



**Associated  
Engineering**

*GLOBAL PERSPECTIVE.  
LOCAL FOCUS.*

## Western Bridge Engineers

*September 2013*

### Accelerated Bridge Construction – Fraser Heights Bridge



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# Outline

- Introduction
- Problem Statement
- Solution
- Seismic Design
- Construction
- Conclusion
- Acknowledgements

# Introduction

- Part of Port Mann Highway Project – a 37 km freeway widening: Vancouver – Langley, BC
- \$2.5 Billion contract between the Province of BC and Design/Build Contractor Kiewit/Flatiron
- 450 m four-lane wetland crossing for the new South Fraser Perimeter Road
- – *Construction equipment prohibited in wetland*
- – *Total “footprint” restricted to 45 m<sup>2</sup>*

# Problem Statement

- Wetland comprises highly compressible soils
- Challenging seismic performance requirement
- Construction schedule – limited to 12 months
- 4–5 m deck height results in stiff substructure
- Long segment lengths are desirable to minimize expansion joints and seismic interfaces, and improve vehicle ride quality
- – *Innovative solution required*

# Location

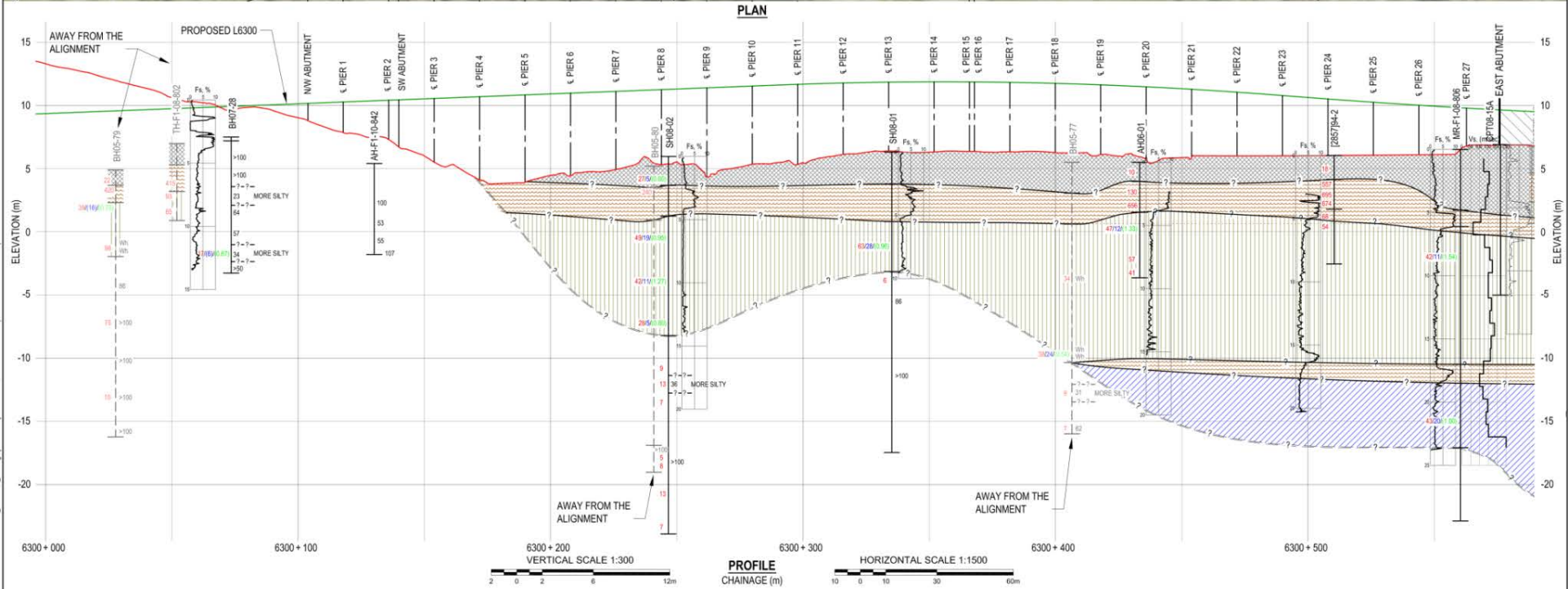
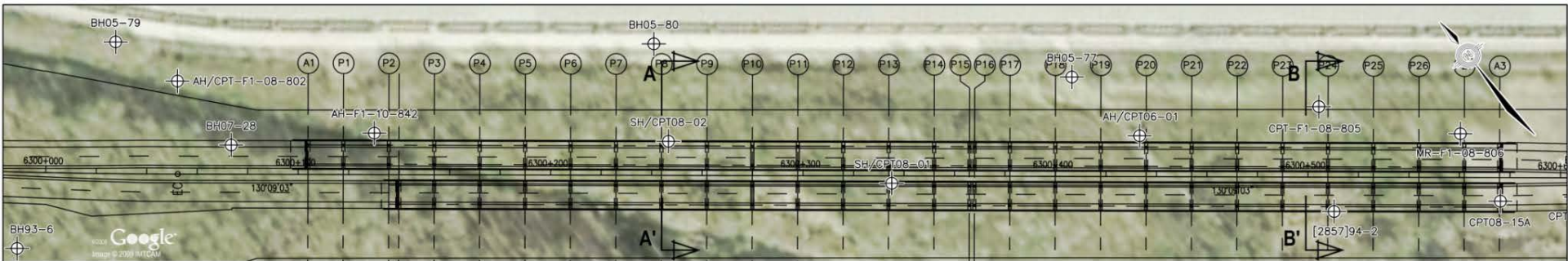


