

Each Superpave project was compared to the previous overlay or construction that was done at the same location (State Route and milepost) to limit variables such as environment and traffic levels. The PSC, IRI, and rut depths were retrieved from the WSPMS for both projects at the same age.

The Superpave projects range in age from one to six years old (placed from 1996 to 2001), of which approximately 70 percent are age 3 or less. The Hveem-designed projects were constructed anywhere from 1967 to 1998 (approximately 68 percent of these projects were placed prior to 1990). For the comparison, the Superpave and Hveem-designed projects were broken into similar sections, which consist of the same year or class of mix and a similar lift thickness. All Superpave projects utilized a PG binder, while almost all the Hveem-designed mixes (94 percent) used AR4000W. Since lift thickness can affect the PSC, IRI, and rut depths, comparisons were only made when the lift thicknesses for both projects were within 0.10 feet of each other.

Pavement Condition Performance

The Superpave PSC ranged from 47 to 100 while the Hveem-designed mixes ranged from 22 to 100 (Figure 2). The majority of the Superpave sections have a higher PSC than its comparable Hveem-designed section. After a maximum of six years performance:

- 48 percent of Superpave sections have a higher PSC
- 29 percent of the comparable sections had the same PSC
- 23 percent of the Superpave sections have a lower PSC

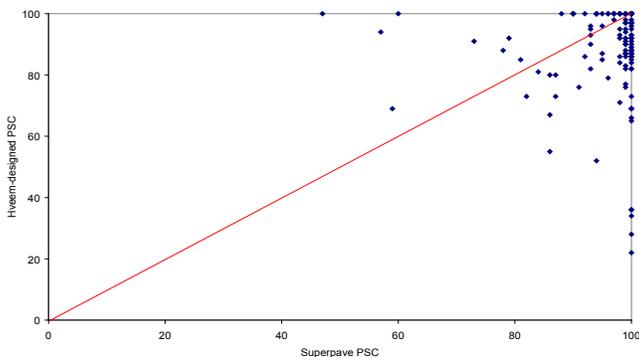


Figure 2. Comparison of PSC.

Roughness Performance

The Superpave IRI ranges from 25 inches/mile to 204 inches/mile while the Hveem-designed IRI ranges from 48 inches/mile to 319 inches/mile. Almost all of the Superpave sections have a lower IRI (smoother)

than the Hveem-designed mixes (Figure 3). After a maximum of six years performance:

- 91 percent of the Superpave section’s IRI values are lower than the Hveem-designed sections

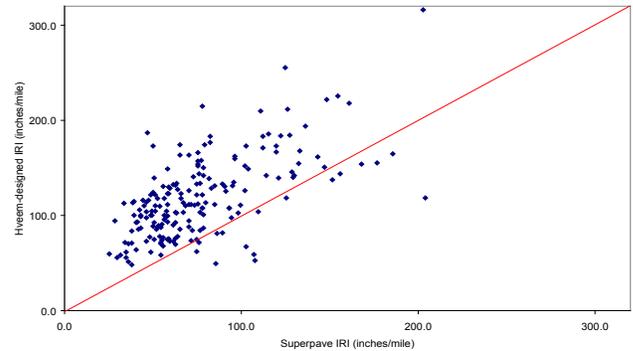


Figure 3. Comparison of IRI.

Rutting Performance

The range of rut depths for the Superpave sections was 0 inches to 1/4 inch while the Hveem-designed mixes ranged from 1/32 inch to 7/16 inch (Figure 4). The rut depths for the Hveem-designed sections were usually higher than the rut depths of the Superpave sections. After a maximum of six years performance:

- 60 percent of the Superpave sections had lower rut depths than the Hveem-designed sections
- Of the remaining 40 percent, 12 percent had the same rut depths and 28 percent of the Superpave sections had higher rut depths than the Hveem-designed sections

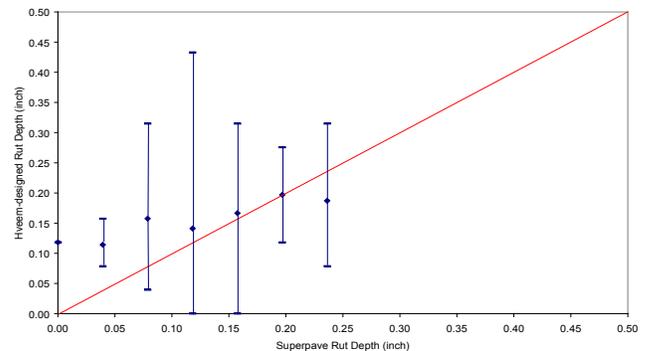


Figure 4. Comparison of rut depths.

