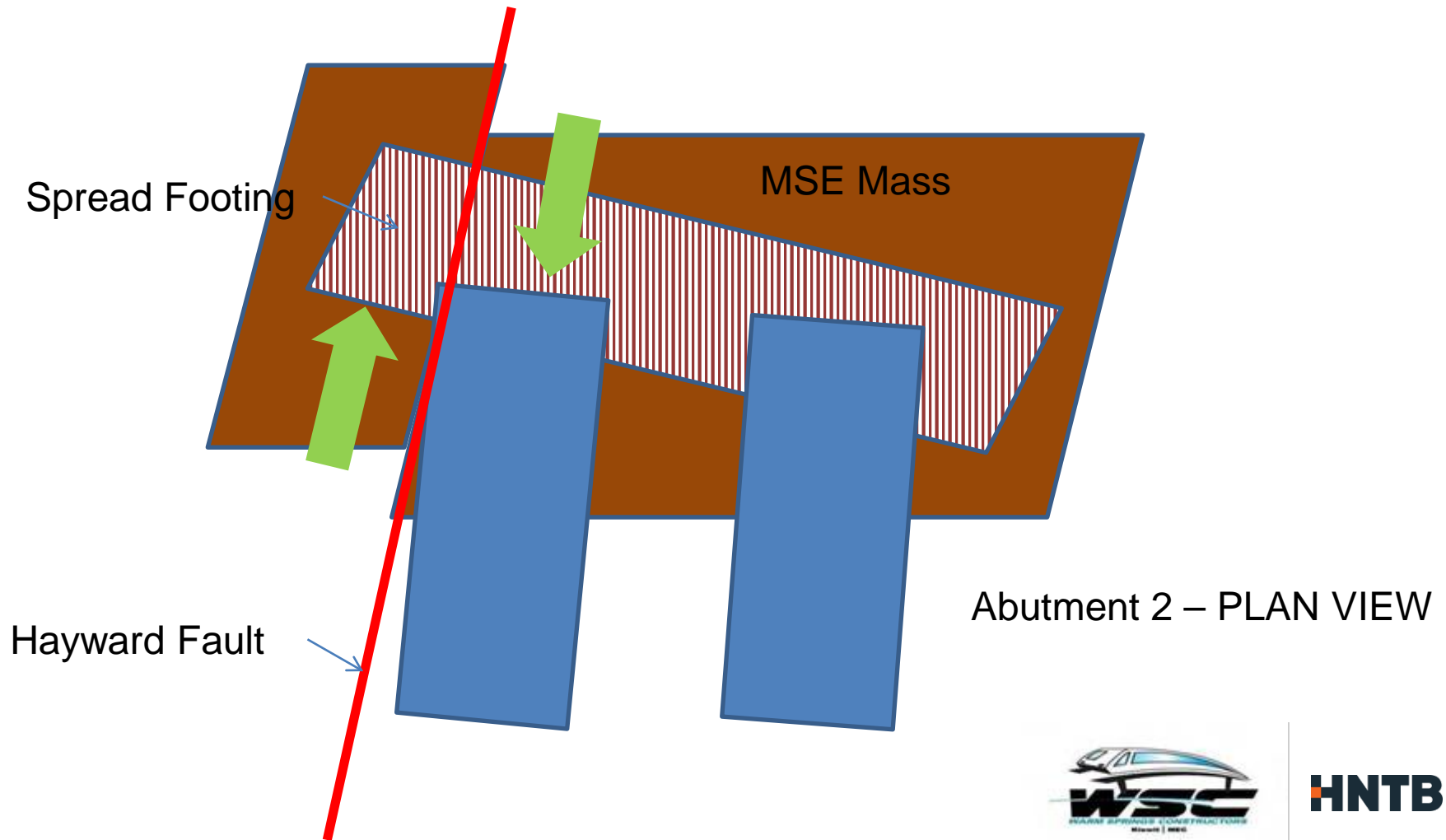
# Superstructure – Horizontal Fault Rupture