*North Surrey, BC*

# Solution

- Final design *very similar* to bid design
- Parallel 436 m and 472 m long, 11 m wide superstructures
- Typical span length 18 m – end spans 14 m
- Bents: two steel pipe piles per trestle supporting box-section steel cap beams
- 302 identical reversible full-depth 250 mm precast deck panels – 3 m long, 11 m wide
- 100 mm membrane and asphalt wear surface

# Wetlands and soil profiles



**LEGEND**

- Existing Fill
- Sand/Silty Sand/Sandy Silt
- Peat
- Marine Clay
- Alluvial Overbank Silt - Organic Silt, Bedded with Sand, Clay
- Approximate Top Boundary of Dense Glacial/Pleistocene Deposit

**NOTES**

- 7 - Numbers in red indicate measured moisture content
- 36 - Numbers in black indicate measured SPT N values
- 28 - Numbers in blue indicate plasticity index as shown on borholes
- 98 - Numbers in green indicate liquidity index

**NOTES**

- All subsurface geological contacts are approximate or inferred.
- Geological profile drawn along centerline of bridge. Surface of dense glacial material dips from South to North below bridge.
- See Figure 3 for Cross-Sections A-A' and B-B'.

CLIENT

**H5M** Hatch Mott MacDonald

EBA Engineering Consultants Ltd. eba

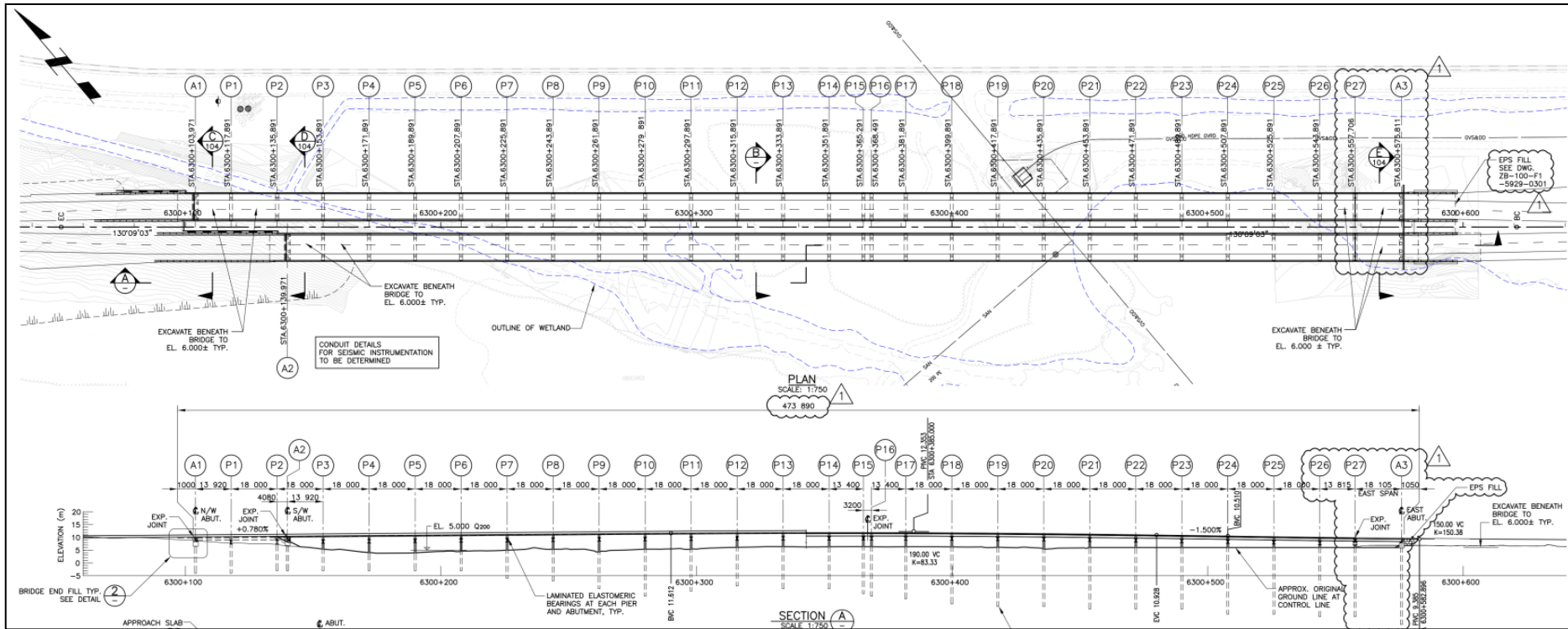
PORT MANN - HIGHWAY 1  
FRASER HEIGHTS CONNECTOR

