DESIGN of the 4th AVE S OFF-RAMP Eastbound South Spokane Street Viaduct Seattle, WA

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Sacramento, CA September 21-23, 2009

OUTLINE

- Project Overview
- Selection of Preferred Alternative
- Seismic Analysis for Connectivity
- Special Box Girder Design Considerations (see handout)





Owner

Seattle Department of Transportation (SDOT)

Structural, Roadway, Traffic, Storm Drainage, Electrical, Landscaping, Irrigation Jacobs Engineering Inc.

> SEW HDR

Survey Lin & Associates

Fire Protection PACE Engineers **Geotechnical** Shannon & Wilson

- Project Status
 - Existing SSSV Built in 1941; EB Egress only at 1st Avenue South
 - Planned Widening (1994)
 - Partial Seismic Retrofit (1999)
 - Technical Feasibility Study (2006-PB)
 - TS&L Report (2007-JE)
 - Final Design Complete (2008)



- Benefits of 4th Ave Off-Ramp
 - Improve access to CBD from West Seattle
 - Facilitate transit access to E-3 Busway
 - Create grade-separation bypass over railroad
 - Alternative route into CBD during Alaskan Way Viaduct closure/construction
 - Provide access to surface roadway to alleviate I-5 ramp congestion



- Complete Redevelopment of EB Lower Roadway:
 - Roadway Re-Grading: pavement, curbs, sidewalk; SCL parking lot
 - Traffic: new signalized intersection, signal interconnect, signal loops, pedestrian movements, signs
 - Storm Drainage: upgrades and utility relocations
 - Illumination: bridge structure and parking lot



- Selection Process
 - Determine Constraints & Assumptions
 - Develop Selection Criteria
 - Method 1 Weighted Criteria
 - Method 2 Unweighted Criteria
 - Prepare Alternatives Selection Matrix
 - Hold Alternative Selection Workshop



Constraints & Assumptions

- No Piers in EB Lower Roadway
- No Column in 4th Avenue South
- Ramp to be Seismically Isolated from Viaduct
- Maintain 20-foot Overheight Vehicle Corridor
- No Lane Closures on Viaduct for Installation of Substructure
- 15-foot Minimum Clearing in SCL Parking Lot
- Staging in SCL Parking Lot



Selection Criteria

- Non-Varying Elements between Alternatives
 - Site Prep & Removals
 - Illumination
 - Storm Drainage
 - Signalization
 - Paving
 - Impacts to Seattle Fire Dept or Metro Bus during Construction
 - Signing
 - Fire Protection

- Utility Impacts Qwest / MCI
- Generalized Staging of Work



Selection Criteria

- Applicable Criteria (for Selection Matrix)
 - Construction Cost (Year of Cost)
 - Life Cycle Costs (Total)
 - Performance Risk
 - Environmental Impacts
 - Ease of Construction (falsework, staging)
 - Aesthetics
 - Engineering Cost
 - Traffic Impacts
 - Utility Impacts during Construction (SPU / OPL)
 - Permanent SCL Parking Lot Impacts

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Selection Criteria

- Non-Applicable Criteria
 - Community Impacts
 - Geological/Geotechnical Variation
 - Environmental Impacts (Wildlife, Wetlands, Air Quality, Noise)
 - Cultural-Archeological-Historic Preservation Concerns
 - Hazardous Materials Cleanup
 - Parking Lot Disruption During Construction
 - Soil Remediation
 - Weather Sensitivity of Structure
 - Maintenance

- In-Service Bridge Inspection Frequency and Ease
- Minimizing Number of Bridge Spans



• Method 1 – Weighted Criteria

CRITERIA WEIGHTING						
		Maintenance	Geometrics	Aesthetics	Geotech	
CRITERIA		В	С	D	Е	WT
Capital Cost	Α	ЗA	2C	4A	2E	7
Maintenance	В	-	3C	3B	3E	3
Geometrics	С	-	-	3C	2C	10
Aesthetics	D	-	-	-	3E	0
Geotechnical	Е	-	-	-	-	8
	4	Major Preference				
	3	Medium Preference				
	2	Minor Preference				
	1	No Preference				

