

TIRE DERIVED AGGREGATE BACKFILL FOR RETAINING WALLS

Christopher L. Helstrom
Haley and Aldrich, Inc.

James L. Foster Jr., P.E.
Quincy Engineering, Inc.

**Western Bridge Engineers' Seminar
September 21, 2009**

Acknowledgements

Stacey Patenaude



California
Integrated Waste
Management Board

Acknowledgements



Why use TDA for retaining wall backfill?

- Low unit weight (0.8 Mg/m^3)
- Free draining ($k > 1 \text{ cm/s}$)
- Good thermal insulation (8 x better than soil)
- 100 tires per m^3 !



Five full-scale instrumented Walls

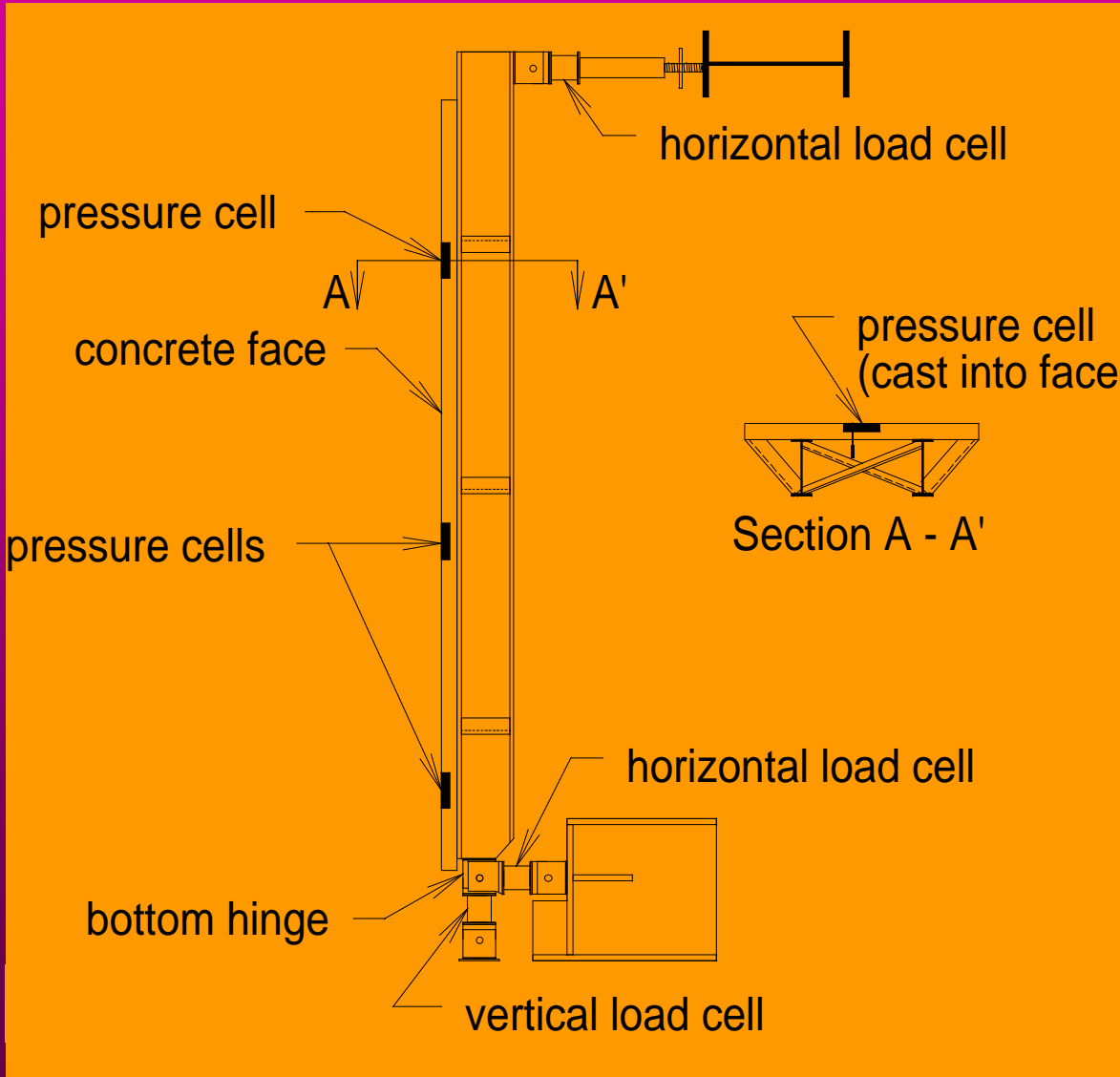
- UMaine Test Wall (three trials)
- North abutment Merrymeeting Bridge (Topsham, ME)
- Limestone Run Bridge (Tarrtown, PA)
- Wall 119 (Riverside, CA)
- Wall 207 (Riverside, CA)