FRASER HEIGHTS BRIDGE BOREHOLE LOCATION  
PLAN AND INTERPRETED STATIGRAPHY

PROJECT NO.	DWA	CHK	REV
V13101049.104.C	SF	BH	1
OFFICE	DATE		
VANC	October 19, 2010		

Figure 2

# Bridge Configuration



**Eastbound: 436 m long – 26 spans**  
**Westbound: 472 m long – 28 spans**



# Underside of Superstructure



*Piles, caps, girders, precast deck panels installed*

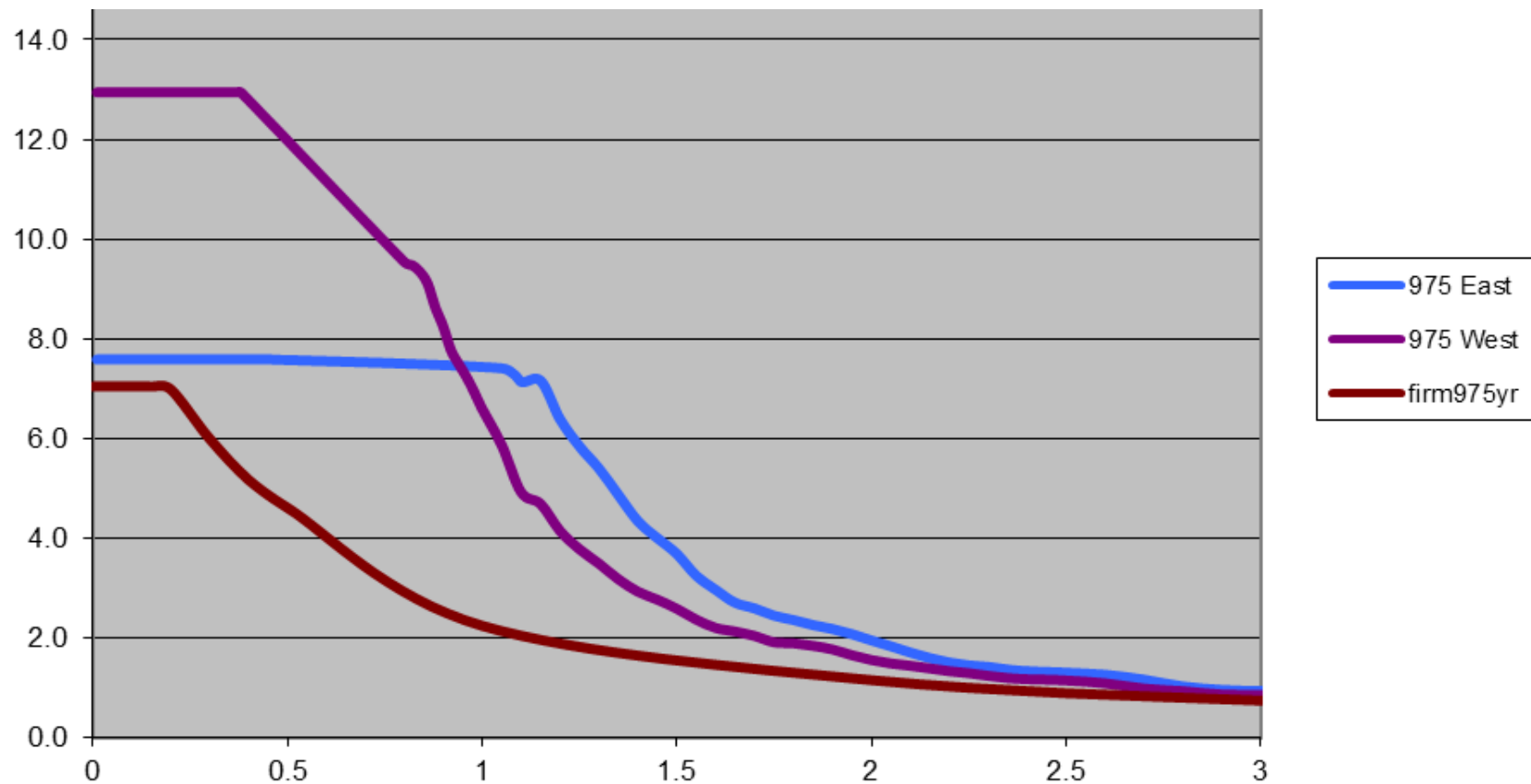
# Seismic Design

## ***Performance Criteria***

- Minimal damage and immediate use by emergency vehicles for 475-year RP event
- Significant damage, return to full service following repairs for 975-year RP event
- No collapse, non-repairable damage acceptable, for 2475-year RP or Cascadia subduction events
- ***Criteria readily achieved by design selected***

