Landmark in the Making : Novel use of Post-tensioning in a Highly Curved Bridge



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- Hoffman Construction Company

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- Terry Lind – Hoffman Construction



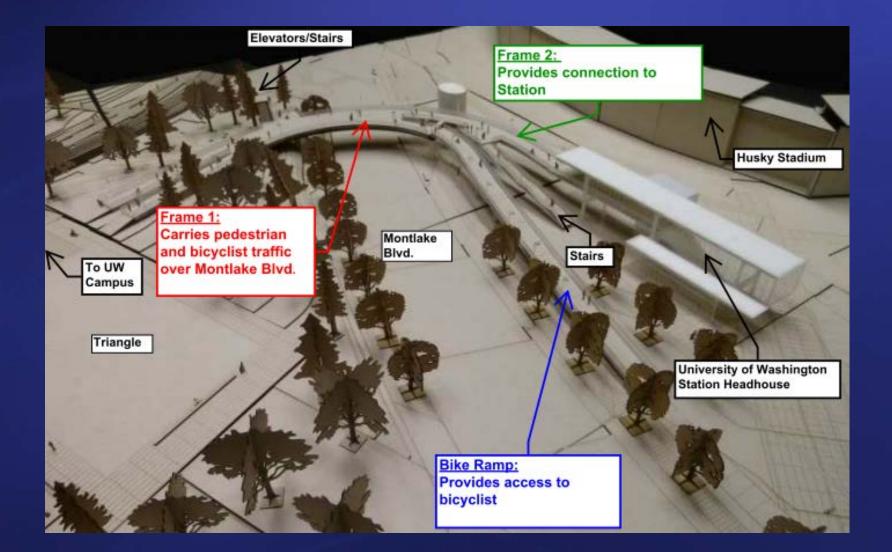
Part 1: Project Overview -Sound Transit U-link project



Part 1: Project Overview -Montlake Triangle Project



Part 1: Project Overview -<u>MTP Pedestrian Bridge</u>

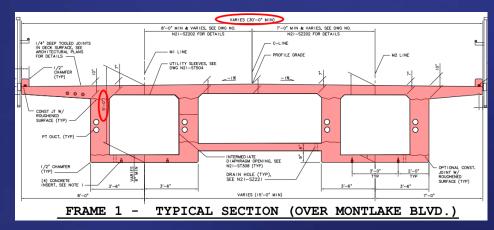


Part 1: Project Overview -Montlake Triangle Project cont.



Part 1: Project Overview -Bridge Type Selection

- CIP Post-tensioned double box
- 30' wide x 5' deep section spanning 130' ft over Montlake Blvd.
- Steel is more common for highly curved bridges.
- Durability and maintenance concerns with steel.
- CIP Post-tensioned Concrete \rightarrow shallow section, low maintenance, and complex geometry