Interior of UMaine Test Wall facility

Orono,
ME



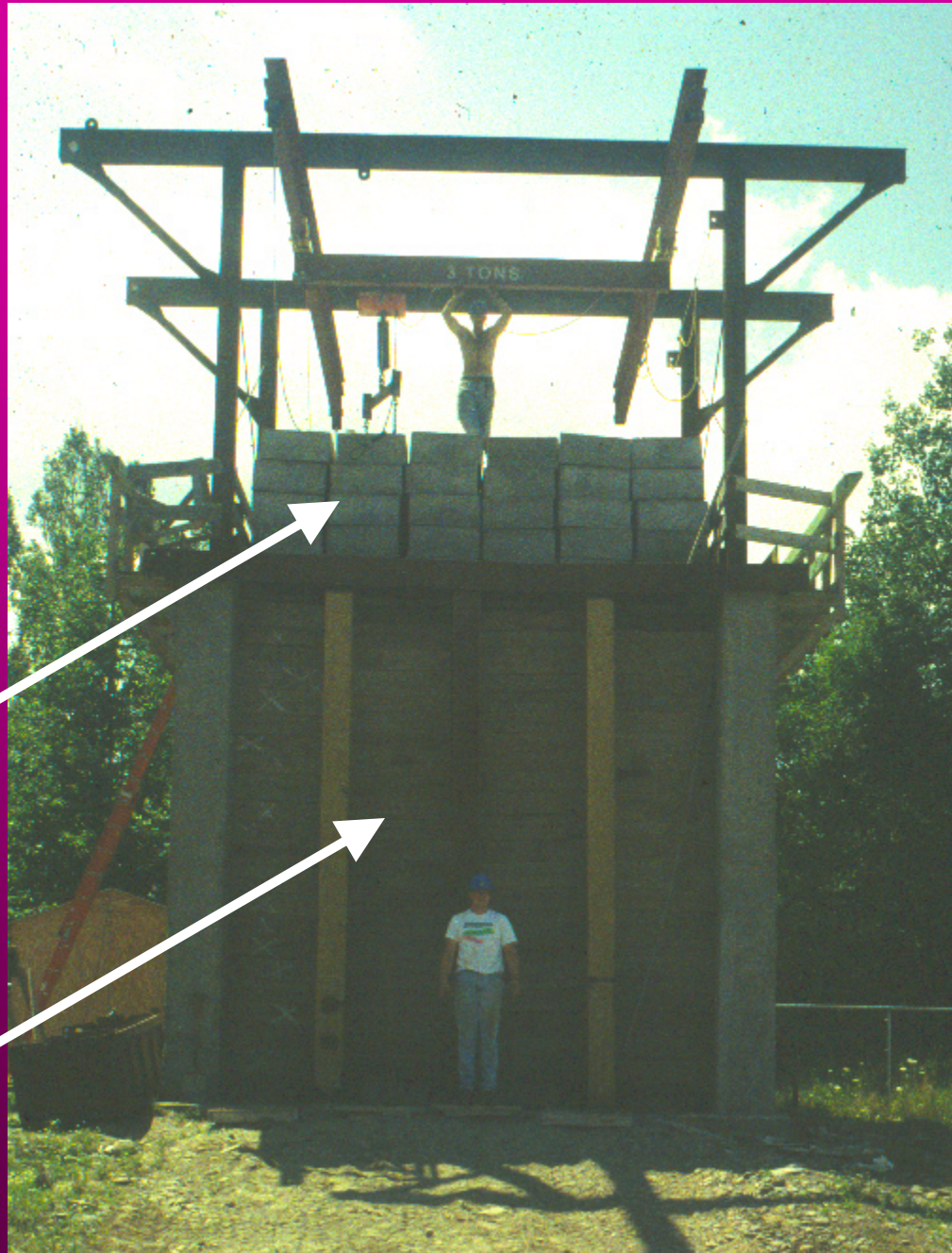
UMaine instrumented front wall



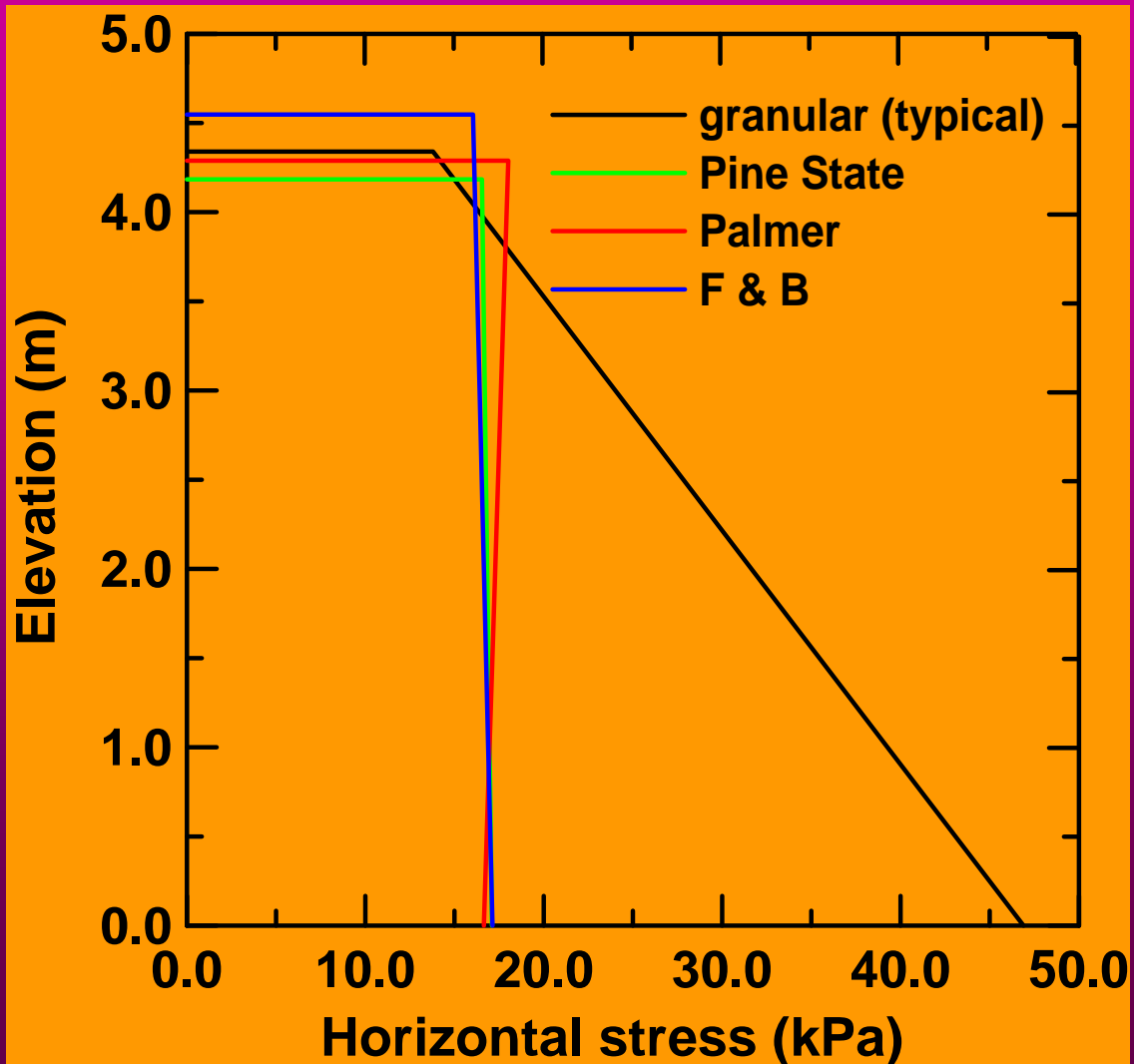
**Test
facility
fully
loaded**

**Surcharge
blocks**

**Removable
backwall**



At-rest stress distribution at 35.9 kPa surcharge

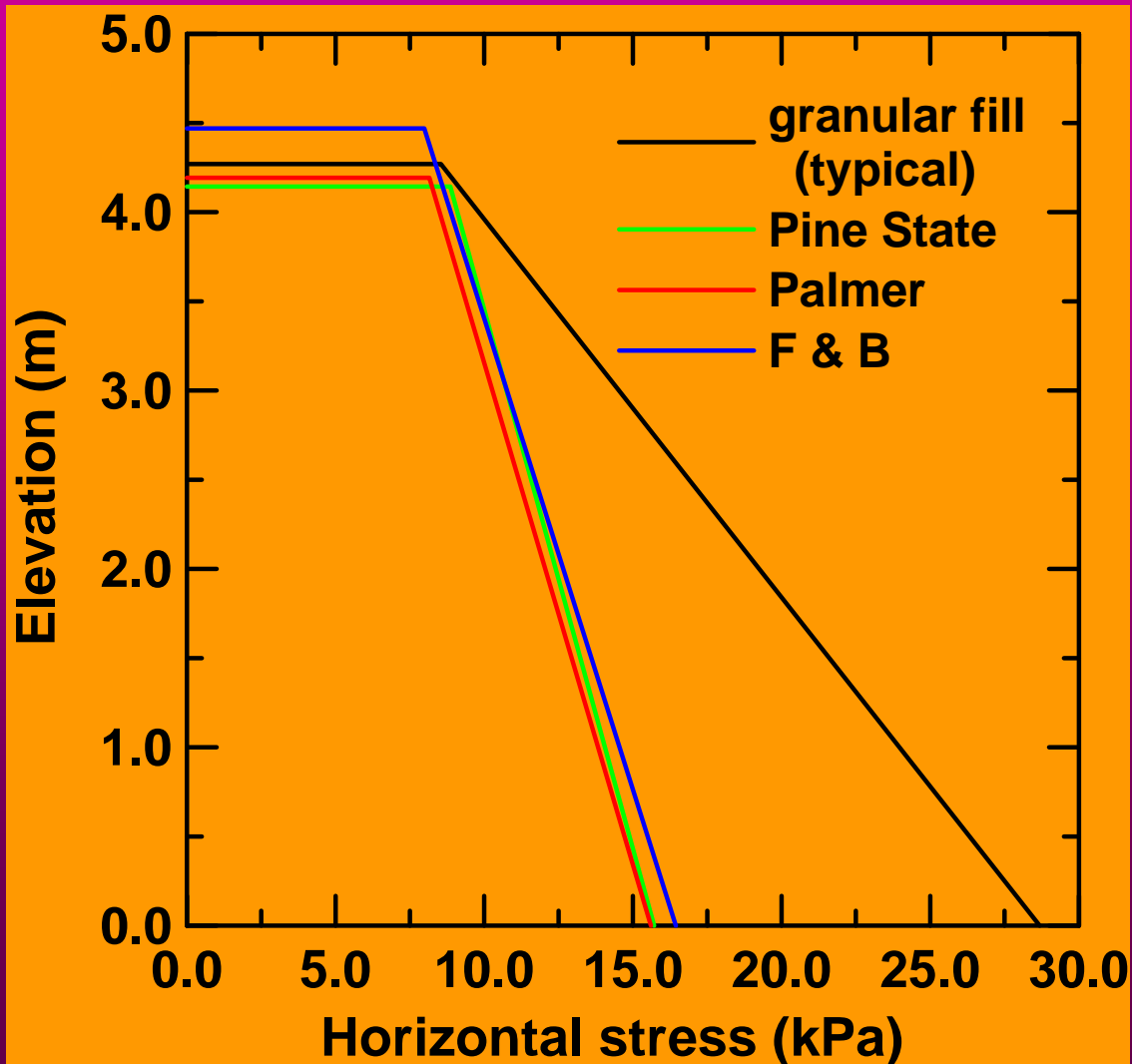


Rotating wall
away from
backfill

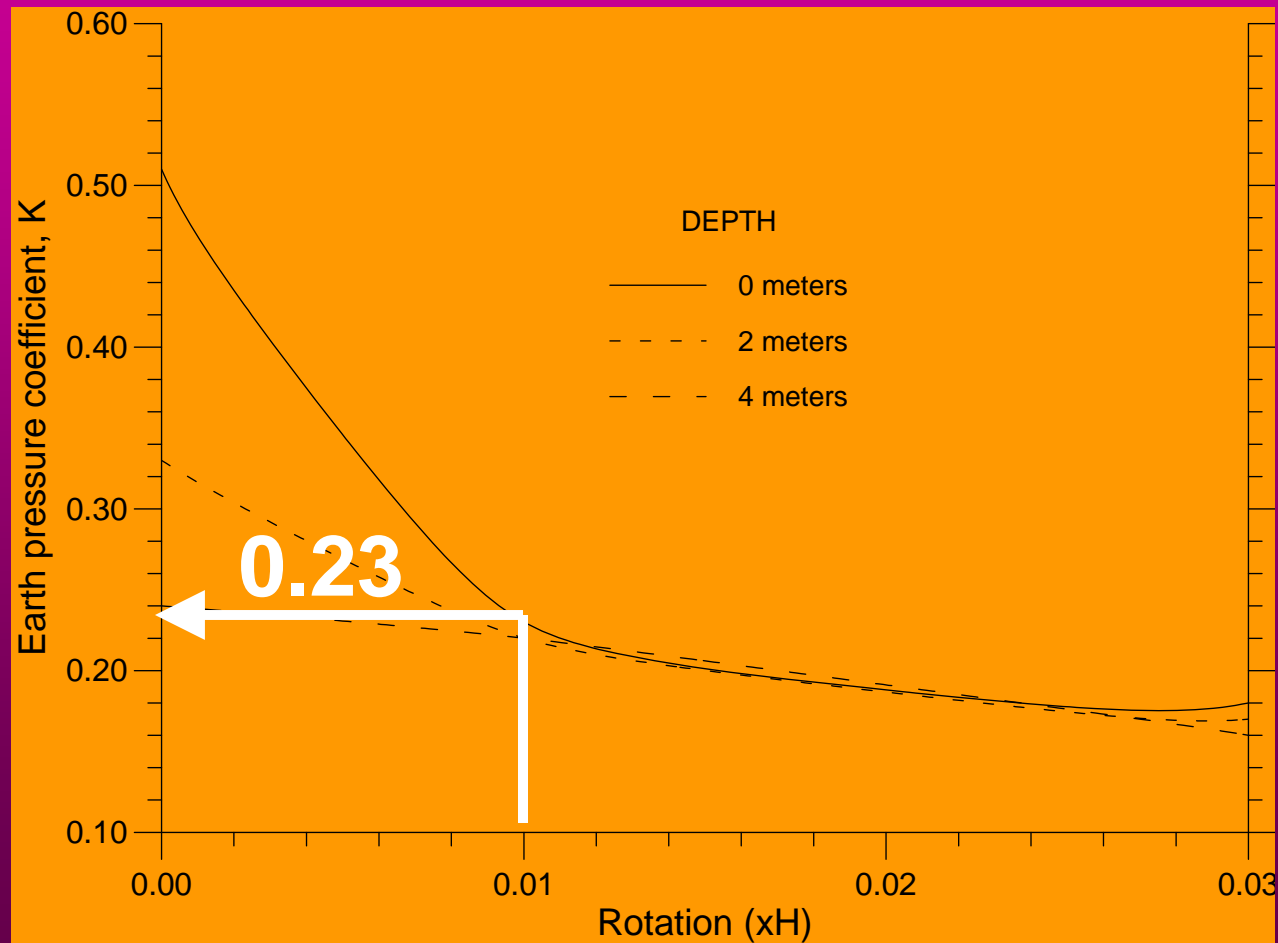
Screw
Jacks



Stress at 35.9 kPa surcharge and 0.01H rotation



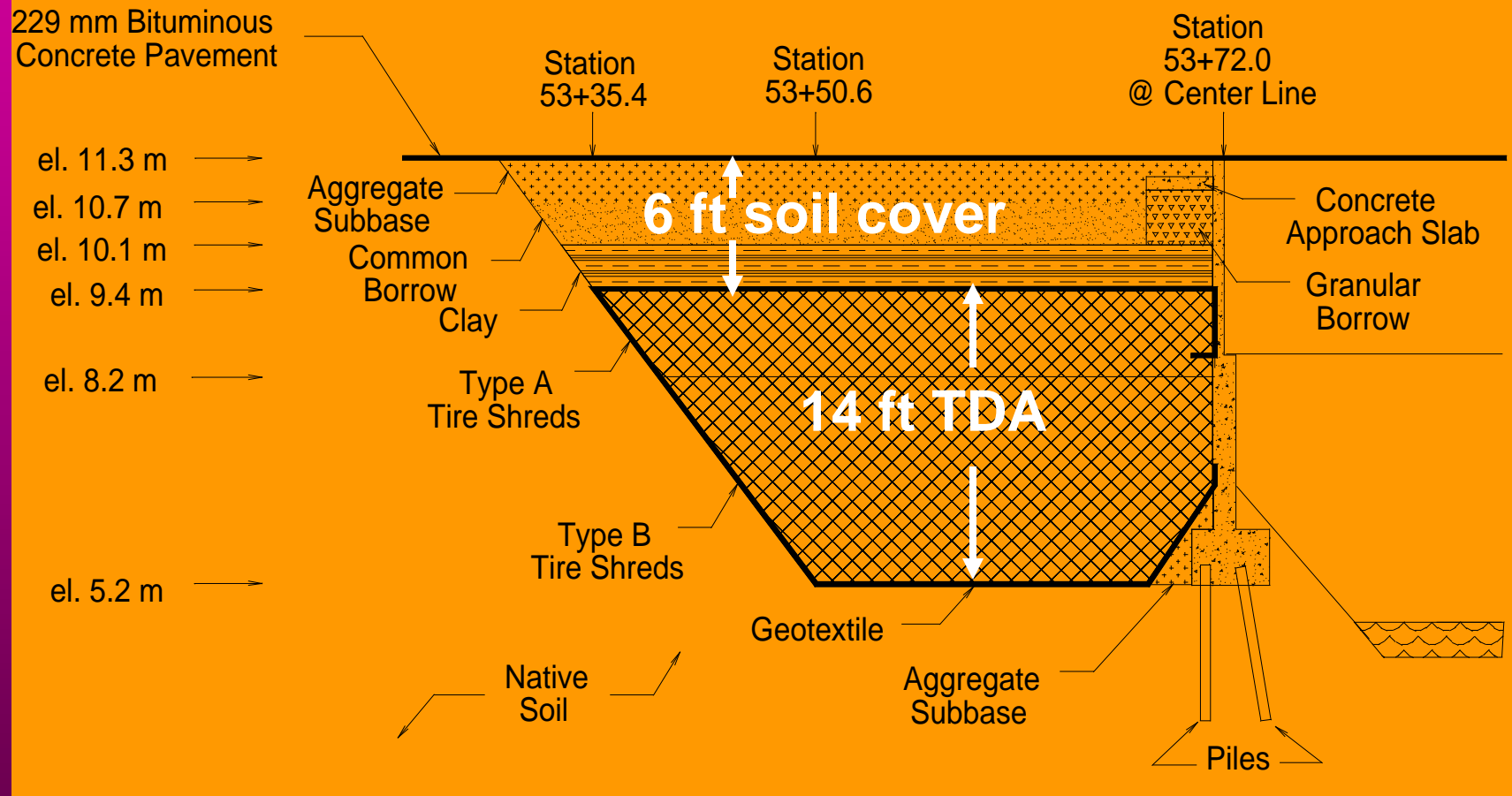
Effect of rotation on earth pressure coefficient



