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**Remarks and Instructions**

The complete manual, revision packages, and individual chapters can be accessed at [www.wsdot.wa.gov/publications/manuals/m23-50.htm](http://www.wsdot.wa.gov/publications/manuals/m23-50.htm).

Please contact Joe Fahoum at 360-705-7193 or [fahoumj@wsdot.wa.gov](mailto:fahoumj@wsdot.wa.gov) with comments, questions, or suggestions for improvement to the manual.

For updating printed manuals, page numbers indicating portions of the manual that are to be removed and replaced are shown below.

| Chapter   | Remove Pages        | Insert Pages        |
|---|---------------------|---------------------|
| Title Page  | i-ii                | i-ii                |
| Appendix 2.2-A3 Bridge Site Data Stream Crossing                      | 2.2-A3-1 – 2.2-A3-2 | 2.2-A3-1 – 2.2-A3-2 |
| Appendix 5.1-A7 Tension Lap Splice Lengths of Grade 60 Bars – Class B | 5.1-A7-1 – 5.1-A7-2 | 5.1-A7-1 – 5.1-A7-2 |
| Chapter 10 Signs, Barriers, Approach Slabs, and Utilities             | 10.8-7 – 10.8-8     | 10.8-7 – 10.8-8     |

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TECHNICAL MANUAL

# **Bridge Design Manual (LRFD)**

M 23-50.07

September 2011

**Engineering and Regional Operations**

Bridge and Structures Office

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**Bridge Site Data  
Stream Crossings**

|  |                           |  |              |
|--|---------------------------|--|--------------|
| Region   |                           | Made By  | Date         |
| <b>Bridge Information</b>  |                           |  |              |
| SR   | Bridge Name               | Control Section  | Project No.  |
| Highway Section  | Section, Township & Range | Datum (e.g. NGVD29, NAVD88, USGS)                                |              |
| Name of Stream   |                           | Tributary of   |              |
| Elevation of W.S. (@ Date/Time of survey)  |                           | <input type="checkbox"/> <b>Non-Tidal</b><br>Flow (CFS) WSE (ft) |              |
| _____  |                           | <input type="checkbox"/> <b>Tidal</b><br>Flow (CFS) WSE (ft)     |              |
| Streambed Material   |                           | 2-YR _____   | 2-YR _____   |
| <input type="checkbox"/> Fines <input type="checkbox"/> Gravel <input type="checkbox"/> Boulder<br><input type="checkbox"/> Sand <input type="checkbox"/> Cobble                       |                           | 100-YR _____   | 100-YR _____ |
|  |                           | 500-YR _____   | 500-YR _____ |
| Amount and Character of Drift  |                           | MLLW _____   | MHHW _____   |
| _____  |                           |  |              |
| Manning's "N" Value (Est.)   |                           |  |              |
| _____  |                           |  |              |
| <b>Attachments</b>   |                           |  |              |
| <input type="checkbox"/> Site Contour Map (See Sect. 710.04 WSDOT Design Manual)   |                           |  |              |
| <input type="checkbox"/> Highway Alignment and Profile (refer to base map and profiles)  |                           |  |              |
| <input type="checkbox"/> Streambed: Profile and Cross Sections (See Sect. 710.04 WSDOT Design Manual)  |                           |  |              |
| <input type="checkbox"/> Photographs   |                           |  |              |
| <input type="checkbox"/> Character of Stream Banks (e.g., rock, silt.) / Location of Solid Rock  |                           |  |              |
| <input type="checkbox"/> Other Data Relative to Selection of Type and Design of Bridge, Including your Recommendations (e.g., requirements of riprap, permission of piers in channel.) |                           |  |              |