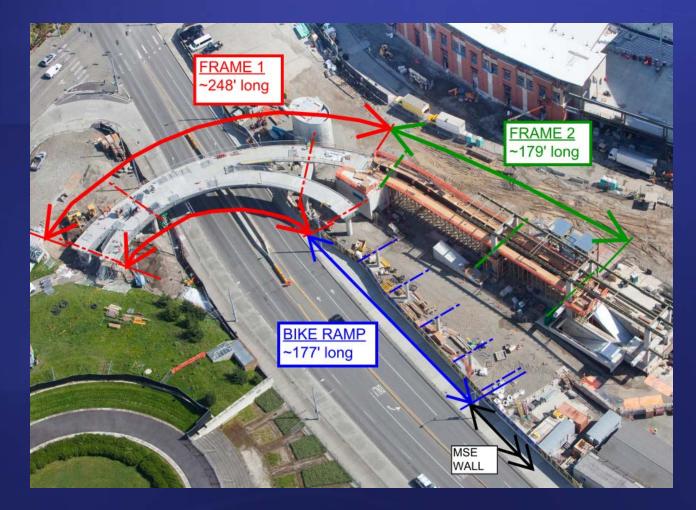






Part 2: Bridge Description – <u>Bridge Layout</u>

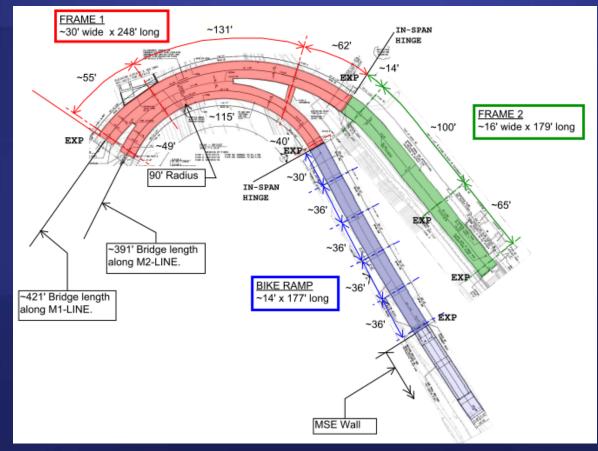
• Three segments: Frame 1, Frame 2, and Bike Ramp



Part 2: Bridge Description –

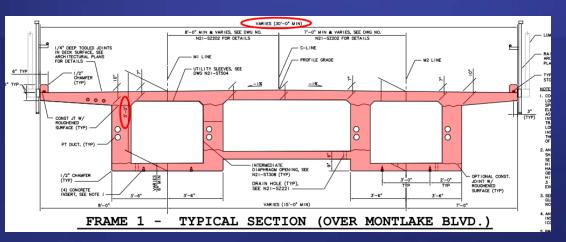
Bridge Layout cont.

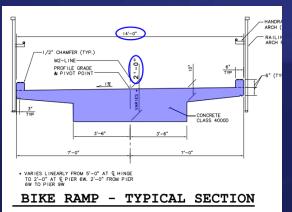
- Frame 1 3 spans, 130 ' max , short end spans, radial Pier layout, Exp at Pier 1 and hinges
- Frame 2 2 spans, 100' max span, exp at Headhouse frames and hinges
- Bike ramp 5 spans, 36' max span, exp at abutment and hinge.
- Hinges split bridge into more regular segments improving behavior and simplifying design

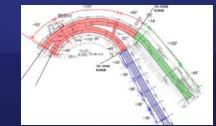


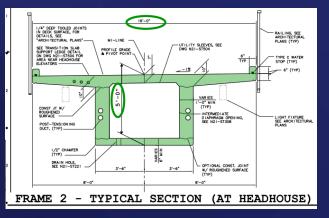
Part 2: Bridge Description – <u>Typical Sections</u>

- Three typical sections
- Frame 1 30 ' wide x 5 deep;
- Frame 2 16' wide x 5 deep (CIP PT Box)
- Bike Ramp 14 ' wide x 2'ft deep (RC Slab)





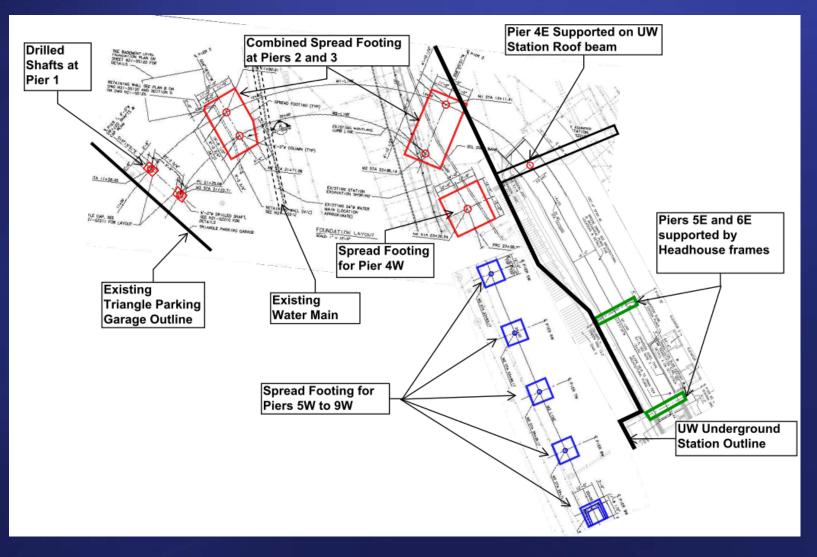




Part 2: Bridge Description –

Substructure

• Combination of drilled shafts, spread footings and the UW Station itself

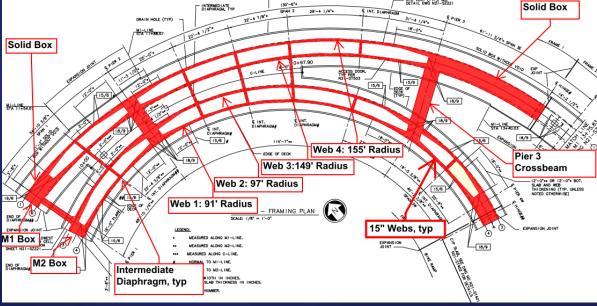


Part 2: Bridge Description – Framing Plan – Frame 1

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- M1 and M2 single boxes run together and separated along the bridge
- Pier 1 and Pier 3 crossbeam tie boxes
- Radius varies from 91 ft to 155 ft
- Radial orientation of Piers
- •Solid box area