North Abutment of Merrymeeting Bridge Topsham, ME

- Bridge approach underlain by weak clay
- Existing factor of safety = 1.1
- Excavate upper portion of existing slope
- 14 ft of TDA fill covered by 6 ft of soil
- Used 400,000 tires

Cross section of north abutment



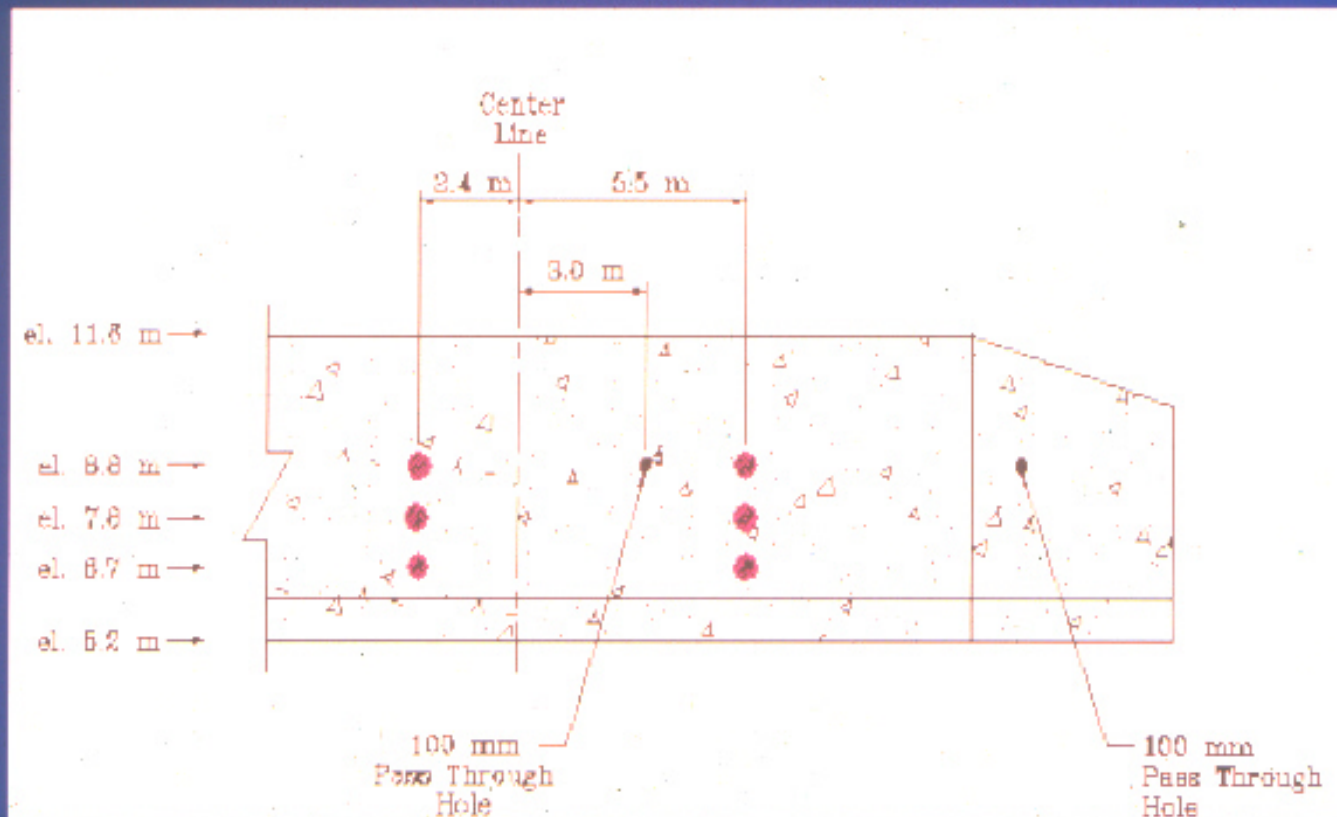
Placement with Dump Truck



Compaction with 10-ton Roller



Pressure Cell Locations North Abutment



Limestone Run Bridge

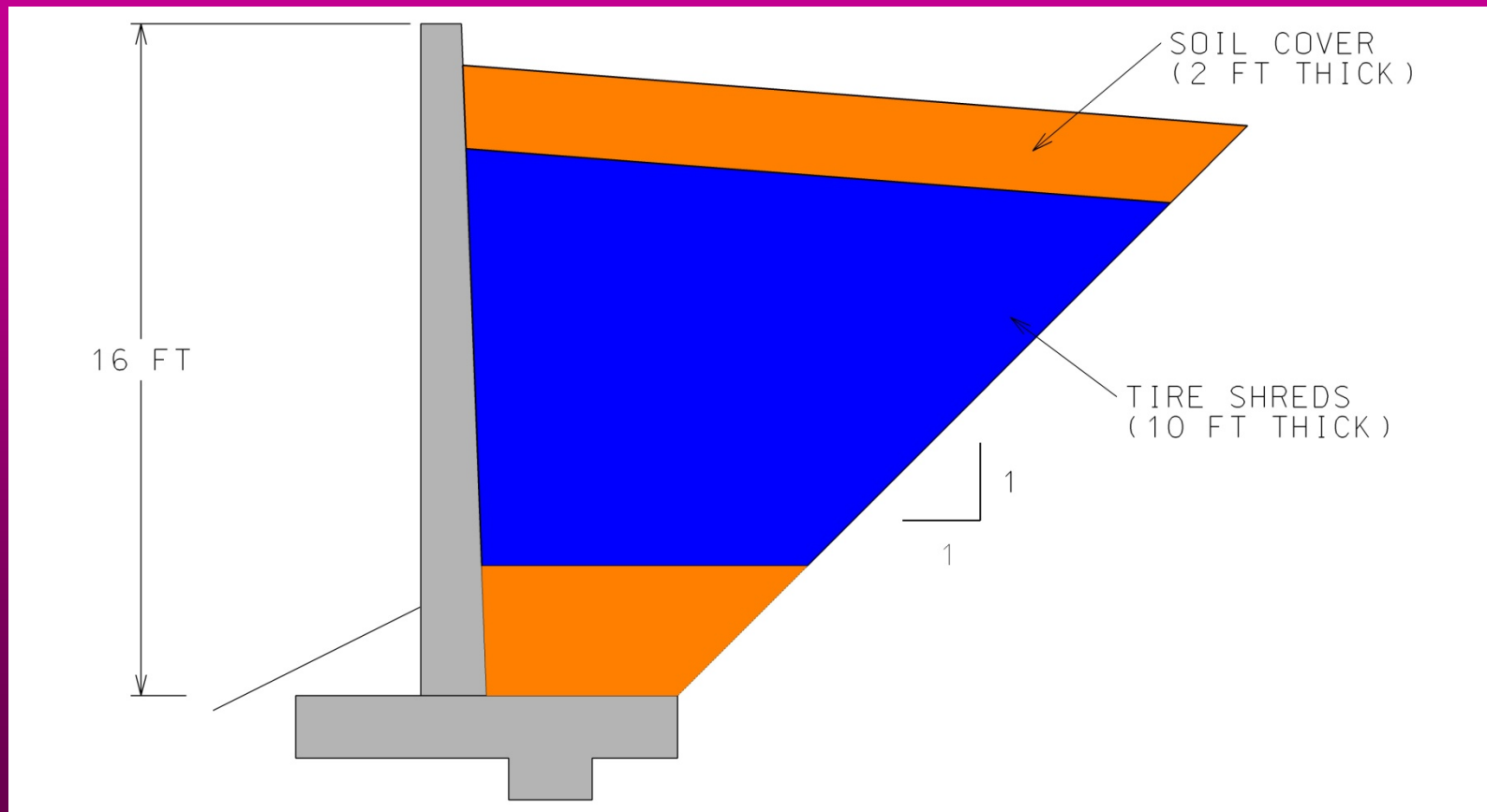
Tarrtown, PA

- Two massive pile supported bridge abutments
- Abutment 1 had a single 10-ft thick layer of TDA overlain by 4.5 ft soil
- Abutment 2 had 10-ft thick TDA layer overlain by 3-ft soil, then second TDA layer with 3-ft thickness at the abutment overlain by 4.5 ft soil

Wall 119 in Riverside, CA

- Freeway widening
- Three sections with TDA and one with soil
- Length: 79 m
- Tires used: 75,000 PTE

Wall 119 cross section



Compacting TDA



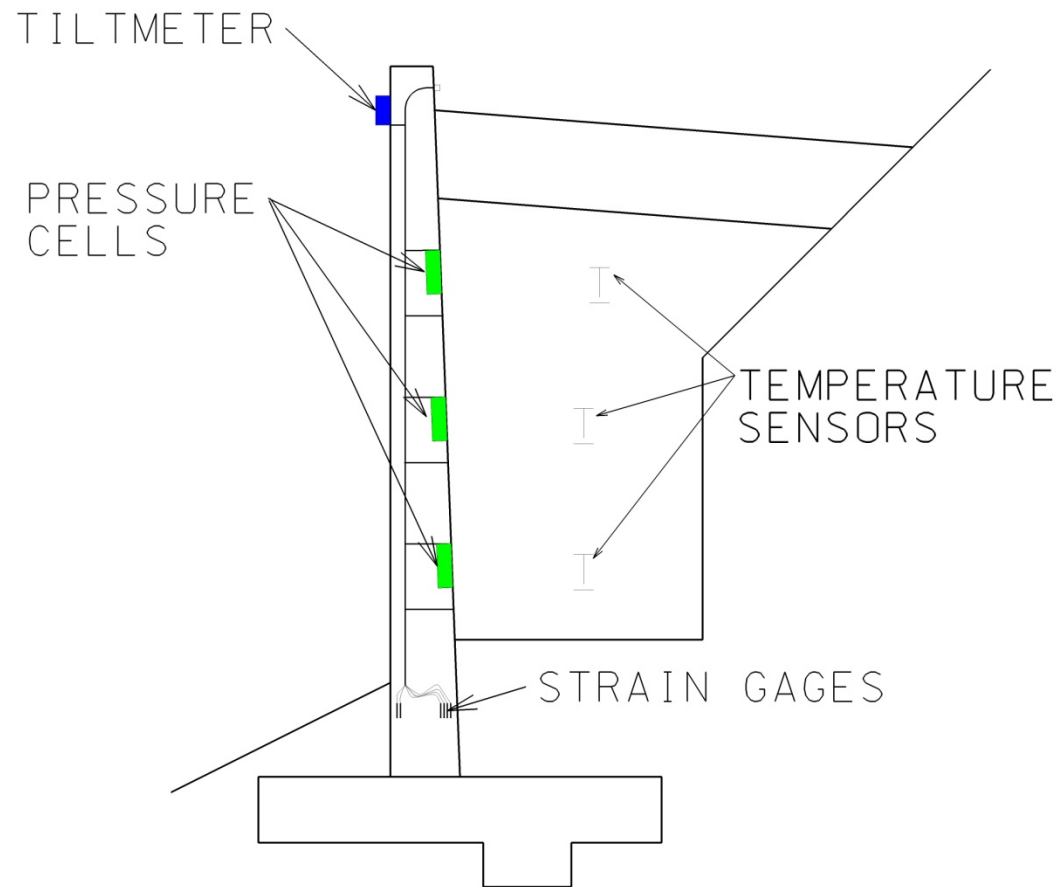
Placing soil cover



Heavy equipment
Immediately behind
wall!!!