- ABUTMENT 2 – Before Fault Rupture

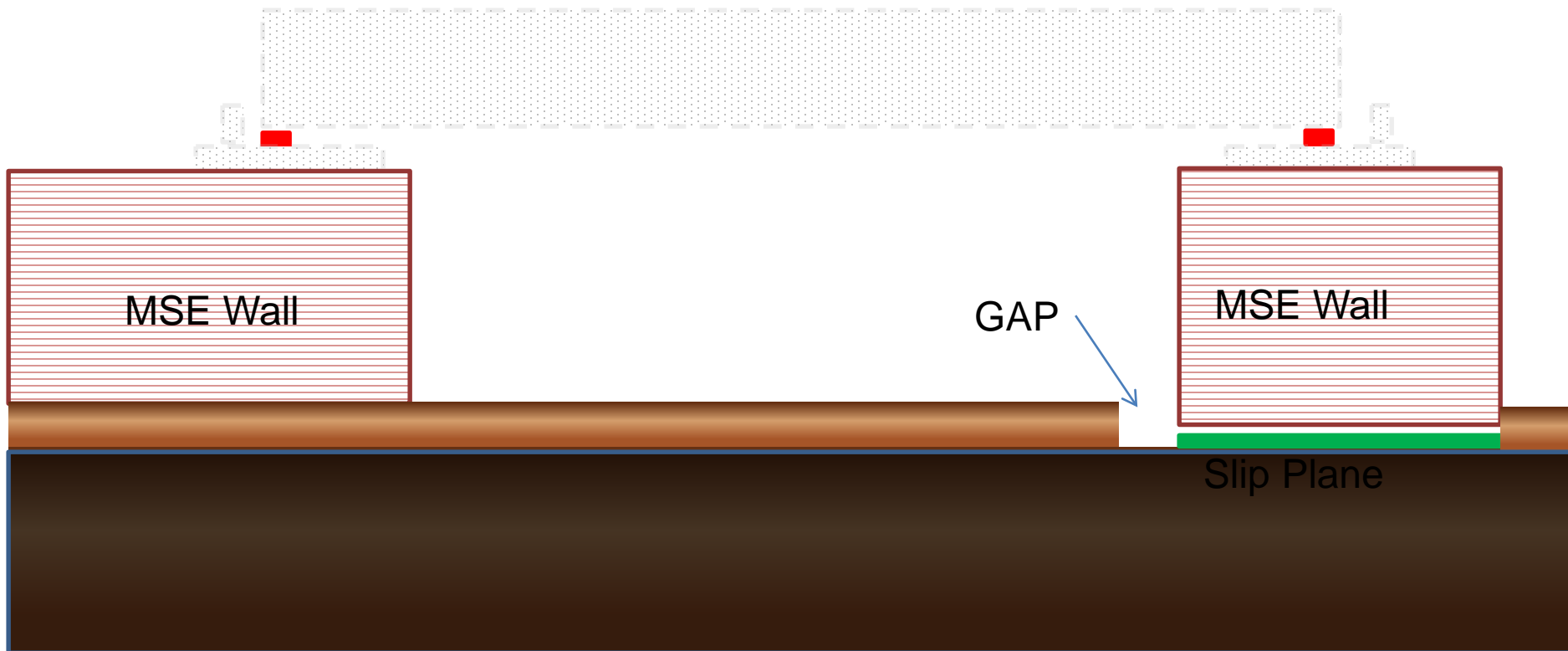


# Fault Rupture – SEAT LENGTH

- ABUTMENT 2 - Sufficient seat length is provided



# SUBSTRUCTURE



# Substructure - MSE Abutments

## ABUTMENT 1



# Substructure - MSE Abutments

## ABUTMENT 2





# Substructure - MSE Abutments

## BEHAVIOR OF REINFORCED WALL SYSTEM DURING THE 1999 KOCAELI (IZMIT), TURKEY, EARTHQUAKE

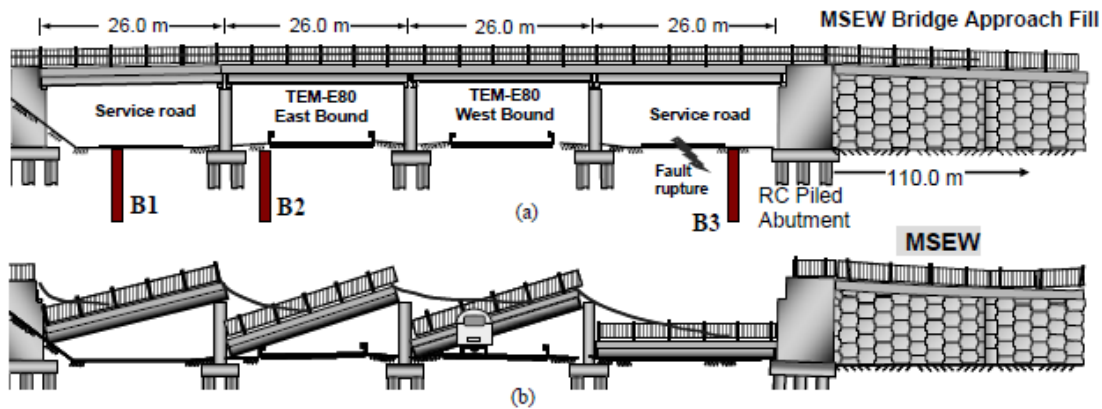


Fig. 2. The bridge overpass at Arifiye and mechanical stabilized bridge approach fill walls (a) before, and (b) after the earthquake.

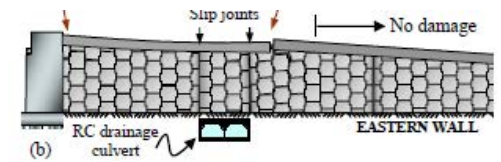
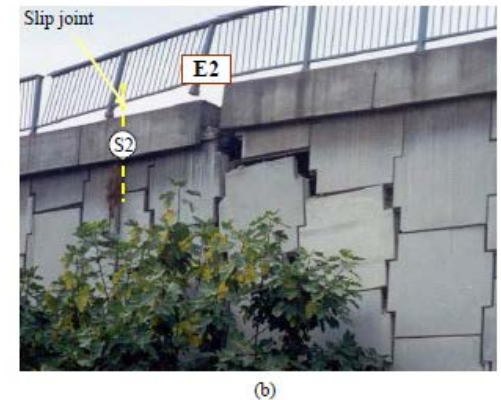
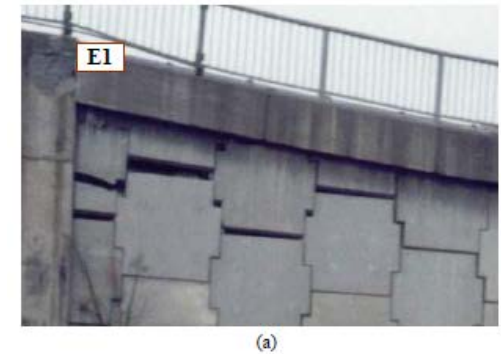


Fig. 5. MSE bridge approach (a) plan view of approach fill with damage-concentration; (b) schematics of eastern wall after the earthquake.