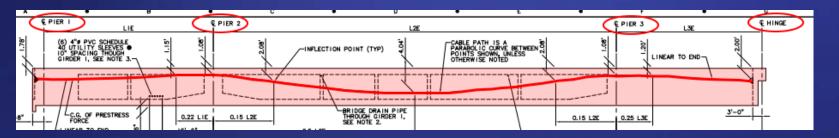


30'-6"



Part 2: Bridge Description – PT Layout – Frame 1

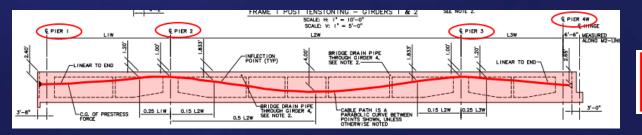
- PT Layout for each web is standard
- Jacking force varies per web
- Radial piers minimize in-plane horizontal forces due to PT





M1 Box

Web 1: 46-0.6"strands, 2000 kip Pjack Web 2: 44-0.6"strands, 1900 kip Pjack



<u>M2 Box</u> Web 3: 31-0.6"stands, 1350 kip Pjack Web 4: 29-0.6"strands, 1250 kip Pjack

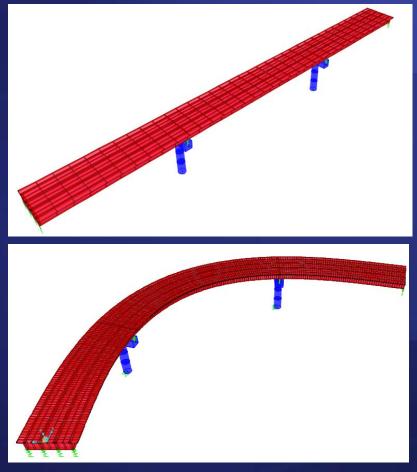
Part 3: Bridge Constraints

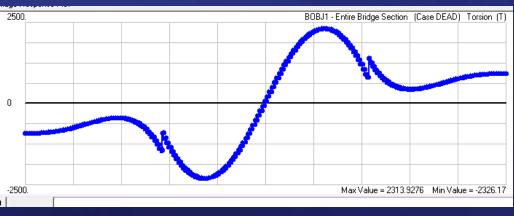
- 1. Highly Curved Plan Geometry
- 2. Limited Vertical Clearance
- 3. Long-Term Durability Requirement
- 4. Challenging Span Arrangement
- 5. Interface with Station Structures

Part 3: Bridge Constraints – *Highly Curved Plan Geometry*

1. Torsion induced by curved geometry;

2. Different bending moment distributions compared with straight bridge;





Part 3: Bridge Constraints – *Limited Vertical Clearance*

- Bridge crosses over Montlake Blvd.
- Post-tensioned box girder allows shallower superstructure depth;
- Depth vs span = 5ft / 130.5ft = 0.038= 1/26;



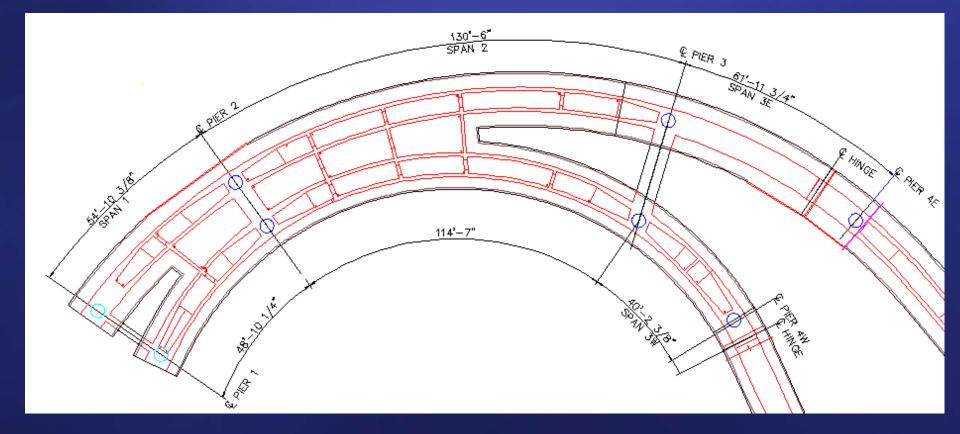
Part 3: Bridge Constraints – <u>Long-term Durability</u>