2003 9 19

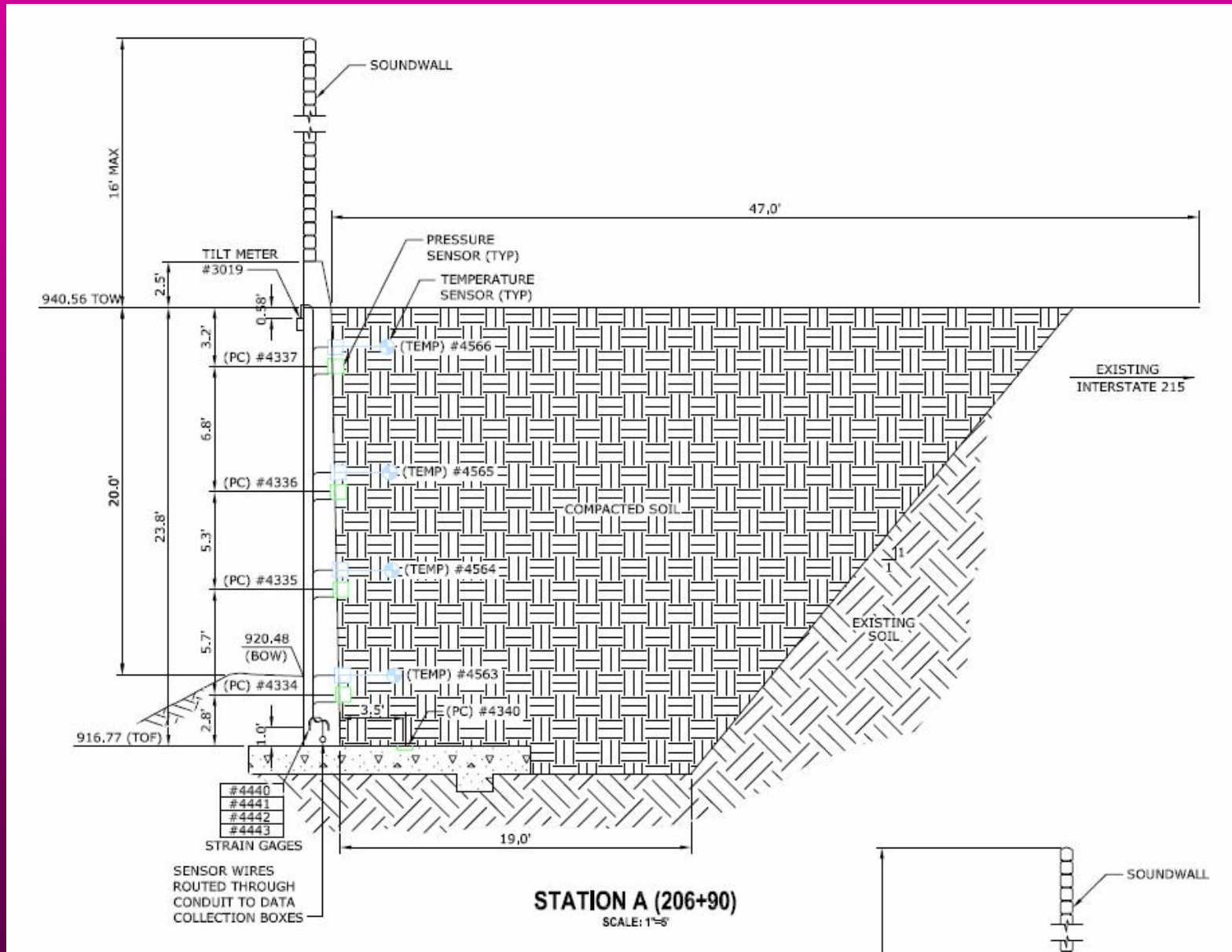
Wall 119 instrumentation



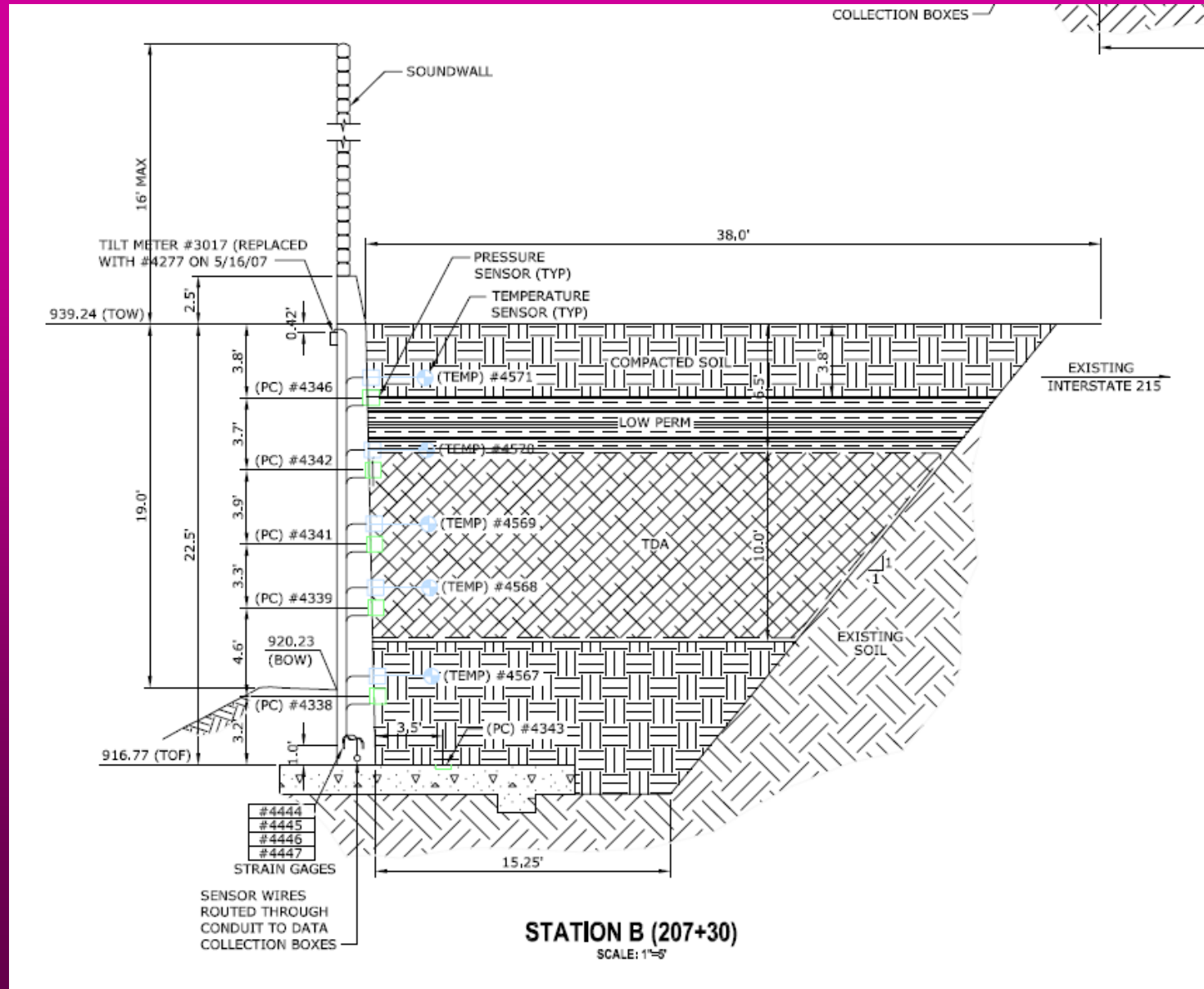
Wall 207 in Riverside, CA

- Two sections with soil backfill
- Two sections with TDA backfill

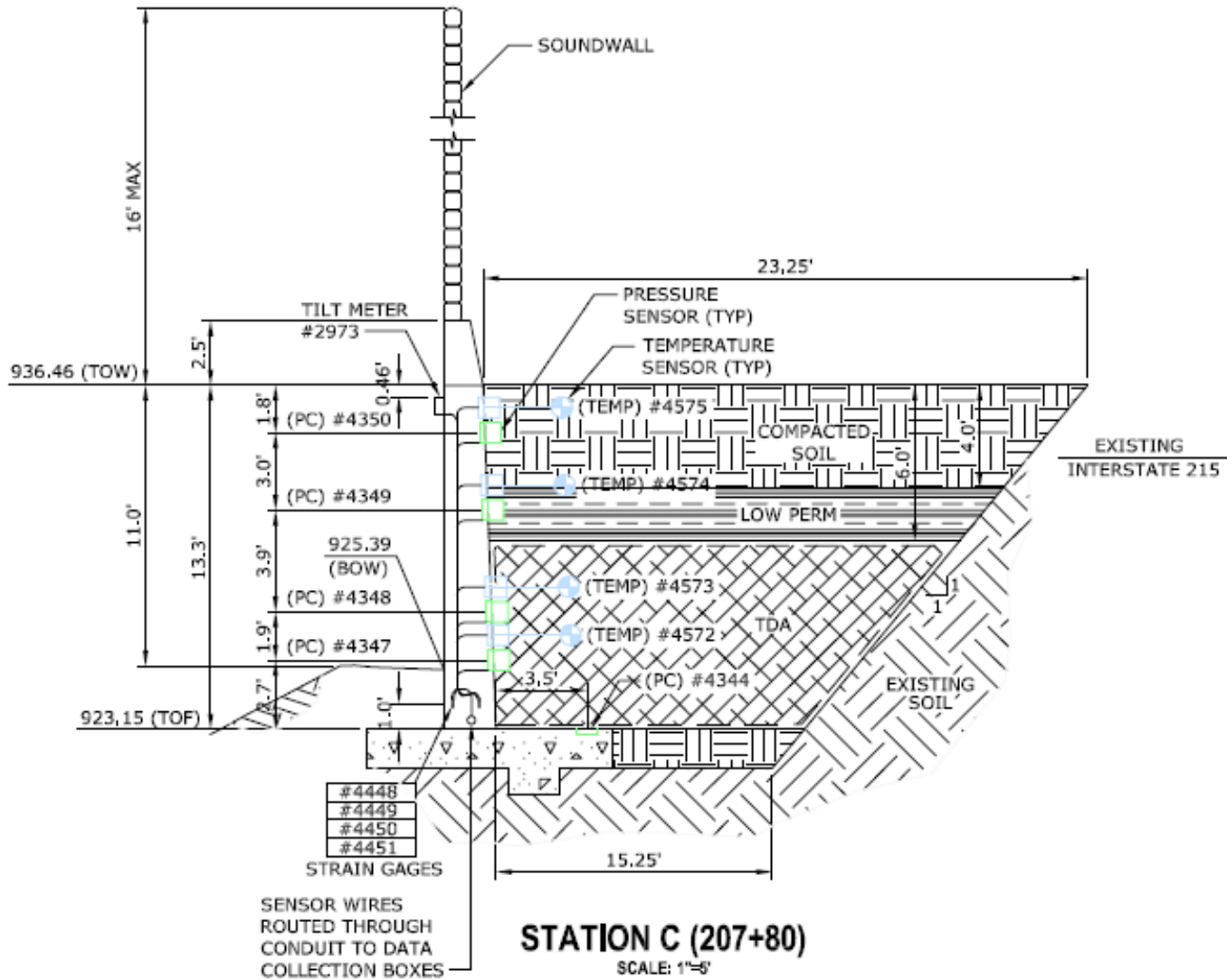
Wall 207 Station A



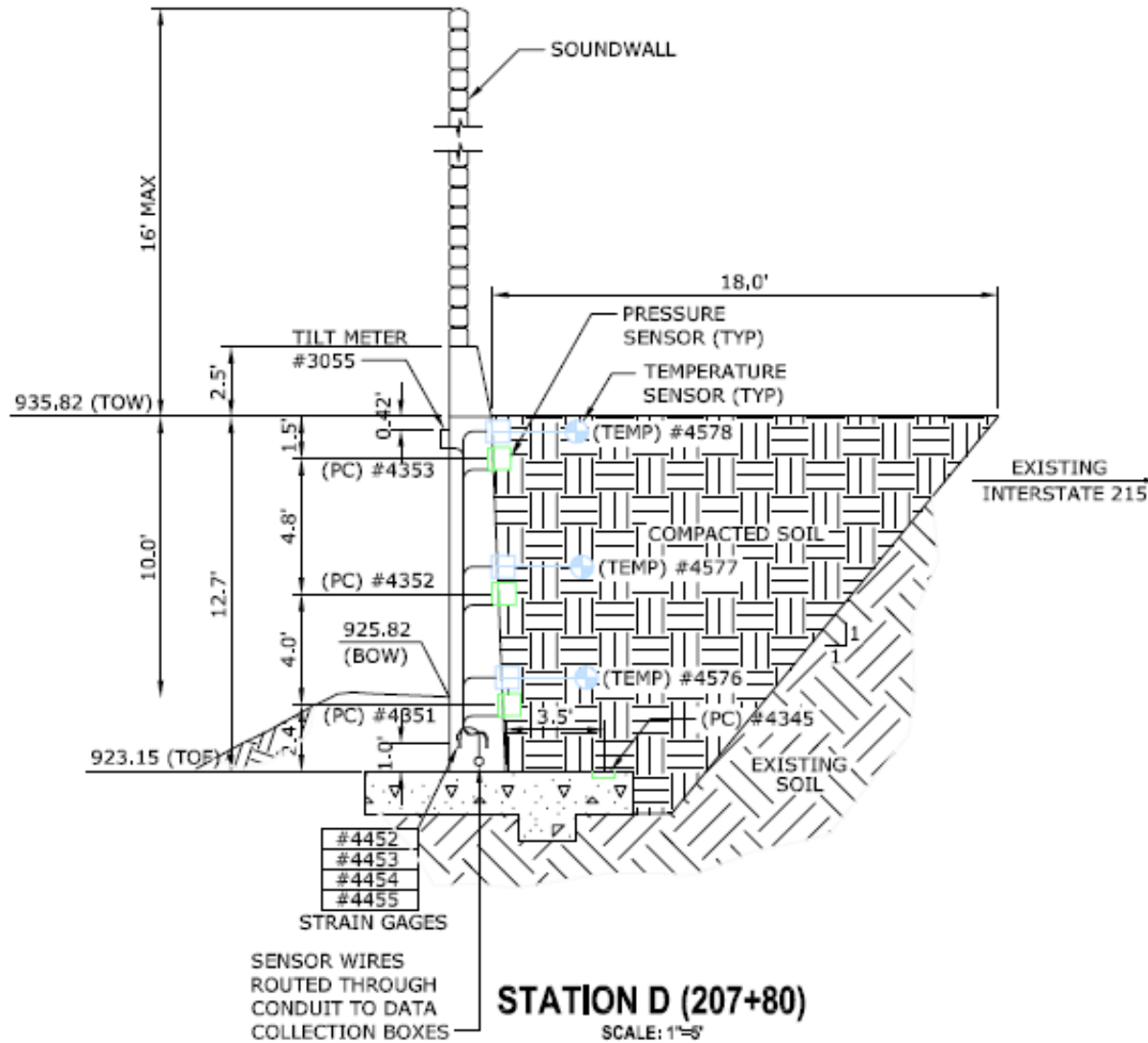
Wall 207 Station B



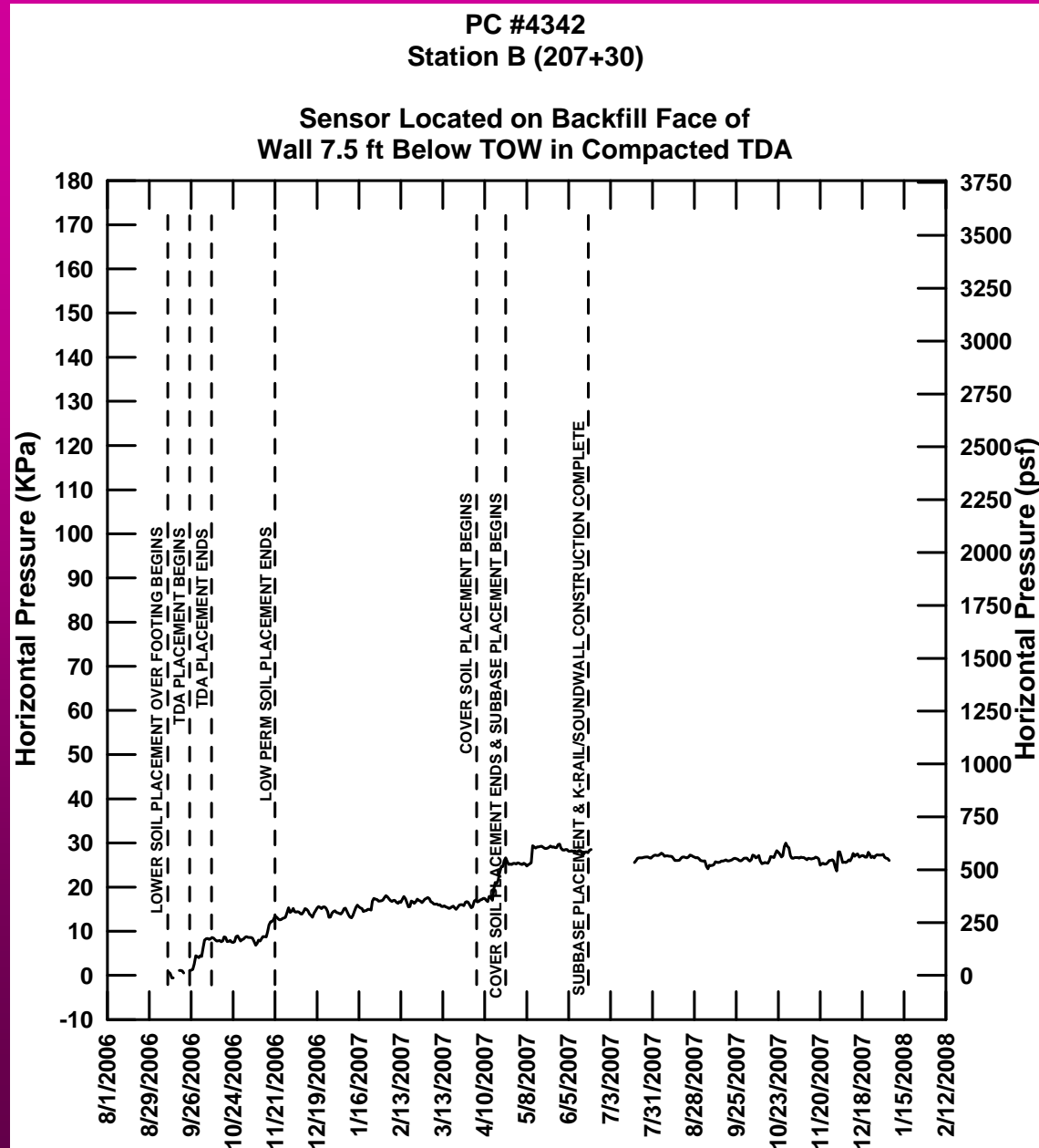
Wall 207 Station C



Wall 207 Station D



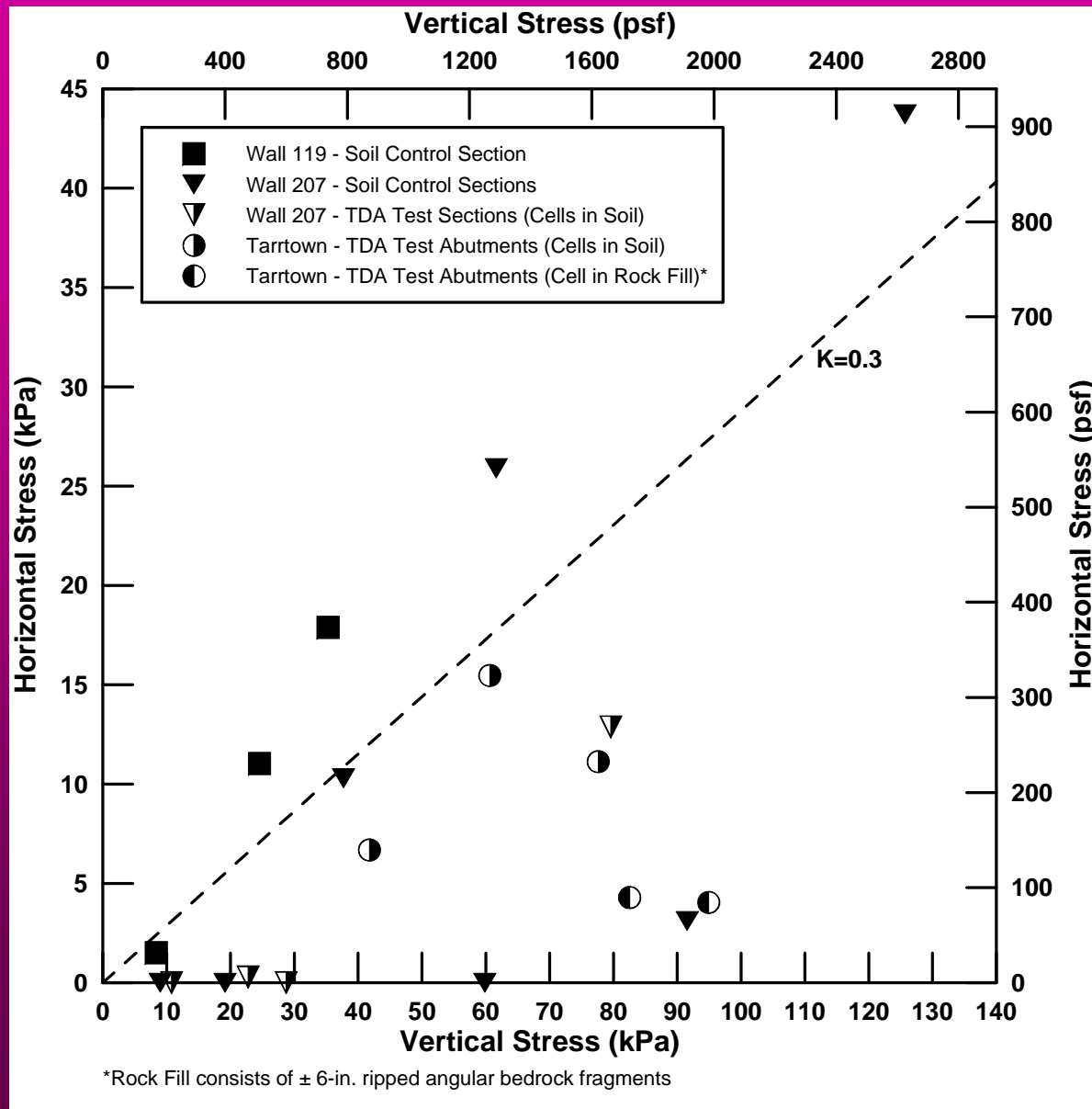
Example of Horizontal Pressure vs. Time



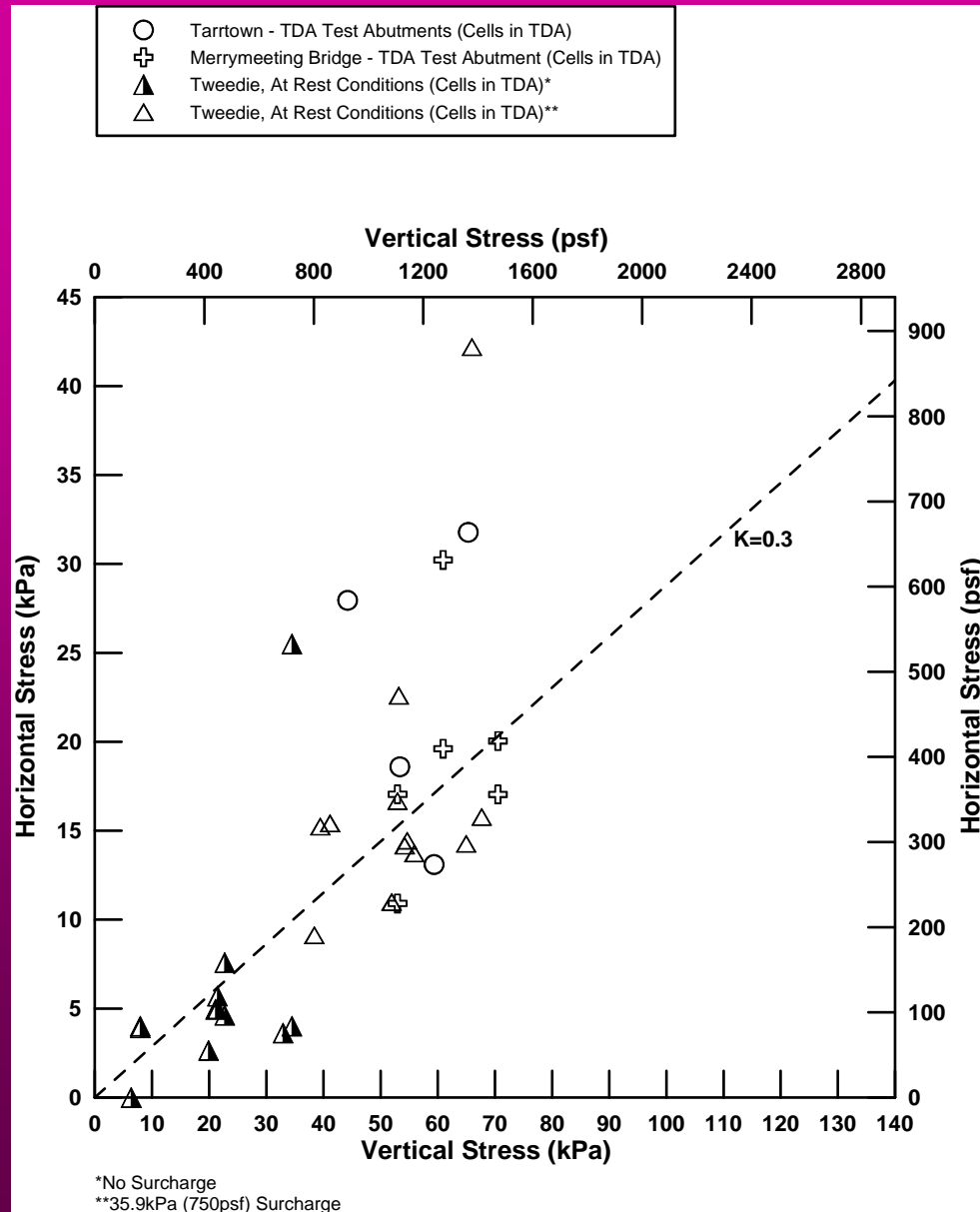