# Substructure - MSE Abutments

Postearthquake Reconnaissance Report  
on Transportation Infrastructure Impact  
of the February 27, 2010,  
Offshore Maule Earthquake in Chile

PUBLICATION NO. FHWA-HRT-11-030

OCTOBER 2011



Figure 268. Photo. Wall corner tilting outward at wall site 28B.

Thanks to: Professor Nicholas Sitar, UC Berkeley

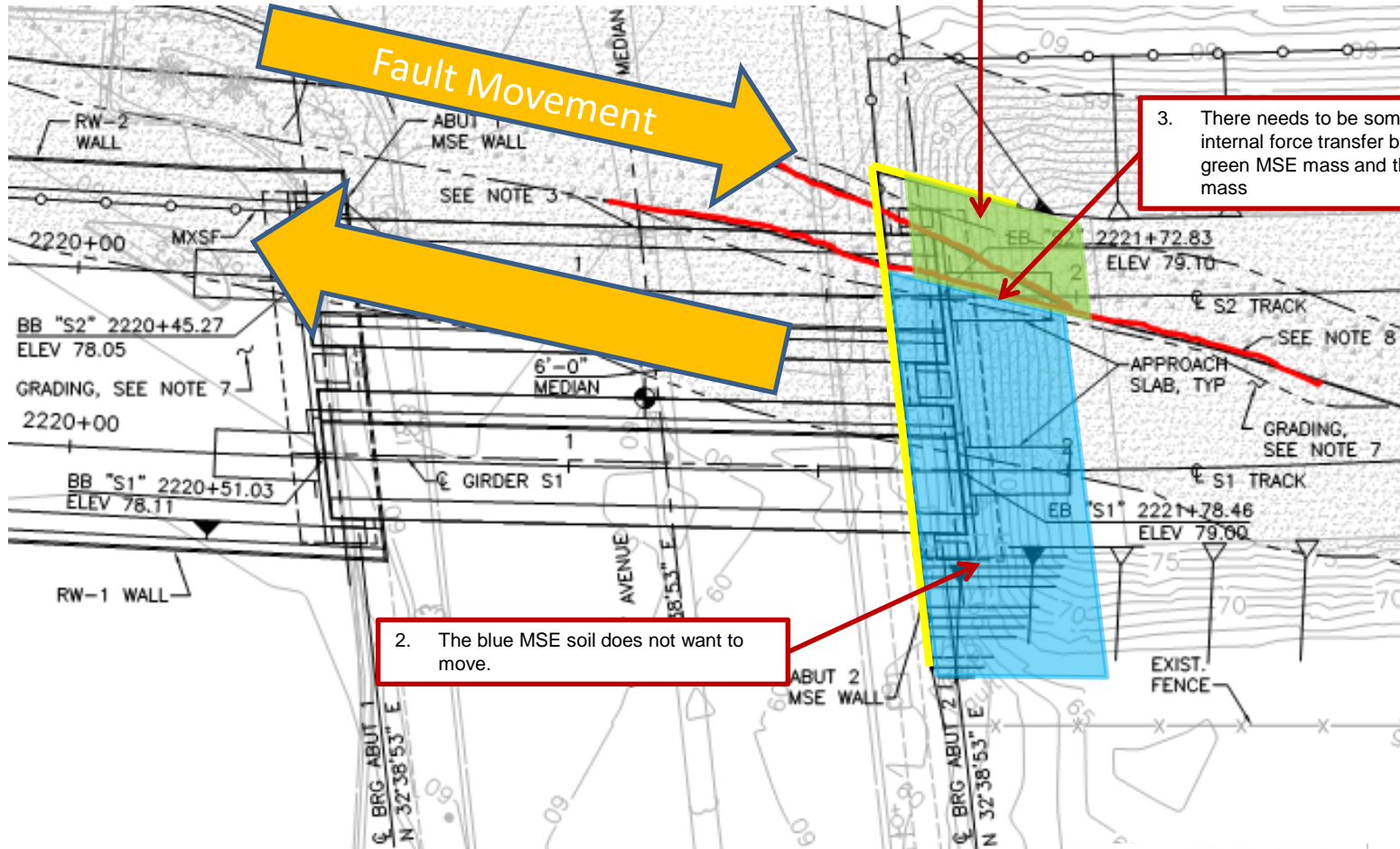
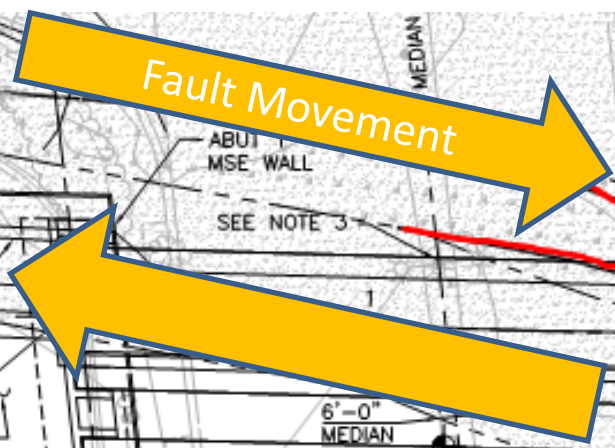
# Substructure - MSE Abutments