- High long-term maintenance cost for steel structures;
- Concrete structure, especially prestressed concrete structure is durable;

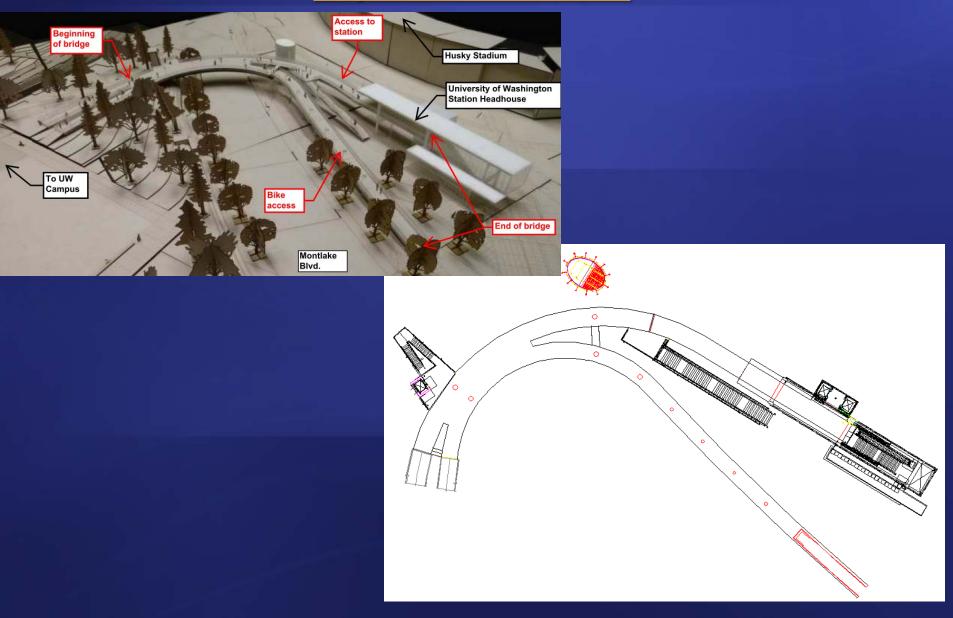


Part 3: Bridge Constraints – Challenging Span Arrangement

Uplifting at the bearing becomes a concern due to unbalanced span arrangement



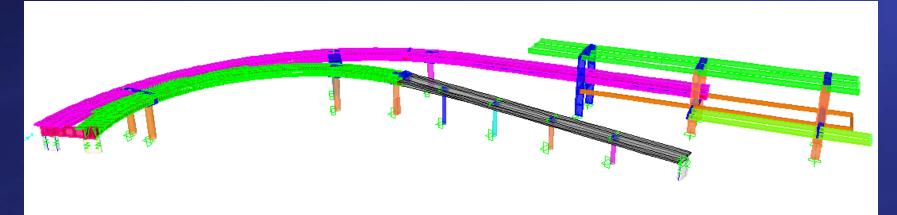
Part 3: Bridge Constraints – Interface with Station Structures

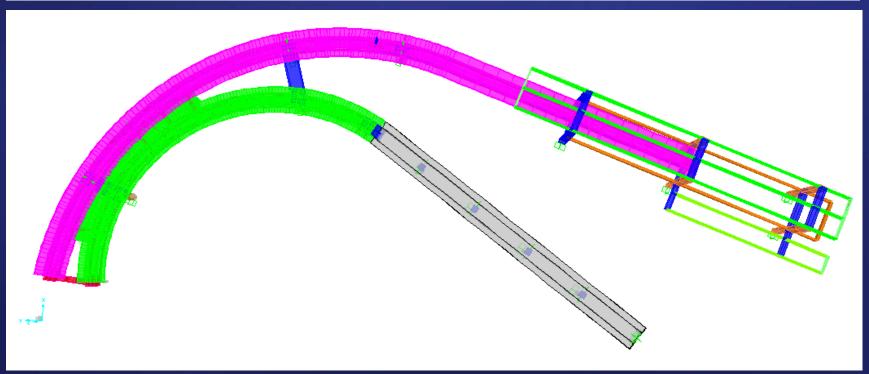


Part 4: Bridge Analysis and Design Challenges

- 1. Global FE Model
- 2. Frame Arrangement
- 3. Bearing Uplifting
- 4. PT Local Effect
- 5. PT Jacking Sequence