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• Method 1 – Selection Matrix

WEIGHTED MATRIX					
CRITERIA	WT	Alt 1	Alt 2	Alt 3	Alt 4
Capital Cost	7	3	4	3	2
Maintenance	3	2	5	2	3
Geometrics	10	3	3	2	4
Geotechnical	8	5	5	5	3
TOTAL WT		97	113	87	87
RANKING		2	1	3	3

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• Method 2 – Selection Matrix

UNWEIGHTED MATRIX					
CRITERIA	WT	Alt 1	Alt 2	Alt 3	Alt 4
Capital Cost	1	3	4	3	2
Maintenance	1	2	5	2	3
Geometrics	1	3	3	2	4
Geotechnical	1	5	5	5	5
TOTAL WT		13	17	12	14
RANKING		3	1	4	2

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- Alternatives
 - Transition Structure
 - T1 2-span T-Beams; 5-span W50G Girders; (81-foot spans); drilled shaft foundations
 - T2 2-span T-Beams (81-feet); 3-span W74G (120-foot spans); drilled shaft foundations
 - T3 Same as T1 with partial micropile foundations
 - T4 Same as T2 with partial micropile foundations
 - T5 9 Spans T-Beams (40-foot spans); drilled shafts

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Alternatives

- Ramp Structure
 - R1 3-span steel girder or box
 - R2 1-span steel girder or box; either 4-spans W50G or 2-spans concrete box
 - R3 2-span steel girder or box with extended SEW
 - R4 6-span W50G with pier in roadway
 - R5 11-span W50G with spliced WF74PTG over roadway
 - B1 2-span T-Beams; 4-span P/T Box; 3-span P/T Box; 2-span P/T Box

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• B2 5-span P/T Box



- Seismic Separation
 - Steel-Filled Grate
 - Successful Previous Use
 - No Maintenance Issues
 - Lateral Load Transfer





Seismic Response Spectrum



- Seismic Models
 - Model 1 Existing SSSV
 - Three Frames
 - Properties from As-Built Plans
 - Includes Retrofits Required for Widening
 - Model 2 Existing SSSV with Widening
 - Model 3 Existing SSSV with Ramp
 - Model 4 Widened SSSV with Ramp
 - Model 5 Ramp Alone



Model 4 – Widened SSSV with Ramp



- Analysis Methodology
 - Linear-Elastic Multi-Modal RSA
 - Seismic Design Forces Not Calculated
 - Similar Analysis Procedures
 - Significantly Different Design Codes
 - Column Displacement Demands Used for Comparative Analysis of Relative Displacements
 - Compression Model w/ Stiffened Joints Lock Frames
 - Tension Model with Reduced Joint Stiffness Allows Independent Transverse and Longitudinal Movement Across Joints

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• Analysis Parameters

- Columns: $I_{cr} = 0.5 I_{g}$
- Cap Beams / Girders: I_{uncracked}
- Foundations
 - Ramp/Widening Drilled Shafts: rotational and translational springs @ base of columns
 - Existing Timber Pile Foundations: assumed pinned (Springs would lengthen period and reduce column displacements)
 - Non-liquefied soil properties maximized foundation stiffness



Tension Model Results



Compression Model Results



Comparative Displacements Summary

Model 1- Existing SSSV	Demands much higher than with Widening (Model 2) installed retrofits do not protect columns
Model 2- Widened SSSV	Displacement demands lower than existing structure (Model 1)
Model 3- Existing w/ Ramp (Pre-Widening)	Significant reduction in demand on most existing columns; increased demand on 4 bents for transverse EQ temporarily until widened
Model 4- Widened SSSV w/ Ramp	Displacement demands not appreciably different than w/ Model 2
Model 5- Ramp Alone	Displacement higher than when connected (Model 4); leads to conservative design forces

Conclusion

- Ramp Structure does not have long-term negative impacts to SSSV structure
- Design Completed 2008 (with closure pour)
- Low Bid \$17.23M
- Completion Planned for Fall 2010



Schematics – Lower Roadway





Schematics – North View