## Seismic Event

1. As the fault moves, the green MSE soil wants to move with it due to the forces being imparted on it by the slip plane friction.

3. There needs to be some kind of internal force transfer between the green MSE mass and the blue MSE mass

2. The blue MSE soil does not want to move.



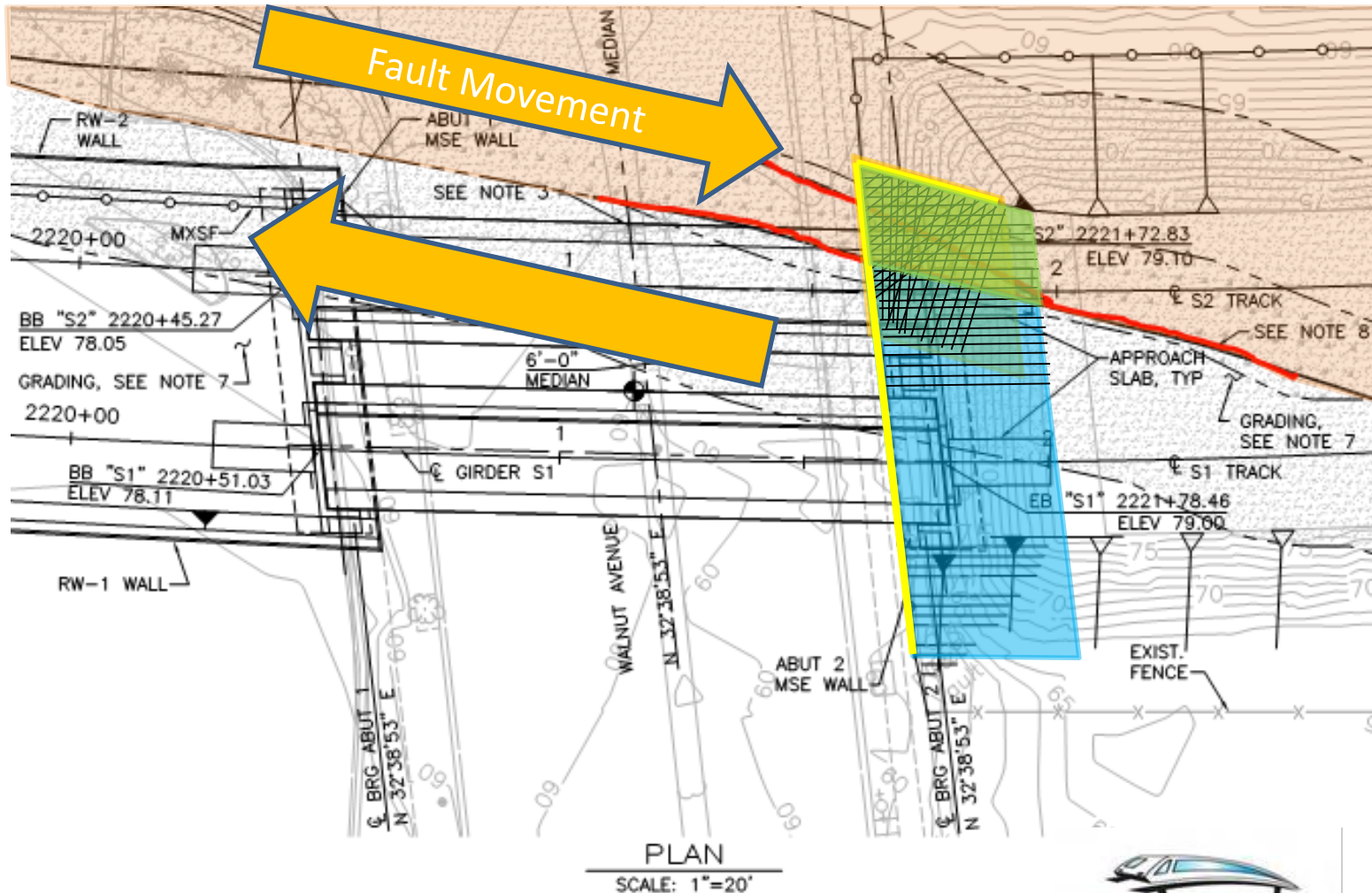
PLAN  
SCALE: 1"=20'





# Substructure - MSE Abutments

- Integrity of MSE mass is maintained
- Need to reduce friction between MSE mass and supporting soil