Summary of horizontal stress vs. vertical stress

- Five projects had pressure cells in TDA
- Three projects had pressure cells in soil
- Three projects had negligible wall movement (at rest conditions)
- Three projects expected to have some wall movement away from backfill (potential for active conditions)

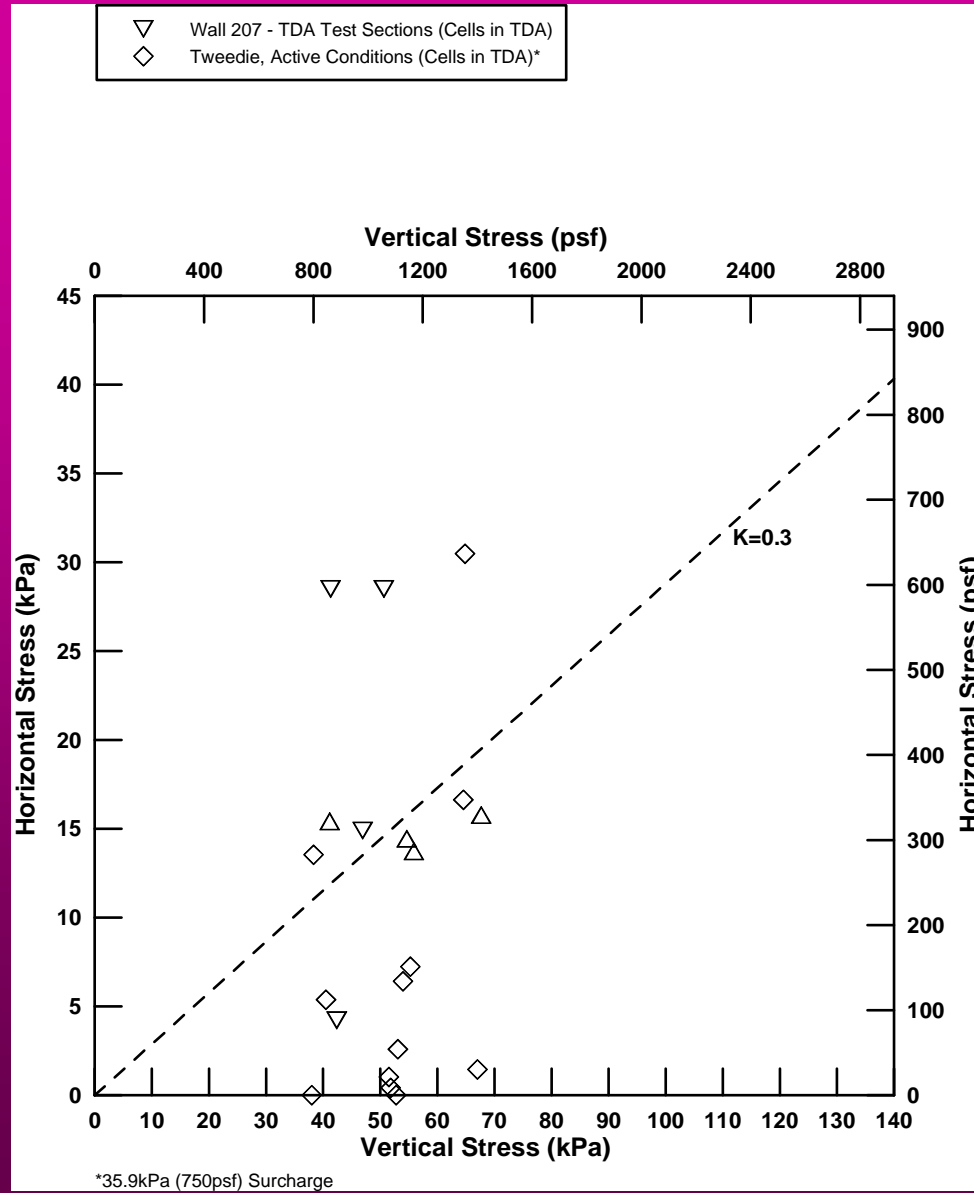
Horizontal vs. Vertical Stress (Cells in Soil)



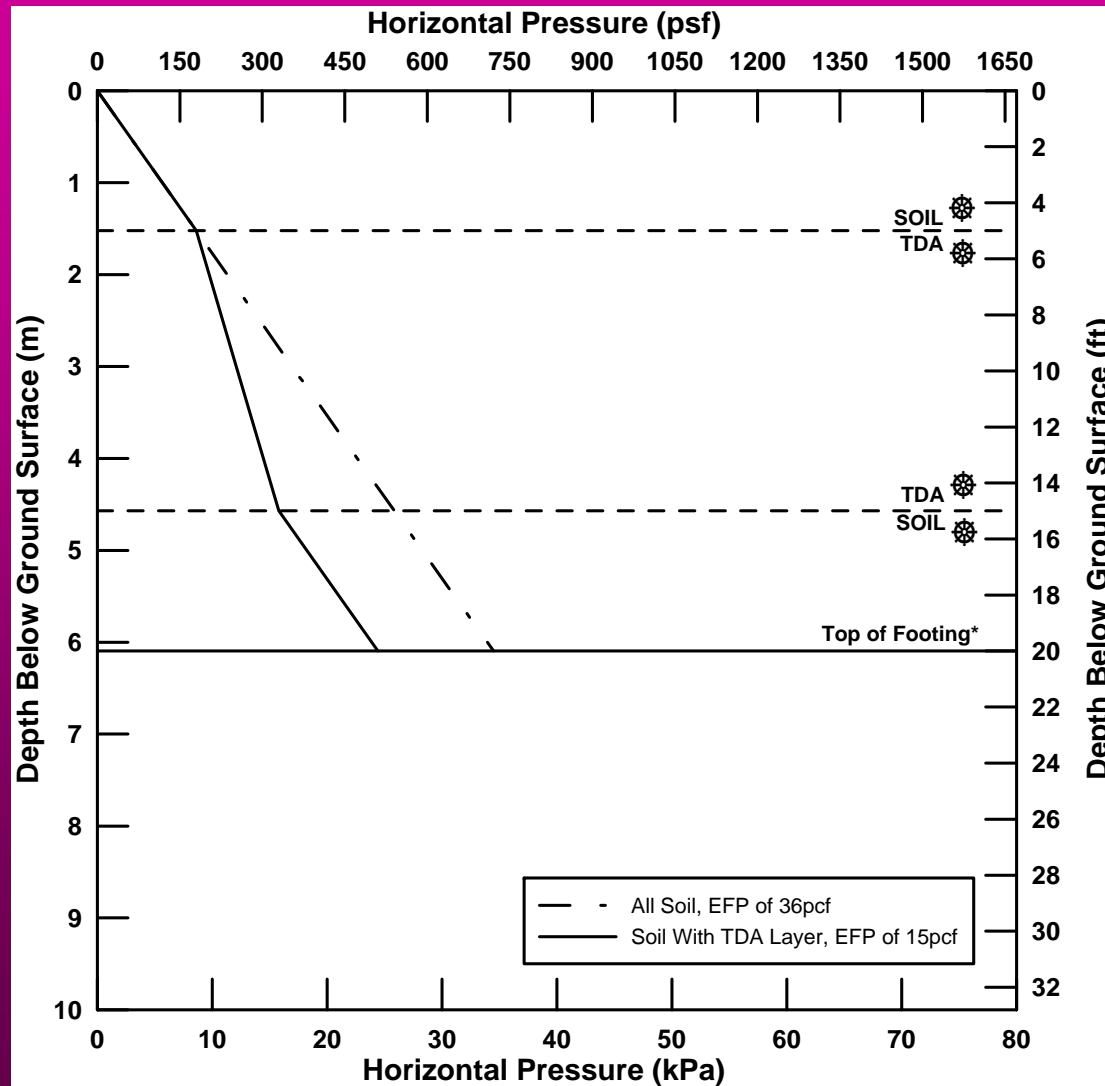
Horizontal vs. Vertical Stress (Cells in TDA, At Rest Sections)



Horizontal vs. Vertical Stress (Cells in TDA, Active Sections)



Example of Potential Benefits



Summary of Research Findings

- Current CALTRANS earth pressure coefficient ($K = 0.3$) applicable to both soil and TDA backfill
- Use equivalent fluid pressure for soil backfill equal to 36 pcf
- Use equivalent fluid pressure for TDA backfill equal to 15 pcf

Western Bridge Engineers' Seminar 2009



ANALYSIS OF TIRE DERIVED AGGREGATE (TDA) AS BACKFILL FOR CALTRANS TYPE 1 RETAINING WALLS

Presented by:
James L. Foster, P.E



In cooperation with



Dana N. Humphrey,
PH.D., P.E.