Part 4: Bridge Analysis and Design Challenges <u>— Global FE Model</u>





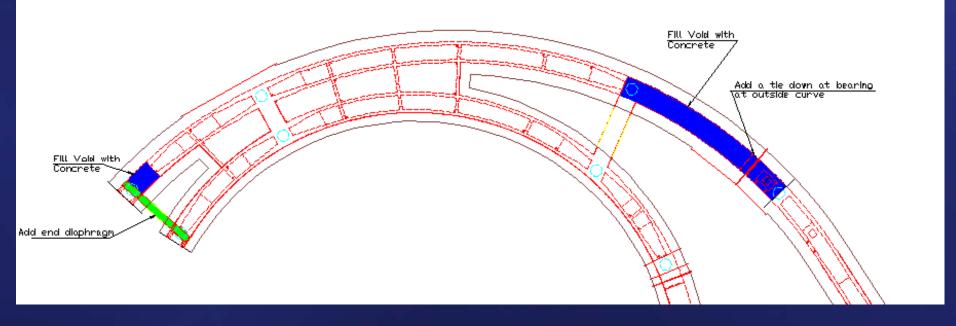
Part 4: Bridge Analysis and Design Challenges <u>– Frame Arrangement</u>

- 1. In-span hinges;
- 2. One pier column is directly founded on station roof;
- 3. Two pier columns are supported by headhouse structure;
- 4. No longitudinal expansion joint



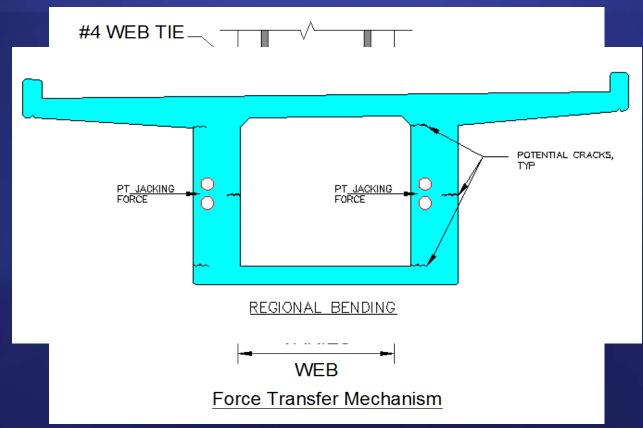
Part 4: Bridge Analysis and Design Challenges <u>— Bearing Uplifting</u>

- 1. Filling up the box of side spans
- 2. Use end diaphragm
- 3. Provide a tie down