# Substructure - MSE Abutments

Slip Plane



# Substructure - MSE Abutments

## California Memorial Stadium Retrofit, Northern California

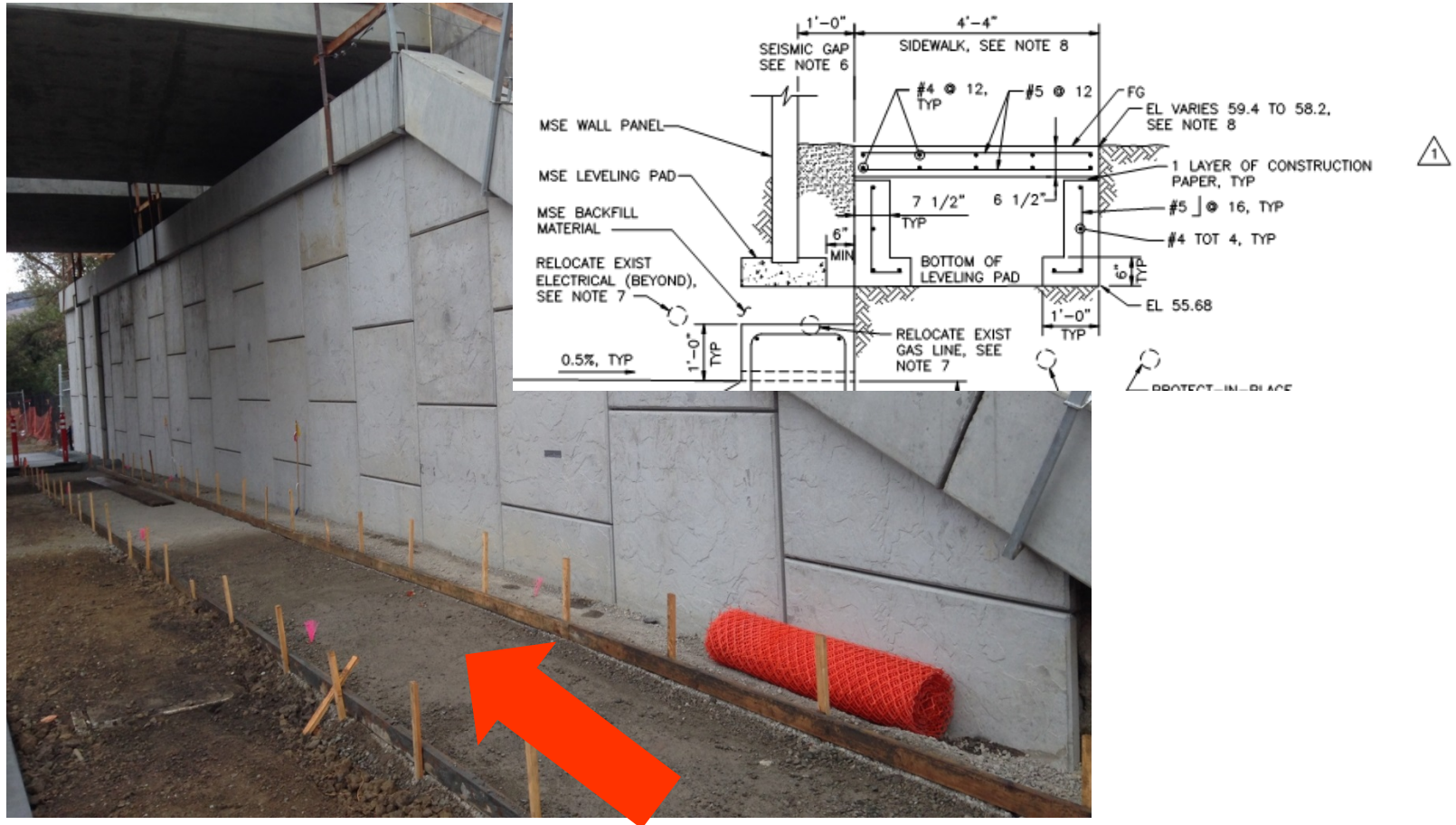


Hayward Fault

Courtesy of: Forell/Elsesser Engineers,  
Jonathan D. Bray, Phd, P.E. UC Berkeley