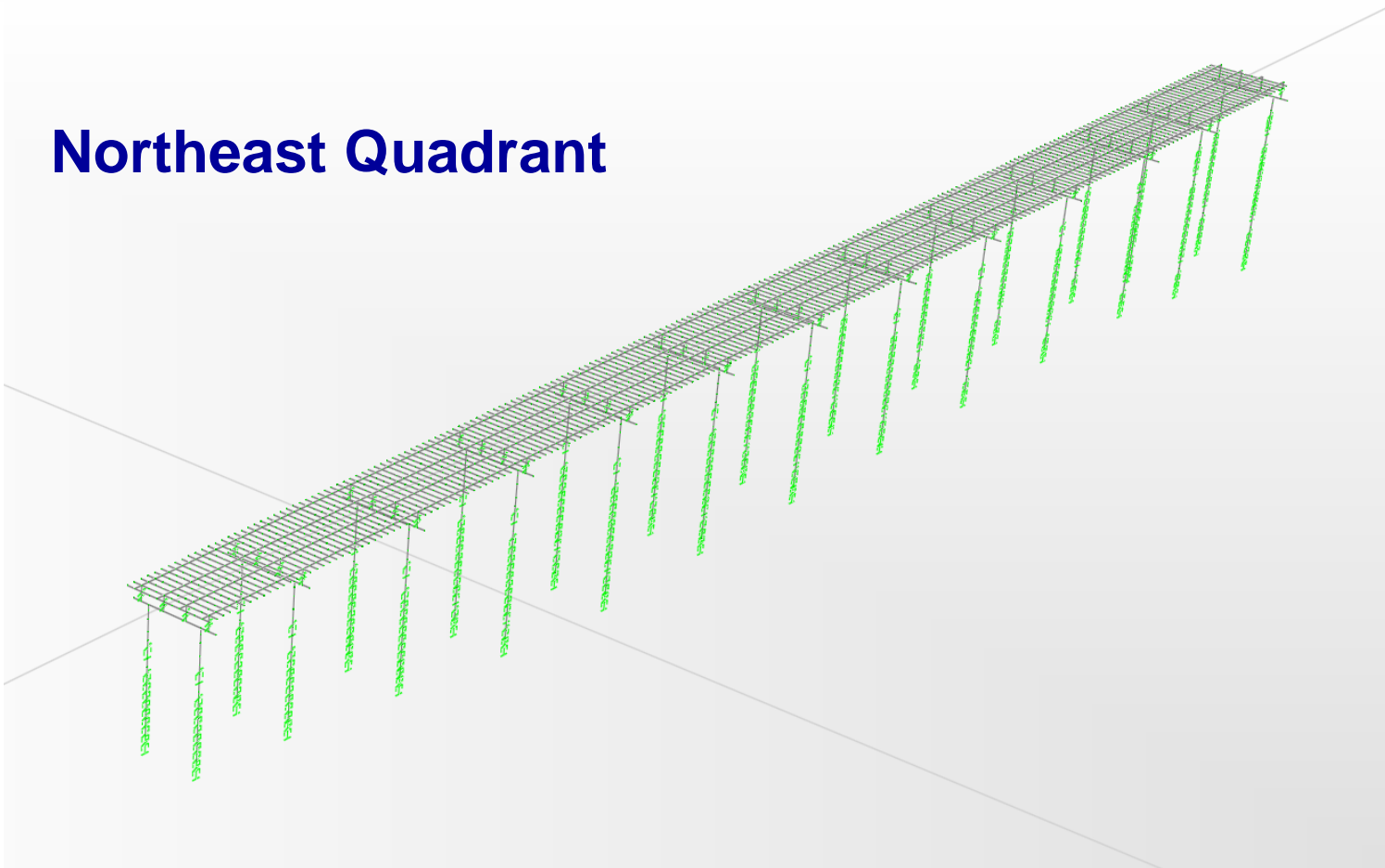
# Seismic Design

## Response Spectra

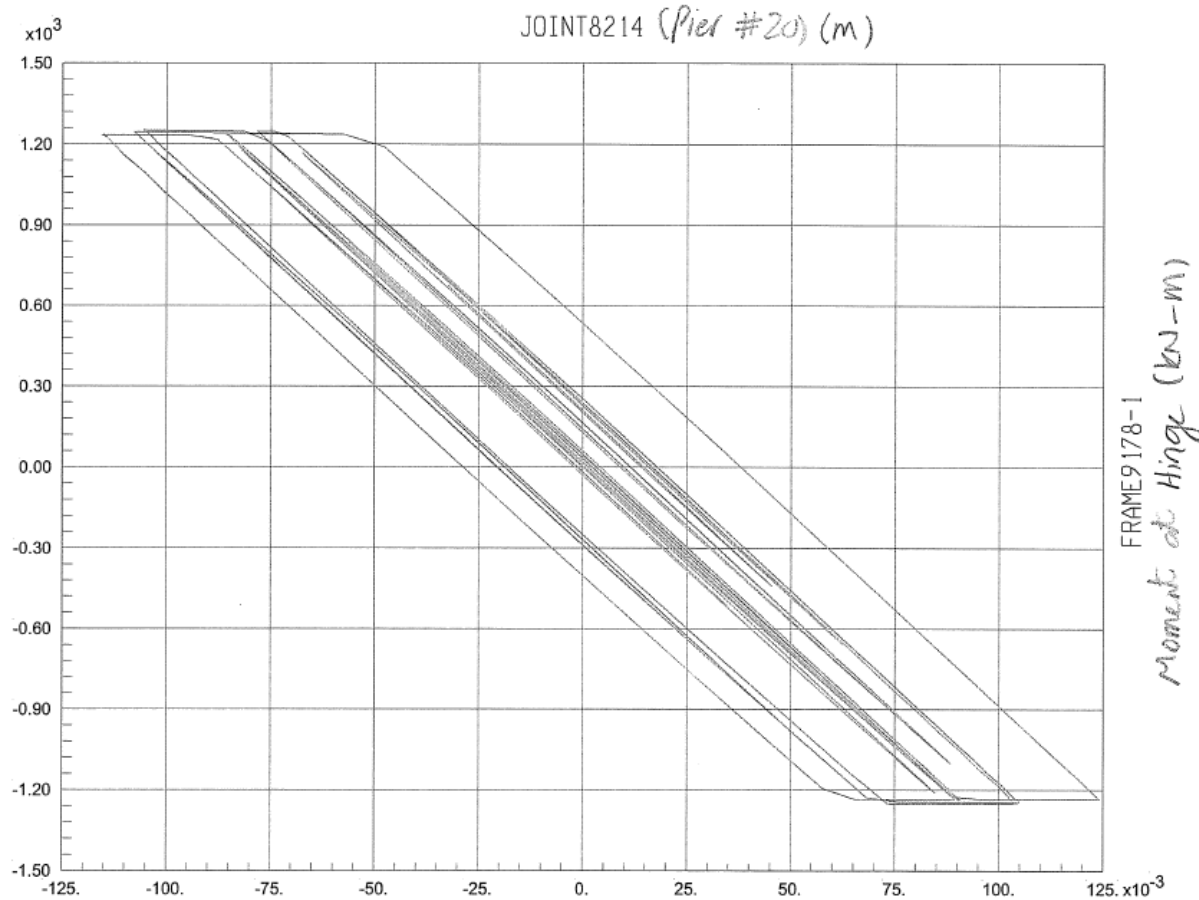


# Seismic Design – SAP Model

Northeast Quadrant



# Seismic Design – Plastic Hinges



**Hysteresis in piles**

# Seismic Design

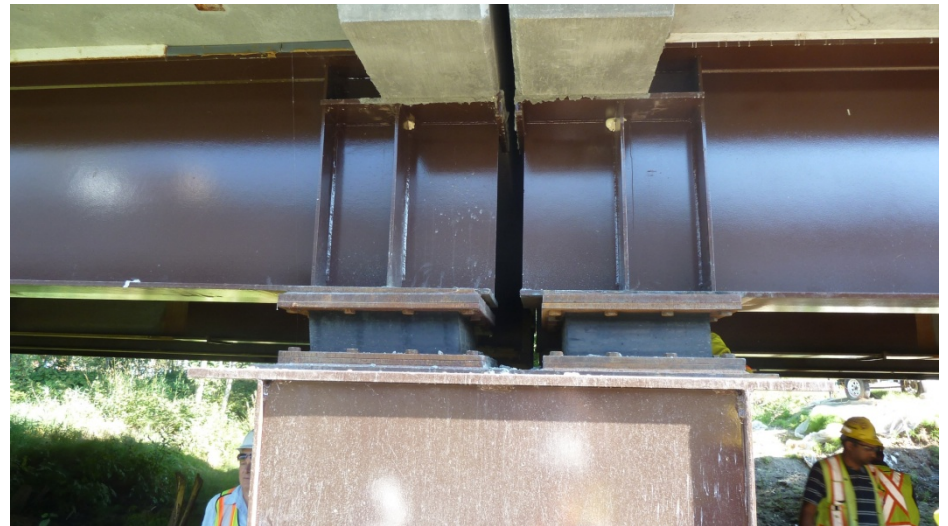
## *Isolated Superstructure*

- Base isolation accommodated thermal strains
- Segment lengths up to 250 m
- 224 bearings – only two types required
- Bearings: height – 144 mm; rubber thickness – 126 mm
- 975 year event displacement (non-linear time history analysis) – 109 mm max

# Isolation Bearings



Typical single-bearing support



Double-bearing support – west end-span

# Seismic Response – Periods

Mode	Natural Period (sec)	
	Westbound Bridge	Eastbound Bridge
Longitudinal	1.7	2.18
Transverse 1	1.49	1.67
Transverse 2	1.33	1.58