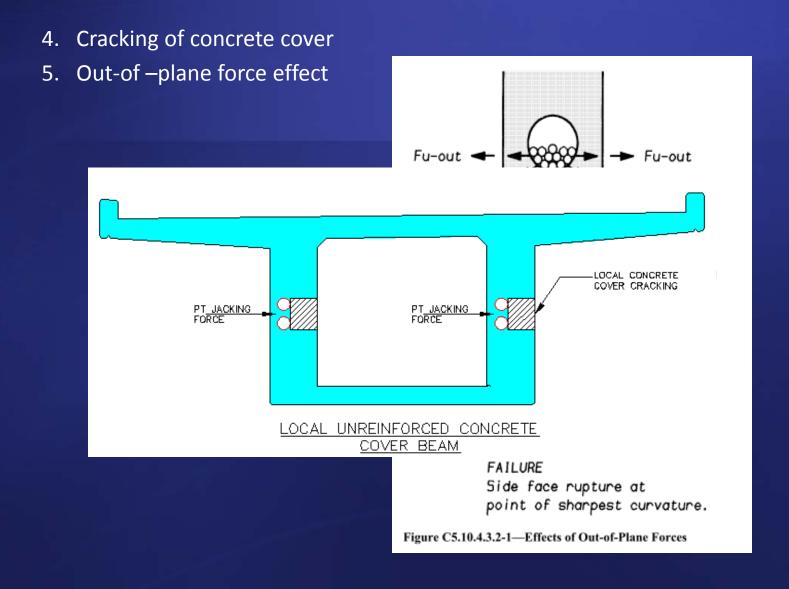


Part 4: Bridge Analysis and Design Challenges <u>– PT Local Effect</u>

- 1. In-plane force
- 2. Strut-and-tie method
- 3. Regional bending

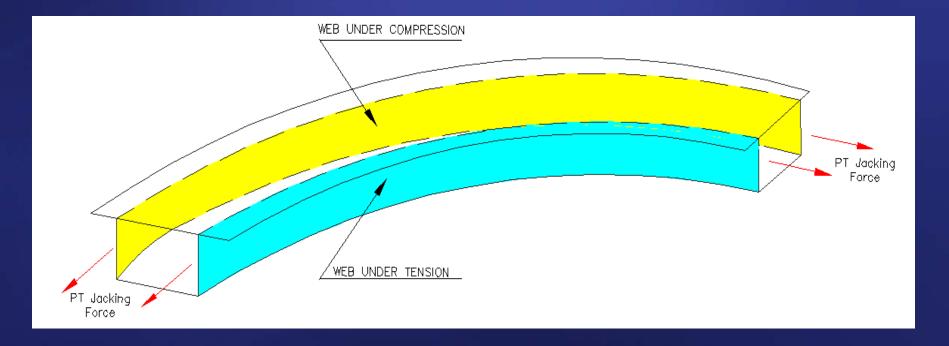


Part 4: Bridge Analysis and Design Challenges <u>– PT Local Effect</u>



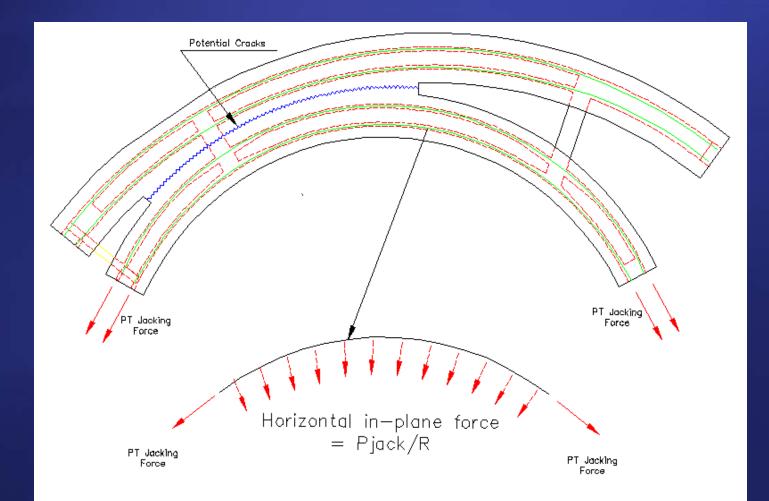
Part 4: Bridge Analysis and Design Challenges <u>— PT Jacking Sequence</u>

1. Tension on the inside of the curve and compression on the outside of the curve.



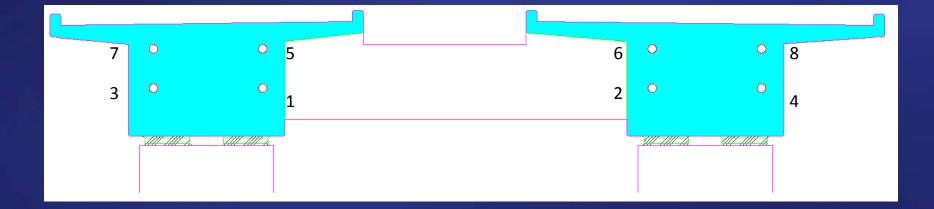
Part 4: Bridge Analysis and Design Challenges <u>— PT Jacking Sequence</u>

2. Differential lateral force can cause transverse tension in the slabs;

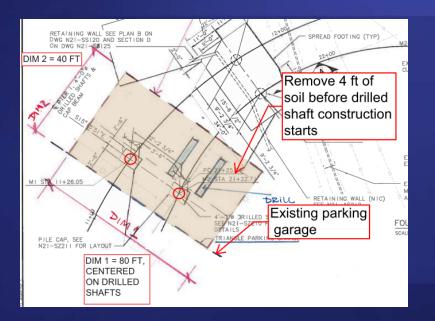


Part 4: Bridge Analysis and Design Challenges <u>– PT Jacking Sequence</u>

Final PT Jacking Sequence Selected :