# Substructure - MSE Abutments

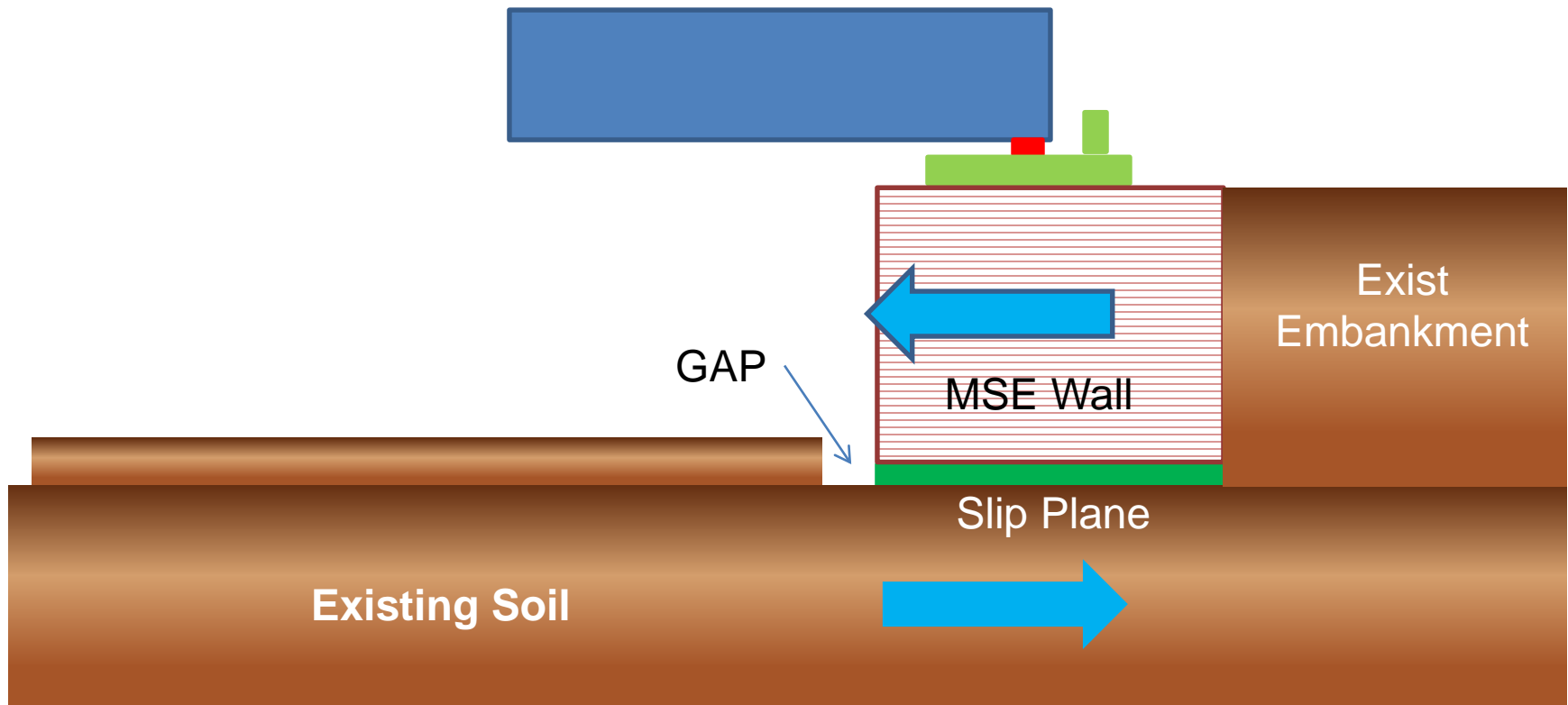
## ABUTMENT 2 – Seismic Gap





# MSE Abutments

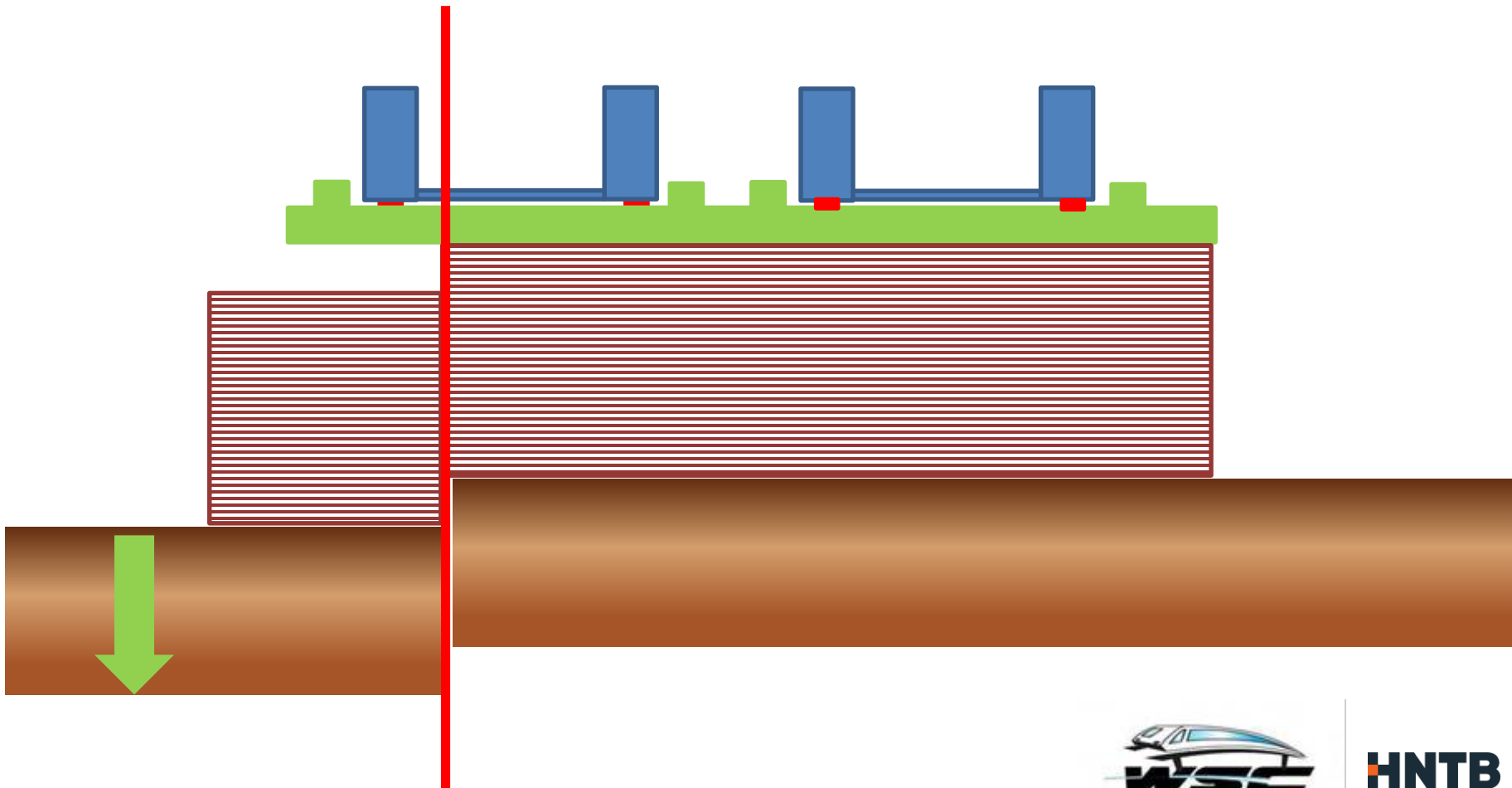
- To reduce sliding forces between the MSE block and the underlying soil, we added a slip plane. PVC or HDPE plastic membrane.