# Seismic Response – Displacement

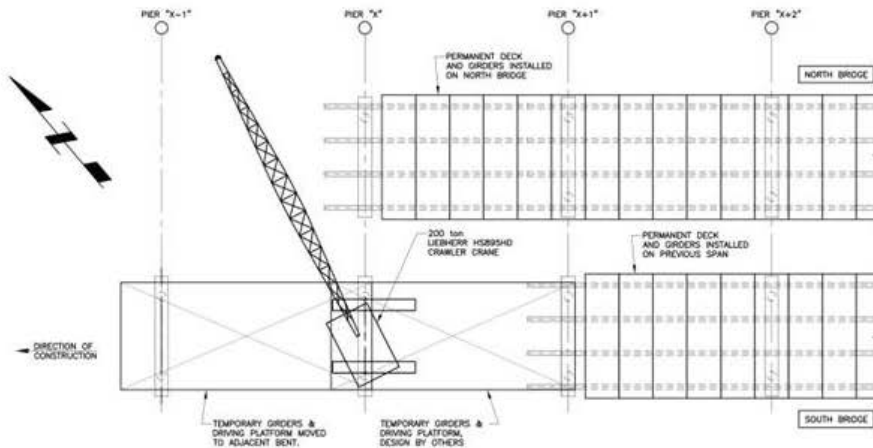
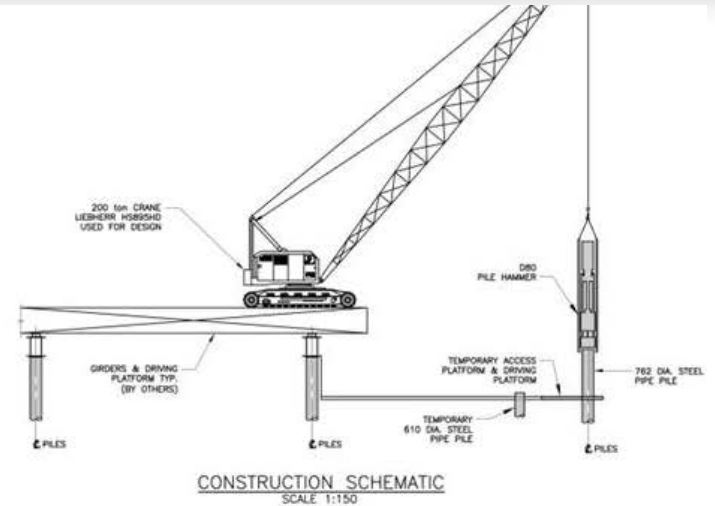
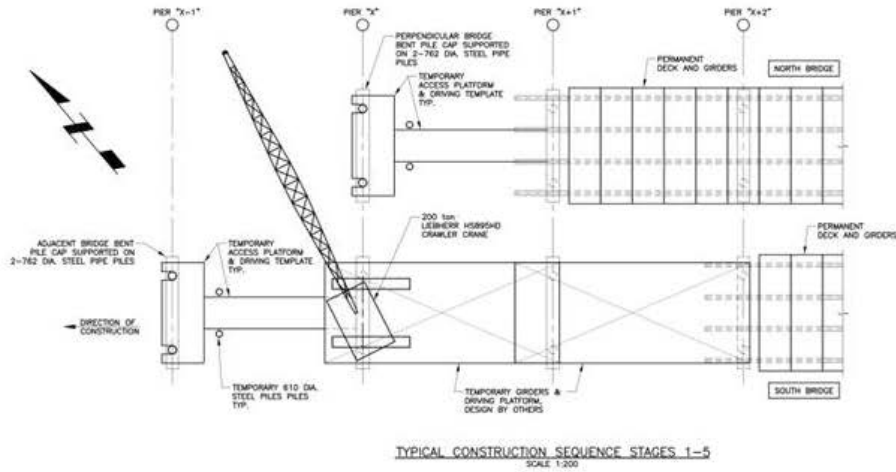
Hazard	Displacement (mm) max.	W/B Bridge	E/B Bridge	Expected Yielding
10% in 50 years	Transverse	154	165	None
	Longitudinal	109	146	
5% in 50 years	Transverse	212	227	Several cap/pile joints
	Longitudinal	154	211	
2% in 50 years	Transverse	341	363	Several cap/pile joints Several piles (minor)
	Longitudinal	253	325	

