# Western Bridge Engineers' Seminar 2009



#### **COSUMNES RIVER PEDESTIRAN BRIDGE**

#### **RANCHO MURIETA, CA**



**Rancho Murieta Association and Community Services District** 

Presented by: James L. Foster, P.E



QUINCY Engineering, Inc.

Paul C. Gilham, P.E., S.E.



In cooperation with



**County of Sacramento** 

# **Project Location**





The project is located within the community of Rancho Murieta approximately 25 miles southeast of Sacramento, California. The new pedestrian bridge crosses the Cosumnes River to connect the north community to the south community.



# **Purpose and Need**



The purpose of this project was to connect the North and South residential area pedestrian trails in the Rancho Murieta Community.

The connection was required by a development specific Sacramento County ordinance.

The project was funded by the community developers.





# **Constraints and Challenges**





Fish – Salmon and Steelhead



Hydraulics – High flood flows Birds – Hawk nesting site





Archeology – Native American mortar site



Foundations – near surface bedrock



# **Process – Public Outreach**



Quincy Engineering developed a virtual 3-dimensional computer model to demonstrate what the proposed bridge would like after it was constructed.

This model was presented at public meetings with great success.





# **Process - Design**



#### **Bridge Description**

410' total length (105'-200'-105'), 3-span timber truss, with 12' clear width

Supported on reinforced concrete seat style abutments founded on steel H-piles at Abutment 1 and spread footing on bedrock at Abutment 4

Reinforced concrete piers founded on bedrock using tie-down anchors







#### Abutments







#### Piers







#### Piers











#### Superstructure

#### Presented by: Paul C. Gilham







Paul C. Gilham, P.E., S.E. Chief Engineer Western Wood Structures, Inc.

#### **Design Criteria**

- Span: 410'-0" out to out.
- Main Span: 200'-0".
- Side spans: 105'-0" each.
- Minimum vertical clearance: 10'-5"
- Width: 12'-0" inside face of trusses.
- Live Load: 85 psf. Reducible per AASHTO.
- $\Delta(LL) \leq L/500$
- Vehicle Load: 10 ton maintenance vehicle.
- Wind Load: 75 psf. of projected area
- Seismic Loading: A=0.3g, Soil Profile Type D



#### **Design Constraints and Issues**

- 4'-0" Elevation change from end to end
- •Continuous 10,000 ft. radius bottom chord curvature
  - •Adjusted for long term deflections.
    - •1.5\* $\Delta$ (DL) + 1/8" for each 10' of span.
    - •4  $\frac{1}{2}$ " (5715'R) camber at main span, 1 5/8" (5044'R) camber at side spans

•Continuous reverse curve on top chord.



#### **Design Constraints and Issues**

- Continuous 10,000 ft. radius bottom chord curvature
- 4'-0" Elevation change from end to end
- Continuous Span to limit member forces & deflections



## **Design Constraints and Issues**

- Continuous curvature to provide flowing profile.
- 4'-0" Elevation change from end to end with overall 10000 ft. radius.
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• Connection designs account for reversal of truss chord forces and to allow quick jobsite installation.







#### **Design Constraints and Issues**

- Continuous curvature to provide flowing profile.
- 4'-0" Elevation change from end to end with overall 10000 ft. radius.
- Continuous Span to limit member forces & deflections
- Connection designs account for reversal of truss chord forces and to allow quick jobsite installation.
- Continuous design resulted in large lateral reactions at interior piers.

