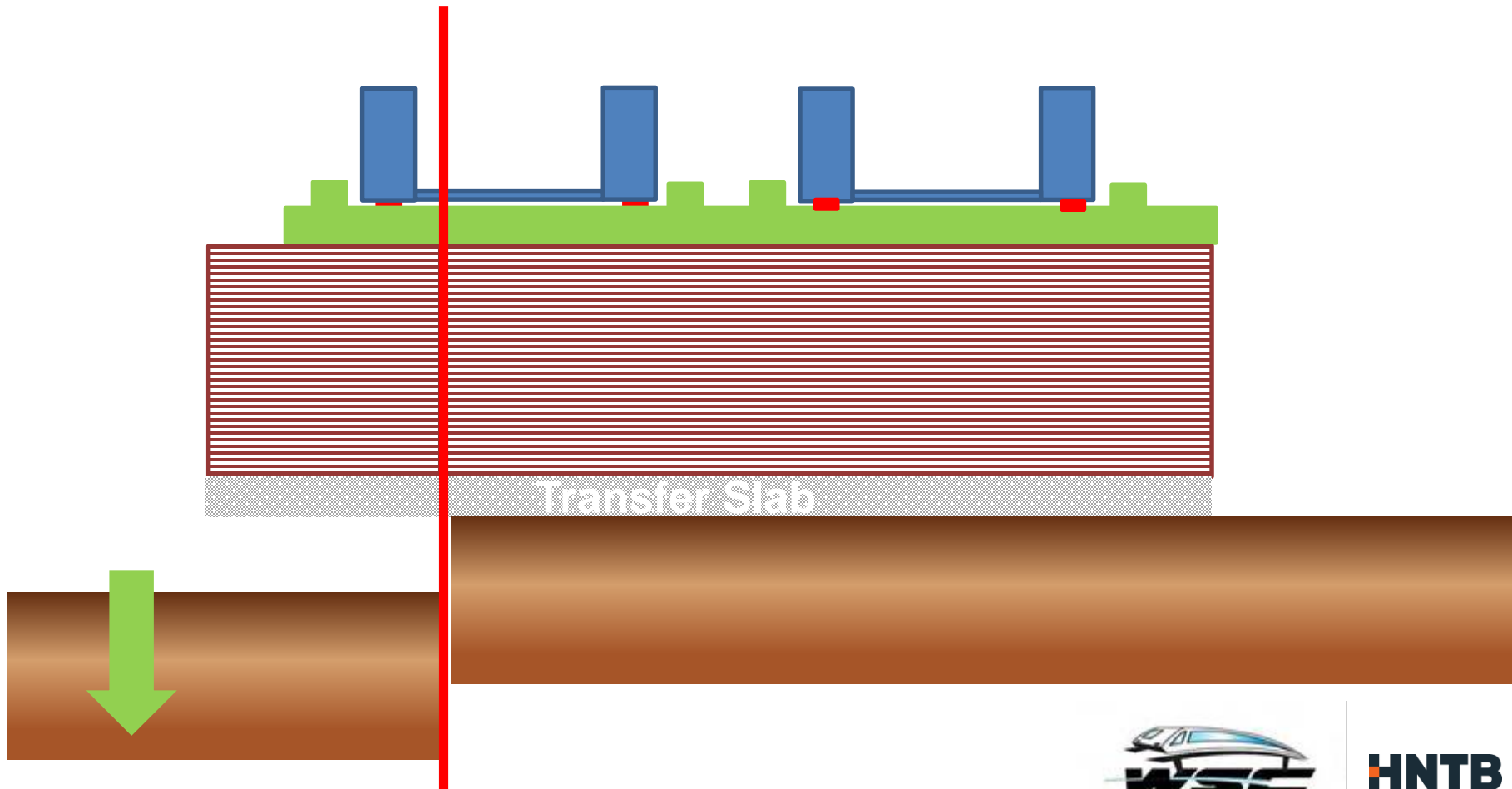
# MSE Abutments

- Vertical Displacement = 1.3 ft (16 inches)