Haley and Aldrich, Inc.



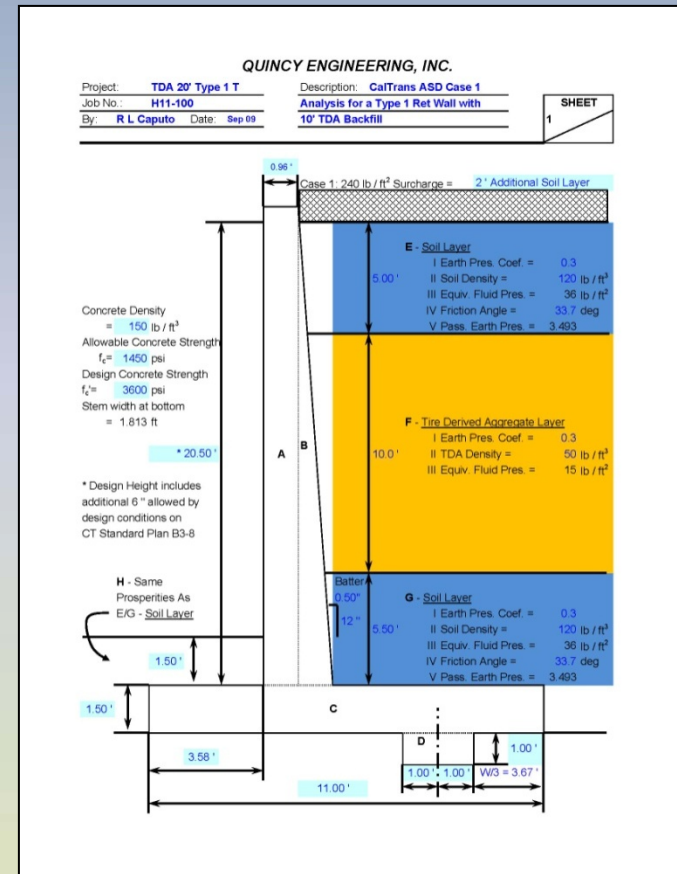
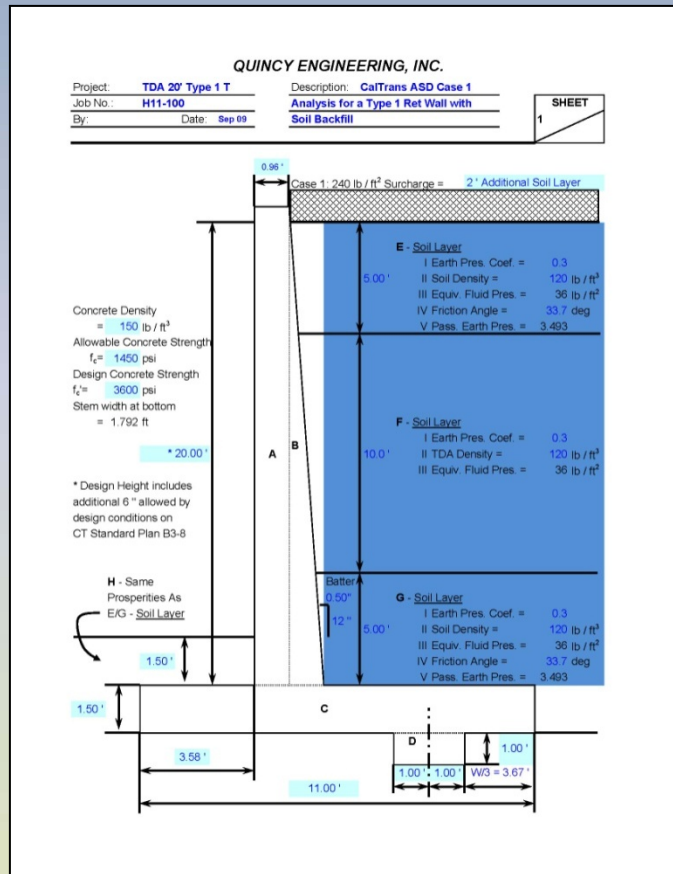
Objective



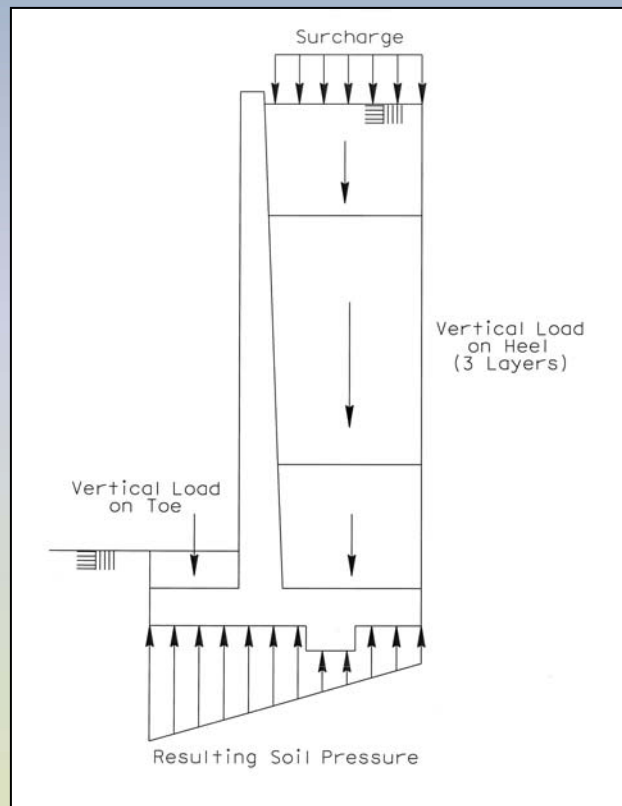
Compare Caltrans Type 1 Concrete Retaining Wall using soil backfill versus tire derived aggregate (TDA) backfill

Determine if additional effort to develop standard plans and specifications for TDA backfilled walls is warranted.