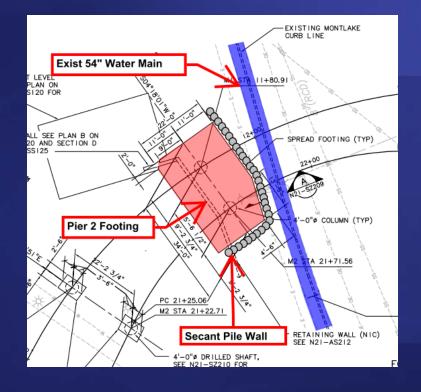


Part 5: Construction Challenges -Pier 1 Drilled Shafts next to Existing Garage



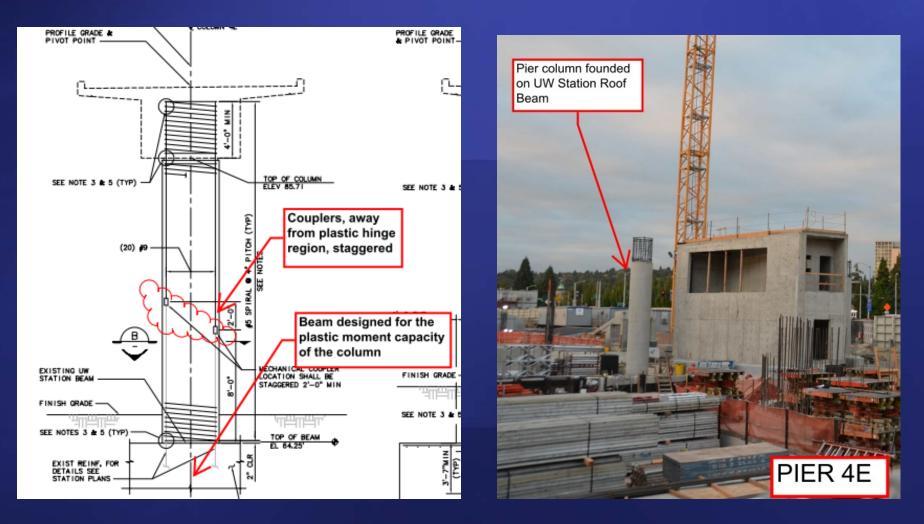


Part 5: Construction Challenges -Pier 2 Footing Construction next to Water Main

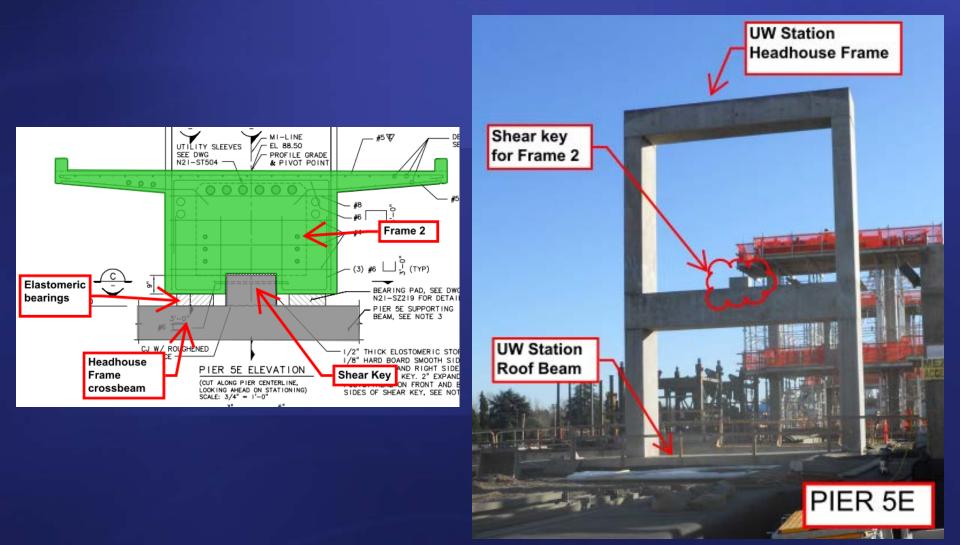




Part 5: Construction Challenges -Pier 4E Column on UW Station Roof beam.