# Substructure - MSE Abutments

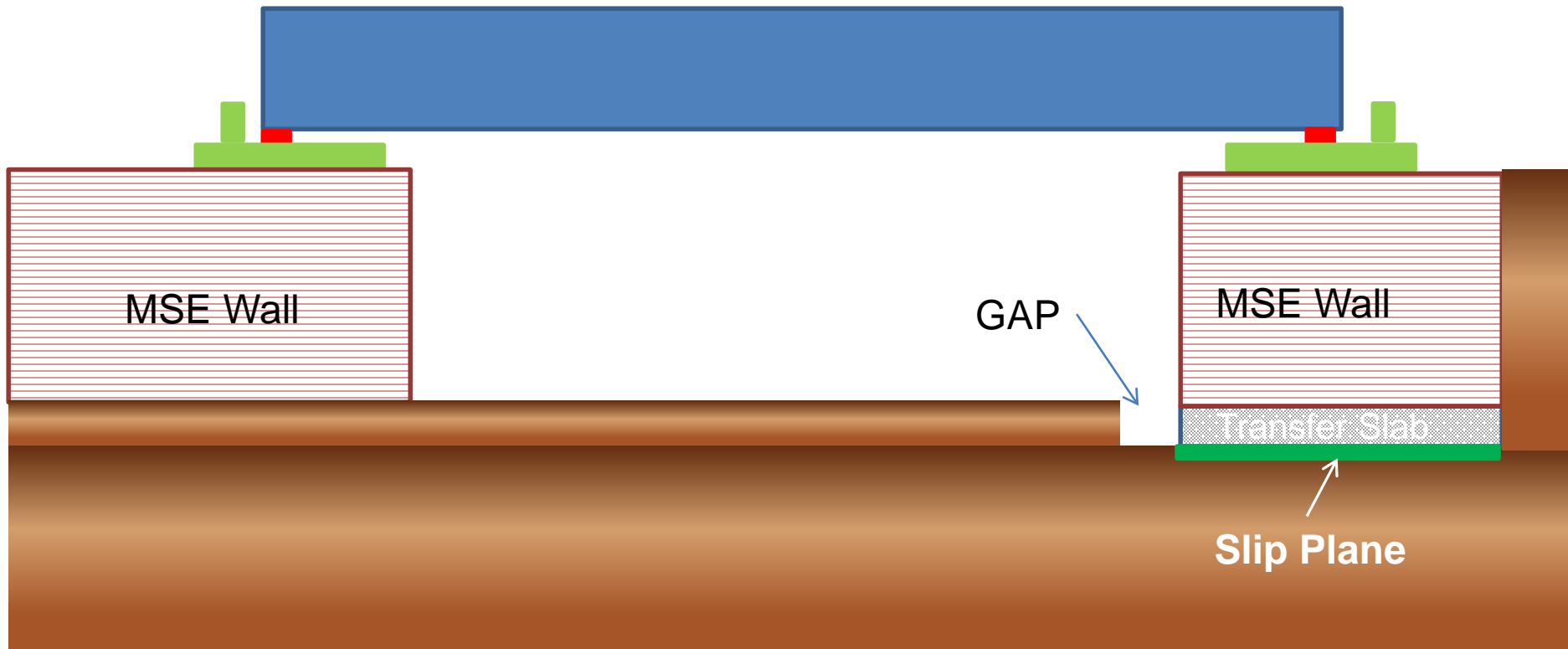
- Transfer slab supports MSE and Bridge when Vertical Rupture Occurs



# WALNUT AVENUE BRIDGE SEISMIC SYSTEM

Abutment 1

Abutment 2



MSE Wall

GAP

MSE Wall

Transfer Slab

Slip Plane

# Construction – ELEVATED FALSEWORK

- Limited Vertical Clearance
- Constructed on Elevated Falsework
- Lowered in Place using Strand Jacks



# Construction – ELEVATED FALSEWORK

## Straddle Lowering Bent





# Construction - LOWERED BRIDGE

- Limited Vertical Clearance
- Constructed on Elevated Falsework
- Lowered in Place using Strand Jacks



# Construction - REINFORCEMENT

## Reinforcement





# Construction - REINFORCEMENT

## Reinforcement – Web Reinforcement





# Construction - POST TENSIONING

## End diaphragms



# Construction - POST TENSIONING

## Anchor Blocks & Abutment Backwall



# Construction - POST TENSIONING

## Anchor Blocks & Lowering Frame





# Construction - AESTHETICS

- Architectural Treatment



# Acknowledgements

## CLIENTS

- BART
- Kiewit
- City of Fremont

## DESIGN TEAM

- Marcelo Vargas, PE
- Nick Murray, SE
- Erik Okada, SE
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- Larry Rowles, PE
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- Ali Rejaie, PE
- Hui Li
- Howard (Bud) Paige
- Scott Fehnel, PE
- David Wang, PhD, PE - PARIKH Consultants, Inc.
- Franciscus S. Hardianto, PE – Reinforced Earth Company



# Questions