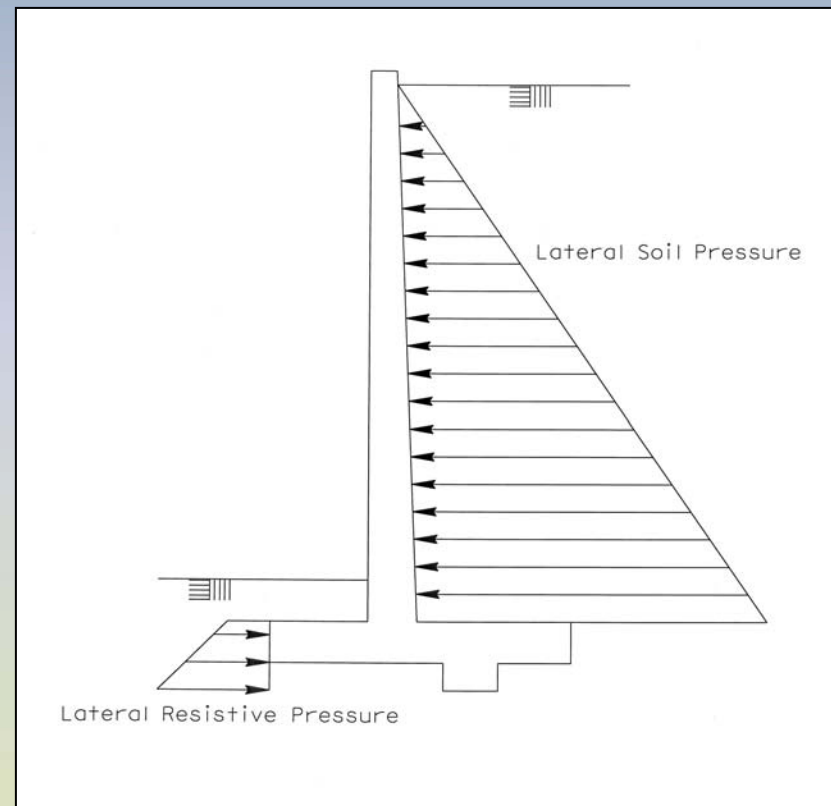
Load Case



Calculate Loads

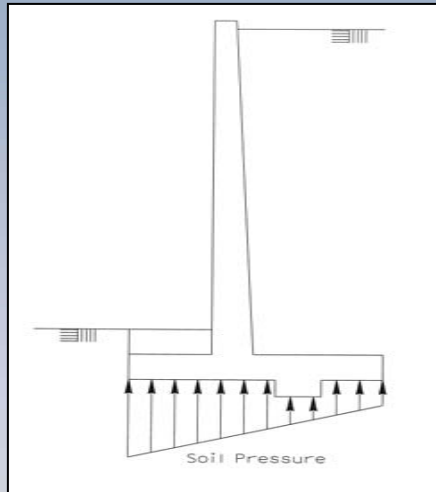


Vertical Loads

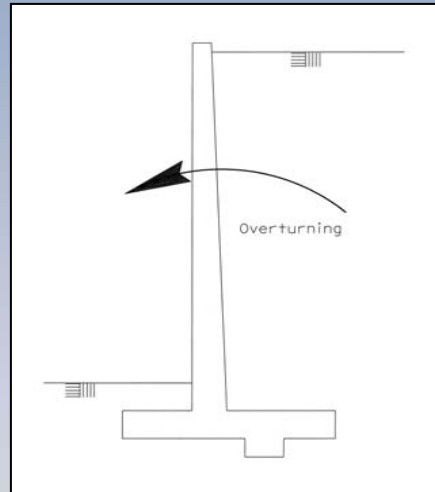


Lateral Loads

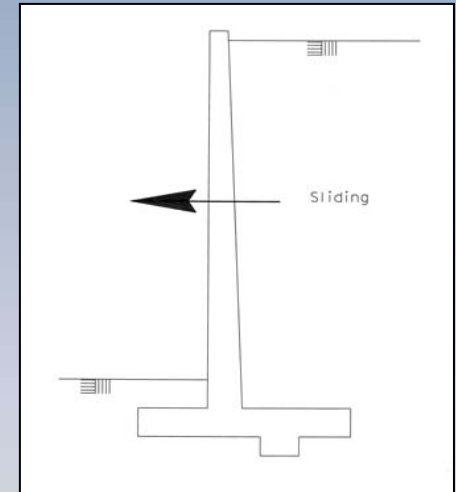
Check



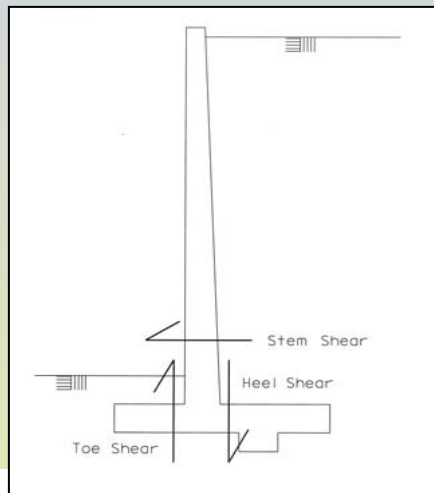
Footing Pressures



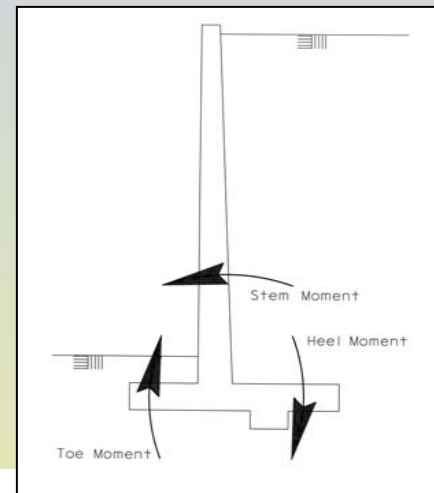
Overturning



Sliding



Shears



Moments

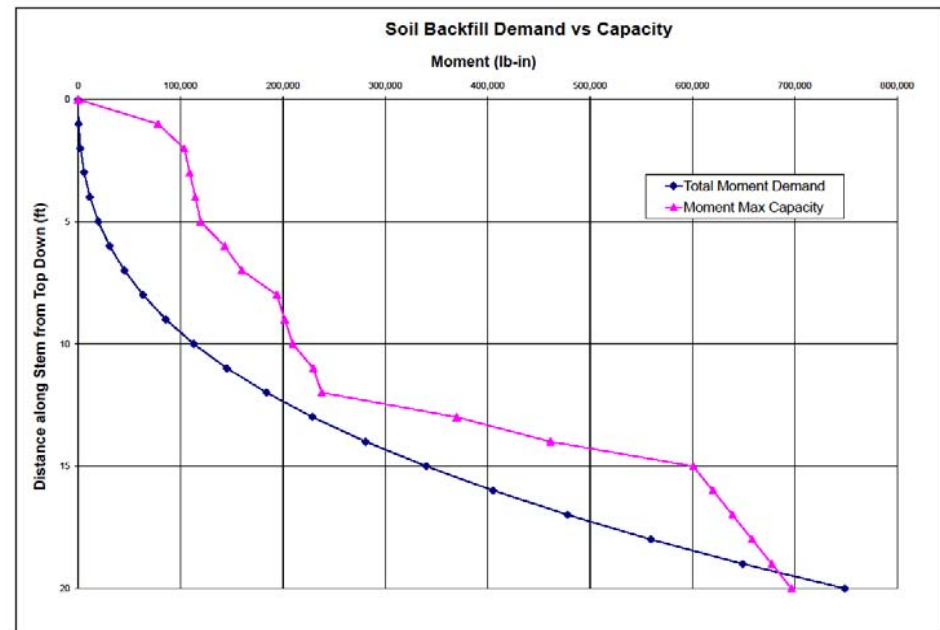
Results – Soil Backfill



Load	Demand	Allowable per Std Plan B3-1
Toe Pressure	4.3 ksf	4.3 ksf

Load	Demand	Capacity	Demand to Capacity Ratio	Minimum D/C (Safety Factor)
Overturning	76.3 k-ft	161.3 k-ft	2.1	2.0
Sliding	9.87 k	17.05 k	1.7	1.5

Load	Demand	Capacity	Demand to Capacity Ratio
<i>Shear</i>			
Stem (at base)	9.87 k	13.34 k	1.4
Toe	8.93 k	11.99 k	1.3
Heel	11.40 k	11.61 k	1.0
<i>Moment</i>			
Stem	See Chart		
Toe	24.6 k-ft	40.7 k-ft	1.7
Heel	44.6 k-ft	44.0 k-ft	1.0



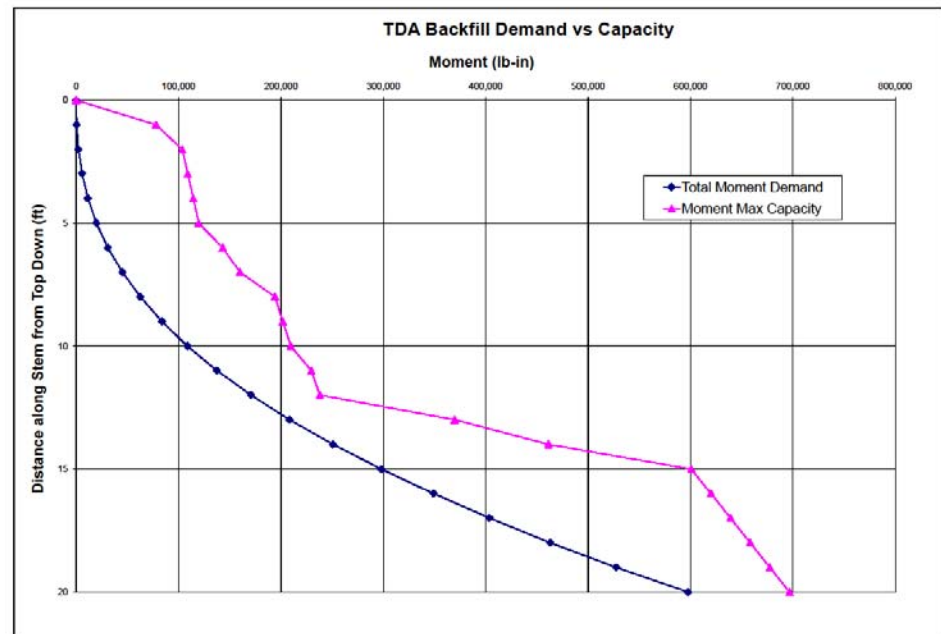
Results – TDA Backfill



Load	Demand	Allowable per Std Plan B3-1
Toe Pressure	3.7 ksf	4.3 ksf

Load	Demand	Capacity	Demand to Capacity Ratio	Minimum D/C (Safety Factor)
Overturning	61.51 k-ft	127.61 k-ft	2.1	2.0
Sliding	7.45 k	14.23 k	1.9	1.5

Load	Demand	Capacity	Demand to Capacity Ratio
<i>Shear</i>			
Stem (at base)	7.45 k	13.34 k	1.8
Toe	7.64 k	11.99 k	1.6
Heel	8.76 k	11.55 k	1.3
<i>Moment</i>			
Stem	See Chart		
Toe	21.3 k-ft	40.7 k-ft	1.9
Heel	32.7 k-ft	44.0 k-ft	1.3



Comparison – Soil vs. TDA Backfill



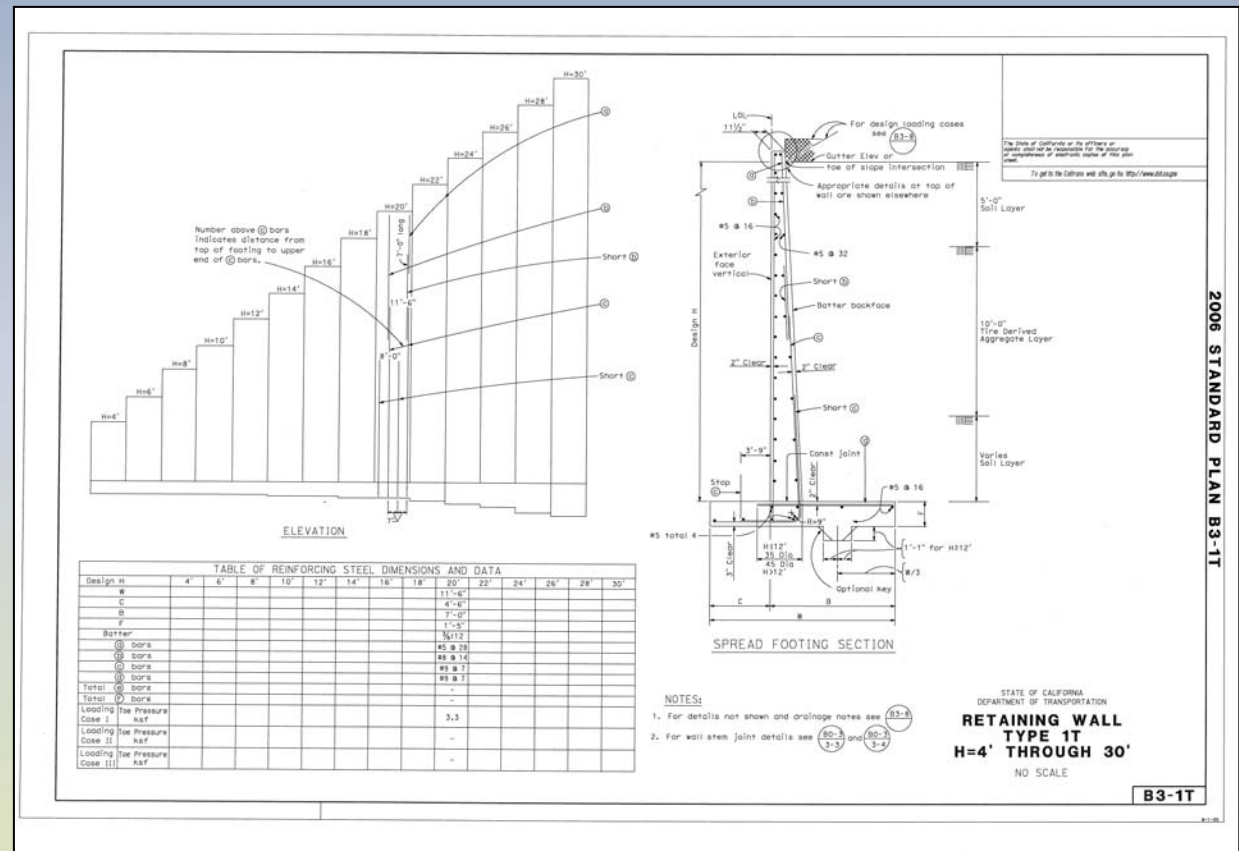
Load	Soil Demand	TDA Demand	Δ (%)
Toe Pressure	4.3	3.7	-16
Overturning	76.3 k-ft	61.5 k-ft	-19
Sliding	9.87 k	7.45 k	-25
<u>Shear</u>			
Stem (at base)	9.87 k	7.45 k	-25
Toe	8.93 k	7.64 k	-14
Heel	11.40 k	8.76 k	-23
<u>Moment</u>			
Stem (at base)	62.4 k-ft	49.8 k-ft	-20
Toe	24.6 k-ft	21.3 k-ft	-13
Heel	44.6 k-ft	32.7 k-ft	-27

Optimization



Lower Construction Costs

- Reduced Concrete
- Reduced Rebar
- Reduced Excavation
- Reduced Backfill



Optimization

Use at Marginal Soil Sites

- Lower Toe Pressure
- May Eliminate Need for Pile Footing



Conclusions



Effective use for previously considered “waste” tire material.

Reduced construction costs through use of less materials and labor.

Spread footings usable at sites where previously not feasible due to unfavorable soil conditions.

Next Steps



Develop design and standardized plans for Reinforced Concrete Type 1T retaining walls with TDA backfill using the LRFD Specifications

Identify and conduct additional studies and tests to further validate the TDA properties and behavior.

Western Bridge Engineers' Seminar 2009



TIRE DERIVED AGGREGATE (TDA) BACKFILL

QUESTIONS

In cooperation with

Presented by:

Christopher L. Helstrom
Haley and Aldrich, Inc.

James L. Foster, P.E



Stacey M. Patenaude

Dana N. Humphrey,
PH.D., P.E.