Performance Results

Although these results favor Superpave over the conventional Hveem-designed mixes, recall that 68 percent of the Hveem-designed sections were placed prior to 1990. In recent years, there has been an increased focus on improving construction practices, improving initial smoothness, and improved design methods that cannot be taken into account with this type of comparative study. Therefore, an additional check was performed on Hveem-designed projects

constructed at approximately the same time as the Superpave projects. To ensure that the traffic levels and environment were similar, only projects constructed near or adjacent to the Superpave projects were chosen. Another constraint was to limit the construction year of the Hveem-designed projects from 1990 to 2002. Due to these constraints, there were only 8 comparison projects (all constructed in 1997 and 1998). The results for these 8 projects were comparable in regards to PSC, IRI, and rutting, but the IRI result was not as remarkably in favor of the Superpave designed sections.

- PSC: 75 percent of the Superpave projects have the same or less surface distress than the Hveem-designed projects
- IRI: 63 percent of the Superpave projects are smoother than the Hveem-designed projects
- Rutting: 63 percent of the Superpave projects have less rutting than the Hveem-designed projects

At this time, Superpave seems to be performing as well as the Hveem-designed mixes in terms of PSC, IRI, and rutting. The comparison sections ranged in age from one to six years, but the majority of the sections were one to three years old. The expected overlay life cycle for Eastern and Western Washington is approximately 12 and 15 years, respectively, so a similar analysis will be performed in the near future to verify these results.

Performance-Graded Binder Performance

Since Superpave consists of an asphalt binder specification and a mix design method, an examination of the performance-graded (PG) binders was also conducted.

For the intersection rutting analysis, there were 8 projects that had signalized intersections. The high-temperature binder grades used were PG 76, PG 70, and PG 64, which corresponds to one or two bumps (four projects each) from the base grade at 98 percent reliability. WSDOT requires a two-grade bump for standing traffic, but depending on the length of the project, the number of intersections within the project, and the previous rut depth severity, a one-grade bump may have been selected for the entire project.

With three years of data, the maximum rut depth was 1/4 inch. After the first year, the maximum rut depths for intersections using a PG 76, PG 70, and PG 64 were 1/8 inch, 1/4 inch, and 3/16 inch, respectively. Figure 5 shows a comparison of the

average rut depths by grade of binder for the overlays up to three years of age. Although none of the average rut depths exceed 1/8 inch, the rut depths for the PG 76 and PG 70 binders remained fairly constant over the three years, while the PG 64 binder increased each year.

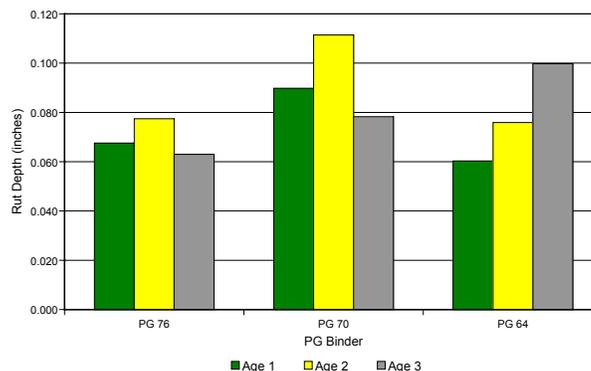


Figure 5. Average intersection rutting by binder grade.

An evaluation of low temperature cracking was performed, but since all of the Superpave projects were overlays, any transverse crack is likely due to reflective cracking and not low temperature cracking. Therefore, this evaluation is more an assessment of a PG binder’s impact on delaying reflective cracking. Based on the WSPMS, the Hveem-designed sections have two to three times more transverse cracking than the Superpave sections at the same location and age.