Part 5: Construction Challenges -Bridge connection to UW Station Headhouse



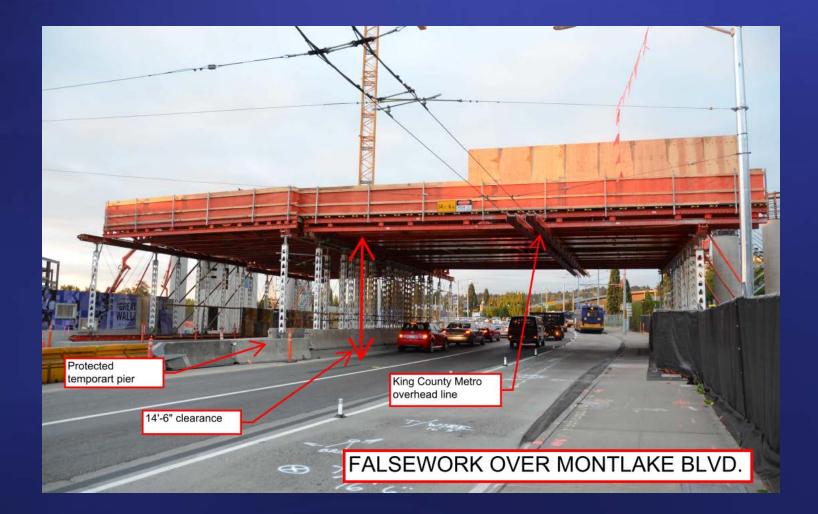
Part 5: Construction Challenges -<u>Formwork</u>



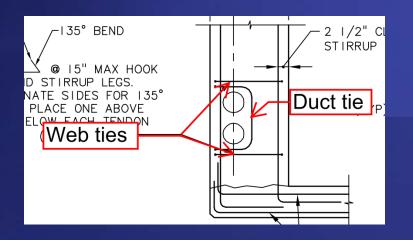


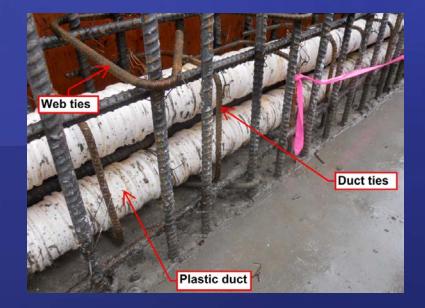


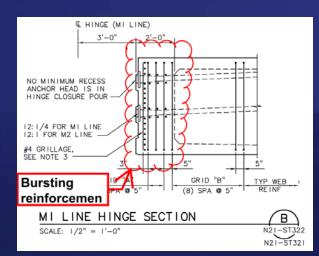
Part 5: Construction Challenges -<u>Falsework</u>

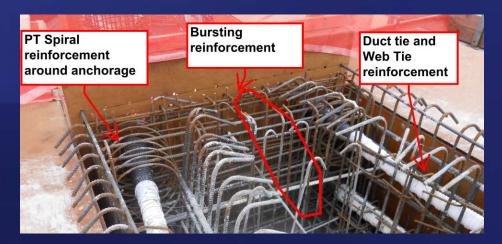


Part 5: Construction Challenges -<u>Post-tensioning</u> construction

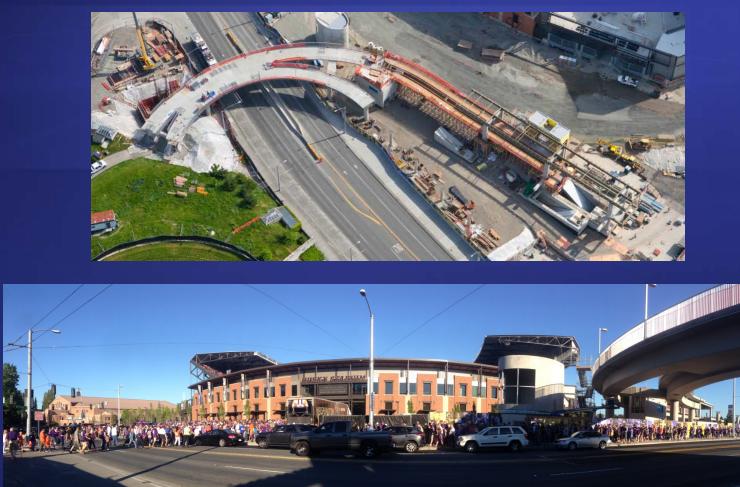








Questions



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