DOT Form 235-001 EF  
Revised 08/2011



# Tension Lap Splice Lengths of Grade 60 Bars – Class B

## Appendix 5.1-A7

Tension Lap Splice Lengths of Grade 60 Uncoated Bars – Class B

| Bar Size | $f'_c = 3,000$ psi |        | $f'_c = 4,000$ psi |        | $f'_c = 5,000$ psi |        | $f'_c = 6,000$ psi |        |
|----------|--------------------|--------|--------------------|--------|--------------------|--------|--------------------|--------|
|          | Top Bars           | Others | Top Bars           | Others | Top Bars           | Others | Top Bars           | Others |
| #3       | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  |
| #4       | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  | 2'-0"              | 2'-0"  |
| #5       | 2'-4"              | 2'-0"  | 2'-4"              | 2'-0"  | 2'-4"              | 2'-0"  | 2'-4"              | 2'-0"  |
| #6       | 2'-11"             | 2'-1"  | 2'-9"              | 2'-0"  | 2'-9"              | 2'-0"  | 2'-9"              | 2'-0"  |
| #7       | 4'-0"              | 2'-11" | 3'-6"              | 2'-6"  | 3'-3"              | 2'-4"  | 3'-3"              | 2'-4"  |
| #8       | 5'-3"              | 3'-9"  | 4'-7"              | 3'-3"  | 4'-11"             | 2'-11" | 3'-9"              | 2'-8"  |
| #9       | 6'-8"              | 4'-9"  | 5'-9"              | 4'-2"  | 5'-2"              | 3'-9"  | 4'-9"              | 3'-5"  |
| #10      | 8'-6"              | 6'-1"  | 7'-4"              | 5'-3"  | 6'-7"              | 4'-8"  | 6'-0"              | 4'-4"  |
| #11      | 10'-5"             | 7'-5"  | 9'-0"              | 6'-5"  | 8'-1"              | 5'-9"  | 7'-4"              | 5'-3"  |
| #14      | Lap Splices        |        | Lap Splices        |        | Lap Splices        |        | Lap Splices        |        |
| #18      | Not Allowed        |        | Not Allowed        |        | Not Allowed        |        | Not Allowed        |        |

Tension Lap Splice Lengths of Grade 60 Epoxy Coated Bars – Class B

| Bar Size | $f'_c = 3,000$ psi |        | $f'_c = 4,000$ psi |        | $f'_c = 5,000$ psi |        | $f'_c = 6,000$ psi |        |
|----------|--------------------|--------|--------------------|--------|--------------------|--------|--------------------|--------|
|          | Top Bars           | Others | Top Bars           | Others | Top Bars           | Others | Top Bars           | Others |
| #3       | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  |
| #4       | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  | 2'-3"              | 2'-0"  |
| #5       | 2'-10"             | 2'-6"  | 2'-10"             | 2'-6"  | 2'-10"             | 2'-6"  | 2'-10"             | 2'-6"  |
| #6       | 3'-7"              | 3'-2"  | 3'-4"              | 3'-0"  | 3'-4"              | 3'-0"  | 3'-4"              | 3'-0"  |
| #7       | 4'-11"             | 4'-4"  | 4'-3"              | 3'-9"  | 3'-11"             | 3'-5"  | 3'-11"             | 3'-5"  |
| #8       | 6'-5"              | 5'-8"  | 5'-7"              | 4'-11" | 5'-0"              | 4'-5"  | 4'-6"              | 4'-0"  |
| #9       | 8'-1"              | 7'-2"  | 7'-0"              | 6'-2"  | 6'-3"              | 5'-7"  | 5'-9"              | 5'-1"  |
| #10      | 10'-3"             | 9'-1"  | 8'-11"             | 7'-10" | 8'-0"              | 7'-0"  | 7'-3"              | 6'-5"  |
| #11      | 12'-8"             | 11'-2" | 10'-11"            | 9'-8"  | 9'-9"              | 8'-0"  | 8'-11"             | 7'-11" |
| #14      | Lap Splices        |        | Lap Splices        |        | Lap Splices        |        | Lap Splices        |        |
| #18      | Not Allowed        |        | Not Allowed        |        | Not Allowed        |        | Not Allowed        |        |

Top bars are so placed that more than 12" of concrete is cast below the reinforcement.

Modification factor for spacing  $\geq 6"$  and side cover  $\geq 3" = 0.8$ .

Modification factor for reinforcements enclosed in spirals = 0.75.