At this time, there is insufficient data to draw specific conclusions on the effectiveness of PG binders for rutting or low temperature cracking. As additional years of performance data become available, the benefits of PG binders will be reassessed.

Cost Comparison

Three types of cost comparisons were performed: 1) unit price of Superpave per year, 2) unit prices of Superpave and Hveem-designed HMA by class of mix, and 3) cost per lane-mile.

The unit price of Superpave per year is presented in Figure 6 along with the tonnage placed (the 1996 test section is not included because the cost was a lump sum change order). The average cost for producing and placing Superpave in 1997 (four projects) was \$36.99/ton, but after the first year, the average cost decreased to \$27.87/ton and has remained around \$28/ton since.

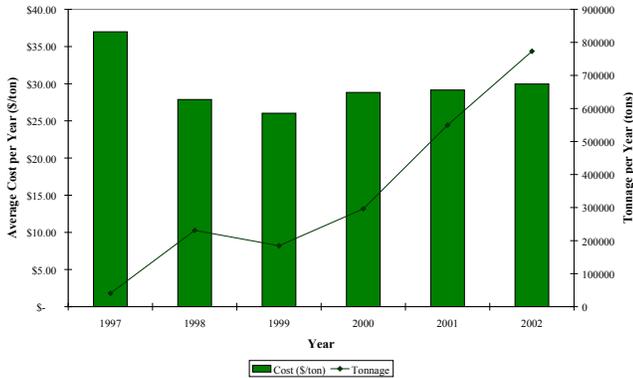


Figure 6. Superpave unit price and tonnage by year.

The comparison of unit prices is presented in Table 1. The Superpave 1/2 inch and 3/4 inch mixes are \$28.66/ton and \$26.40/ton, respectively, while the Hveem-designed mixes (A, B, and E) are between \$27.41/ton and \$27.64/ton. Overall, the costs are very similar with an average of \$27.51/ton. Although not shown in Table 1, there was a slight increase in the Hveem-designed HMA cost when PG binders were introduced. Also, Table 1 shows that HMA, in general, is more expensive in Western Washington than Eastern Washington – as expected.

Table 1. Unit price (\$/ton) comparison.

	Superpave		Hveem-designed		
	1/2 inch	3/4 inch	A	B	E
East	\$26.38	\$25.01	\$24.11	\$25.15	\$26.57
West	\$34.12	\$35.44	\$28.59	\$28.67	\$27.86
Average	\$28.66	\$26.40	\$27.41	\$27.44	\$27.64

The final comparison was based on the project cost per lane-mile. These costs include only the costs related to paving – safety, drainage, bridge work, etc. have been excluded. As expected, there are differences between the cost in rural and urban areas as well as Western versus Eastern Washington. Paving in rural areas and/or Eastern

Washington is typically less expensive, as illustrated in Table 2. The average cost per lane-mile for Superpave and Hveem-designed HMA are \$87,637/ lane-mile and \$94,835/ lane-mile, respectively.

Since market forces cannot be factored into the costs (low-bid process), these results indicate that there is no significant cost difference between Superpave and Hveem-designed HMA.

Table 2. Cost per lane-mile comparison.

Superpave HMA					
East	Rural	\$76,694	Urban	\$76,056	Average
West		\$80,739		\$134,257	
Avg		\$77,705		\$102,918	
Hveem-designed HMA					
East	Rural	\$79,500	Urban	\$91,550	Average
West		\$88,100		\$120,200	
Avg		\$83,800		\$105,875	

Conclusions

- Superpave seems to be performing as well as Hveem-designed HMA in terms of structural condition, smoothness, and rutting.
- The PG binder evaluation is, at this time, inconclusive, as to whether the high temperature binder properties reduce the severity of rutting and whether the low temperature properties prevent or delay transverse cracking.
- The cost for Superpave and Hveem-designed HMA is approximately the same.

Even though not all the improvement can be attributed to the use of Superpave and PG binders, WSDOT has found that Superpave is performing as well as Hveem-designed HMA and costs about the same. Given that Superpave is the national focus and that it is performing well to date, WSDOT will continue to use Superpave mix design methods and PG binders while continuing its implementation schedule.

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