# Construction

## ***Top-down construction method***

- 250 tonne erection crane
- Two movable work platforms on WB trestle
- Crane drives piles, installs pilecaps, places concrete in piles and cap joints, erects girders, and installs deck panels
- Materials delivered over EB and WB trestles
- Girders and panels acting non-compositely designed for one lane of highway loading

# Top-down construction method



# Crane – Work Platform



# Construction – Piles

- 112 open-ended steel pile piles
- 762 mm diameter x 19 mm wall thickness
- SLS – 2100 kN; ULS – 2700 kN
- Required geotechnical capacity – 5400 kN
- APE D80 hammer: 268 kNm; 31.4 tonnes
- Top 11.5 m concrete-filled; reinforcing ratio: 2.6%
- Rebar cage penetrates into joint in pile cap
- Rebar flexural yielding capacity-protects pile cap

# Crane – Pile Driving



# Steel Pilecap Installation



*Joint formwork in place*

# Pilecap Joint





# Pilecap Joint



*Pile rebar cage projects into pilecap joint*

# Accelerated Bridge Construction

## *Time frame*

- 52 weeks available for construction
- Virtually all components critical path
- Span cycle:
  - Advance work platform and crane
  - Drive piles – clean out, fill with concrete
  - Place steel pier cap and cast pile/cap joints
  - Install bearings, girders and precast panels
- ***10-day average span-cycle achieved***

# Challenges – Pile Installation

- East abutment piles 45 m long (35 m anticipated)
- Several piles encountered boulders:
  - Full-height HP 360x174 driven inside pipe to by-pass boulder (3 piles)
  - Shallow boulder excavated (west end)
- Six west-end piles drilled-in from outside wetland to achieve required embedment
- Final total pile length: 2306 m (108% of estimate)
- ***All 112 piles successfully installed***

# Challenges – Pile Installation



*Pile tip damage*

# Girder Erection



***Girders lifted in braced pairs***

# Deck Panels



*All panels identical and reversible*

# Deck Panel Joints



- ***350 mm wide joints***
- ***Suspended forms***

# Deck Panels – Pockets



***Clusters of thirteen 22 mm studs***



# Watercourse Crossing



***Bridge design accommodates existing wetland drainage***

# Completed Bridge



- *Minimal impact on wetland*
- *Open median reduces deck shadow effect*

# West Abutments



*Wire-faced  
MSE  
median wall*



# Deck Paving



*Installation of Protecto Wrap preformed waterproof membrane*



*Surfacing: 40 mm Open Graded Friction Course over 60 mm Hot Mix Asphalt*

# Finished Deck Surface



*Looking west from East  
Abutment*

# Conclusion

- Value achieved by:
  - Maximizing deck segment lengths and work repetition
  - All 112 pile sections were identical
  - Constant girder section – no butt welds
  - All 302 precast full-depth deck panels were identical
  - Non-composite bridge carries materials delivery trucks
  - Building tolerance on support location into design – *no corrective action required*

# Summary

- A custom solution was developed to solve a challenging bridging problem
- Bridging solution developed for bid design envisaged top-down, Accelerated Bridge Construction techniques
- Identical layout used for final design
- Work platform added to avoid 250 tonne crane loading on superstructure
- ***Contractor estimated cost at \$25 Million: \$2500 per m<sup>2</sup>***

# Acknowledgements

- ***Owner:*** BC Ministry of Transportation and Infrastructure
- ***Design-Build Contractor:*** A joint venture of Peter Kiewit Sons Co. and Flatiron Constructors Canada Limited
- ***Onshore Design:*** H5M – a joint venture of Hatch Mott MacDonald and MMM Group Limited.
- ***Bridge Designer:*** Brybil Projects, a subsidiary of Associated Engineering Group Ltd.
- ***Geotechnical Engineer:*** EBA Engineering Consultants Ltd.
- ***Bridge Installation Contractor:*** Gateway Infrastructure Group GP



*Thank You*

- for your attention



# Questions?

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