Definition of splice classes:

- Class A: Low stressed bars – 75% or less are spliced
- Class B: Low stressed bars – more than 75% are spliced  
High stressed bars – 50% or less are spliced
- Class C: High stressed bars – more than 50% are spliced

Class B lap splice is the preferred and most commonly used by Bridge Office.

Modification factor for Class A = 0.77

Modification factor for Class C = 1.31

Modification factor for 3-bar bundle = 1.2





### 10.8.5 Conduit Types

All electrical conduits shall be galvanized Rigid Metal Conduit (RGS) or Rigid Polyvinyl Chloride Conduit (PVC).

**Steel Pipe** – All pipe and fittings shall be galvanized except for special uses.

**PVC Pipe** – PVC pipe may be used with suitable considerations for deflection, placement of expansion fittings, and of freezing water within the conduits. PVC pipe should not be placed in concrete traffic barriers when the slip form method is used due to damage and pipe separation that often occurs during concrete placement.

### 10.8.6 Utility Supports

The following types of supports are generally used for various utilities. Selection of a particular support type should be based on the needs of the installation and the best economy. All utility installations shall address temperature expansion in the design of the system or expansion devices.

Utility supports shall be designed so that a failure will not result in damage to the bridge, the surrounding area, or be a hazard to traffic. Utility supports shall be designed so that any loads imposed by the utility installation do not overstress the conduit, supports, bridge structure, or bridge members.

Designs shall provide longitudinal and transverse support for loads from gravity, earthquakes, temperature, inertia, etc. It is especially important to provide transverse and longitudinal support for inserts that cannot resist moment.

The Bridge Engineer should request calculations from the utility company for any attachment detail that may be questionable. Utility attachments, which exert moments or large forces at the supports, shall be accompanied by at least one set of calculations from the utility company. Bridge attachments designed to resist surge forces should always be accompanied by calculations.

**Concrete Embedment** – This is the best structural support condition and offers maximum protection to the utility. Its cost may be high for larger conduit and the conduit cannot be replaced.

**Pipe Hangers** – Utility lines shall be suspended by means of cast-in-place inserts, whenever possible. This is the most common type of support for utilities to be hung under the bridge deck. This allows the use of standard cast-in-place inserts and is very flexible in terms of expansion requirements. For heavy pipes over traffic (10" water main or larger), a Safety Factor of 1.5 should be used to resist vertical loads for Strength design. This is to avoid complete failure of the utility hanger system by failure of one hanger. Vertical inserts will not provide resistance to longitudinal forces. Longitudinal and transverse supports shall be provided for ITS conduits. Vertical supports shall be spaced at 5 foot maximum intervals for telephone and power conduits, and at a spacing to resist design loads for all other utilities.

When 3/4" or 7/8" diameter hanger rods are suspended from cast-in-place inserts, at least three of the following inserts shall be identified: Cooper B-Line B22-I Series, Unistrut 3200 Series, Powerstrut 349 Series, Halfen HT5506 or similar. The specific cast-in-place insert within each series shall be identified based on the required length of insert. The cast-in-place insert shall be at least 6" long and hot dipped galvanized per AASHTO M 111 or AASHTO M 232.

The Bridge Engineer shall verify that the insert does not interfere with reinforcement in the bridge deck since the inserts are installed level longitudinally and transversely. When the superelevation of the roadway is not significant, a single, long insert may be used to support multiple hanger rods. When the superelevation becomes significant, a single insert may be used for each hanger.

Occasionally large diameter utilities require pipe rolls that only fit on 1" diameter hanger rods. When 1" diameter hanger rods are required, the Anvil Fig. 286 insert shall be used. The designer shall only specify this insert when absolutely necessary.

The Bridge Engineer shall verify that the cast-in-place insert has sufficient capacity to support the loads from the hanger rod.

Transverse supports may be provided by a second hanger extending from a girder or by a brace against the girder. The Appendix 10.8-A1-1 and 10.8-A1-2 depict typical utility support installations and placement at abutments and diaphragms. Transverse supports shall, at a minimum, be located at every other vertical support.