



WSDOT Scour Workshop

Module 7
Contraction Scour

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Senior Hydraulic Engineer

FHWA Resource Center



- Training
- Technical Support
- Technology Development and Deployment



Background and Experience

- 10 years at FHWA Resource Center
- 7 years at Central Federal Lands Hydraulics Team Lead
- 14 years consulting engineering



- B.S. Civil Engineering (Colorado State University)
- M.S. Hydraulics (Colorado State University)



- Hiking
- Camping
- Fishing and Hunting
- Cycling
- DIY Home Improvement





Casey Kramer

Principal River Engineer

Natural Waters, LLC



- Owner of Natural Waters, LLC
- WSDOT HQ staff augmentation team assisting State Hydraulics Engineer



- Previous WSDOT State Hydraulic Engineer
- Private sector hydraulics and river engineering consultant



- B.S. Civil Engineering (Washington State University)
- M.S. River Engineering (University of Iowa lowa Institute of Hydraulic Research)





- Spending time with family
- Rivers
- Dirt biking, Fishing, Camping, and Coaching/Watching Sports



Contraction Scour Overview

- Types of contraction scour
- Summary for how to compute contraction scour
- Steps to compute contraction scour



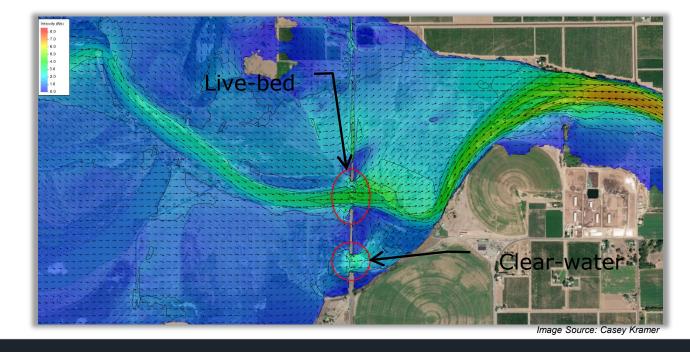
Image Source: Casey Kramer

Types of Contraction Scour

 Clear-water – No transport of bed material sediment from upstream. The area of the contracted section increases until the velocity of the flow is equal to the critical velocity of the bed material

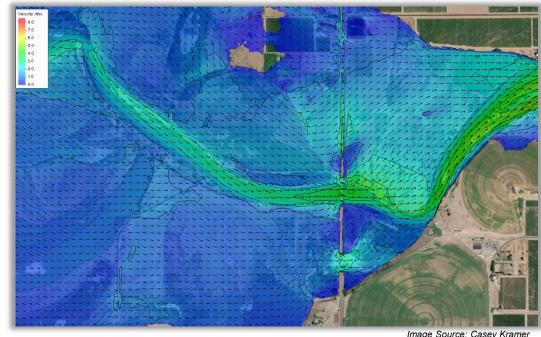
Live-bed — Bed material from the riverbed upstream is transported into the

crossing



Contraction Scour – Clear-water or Live-Bed

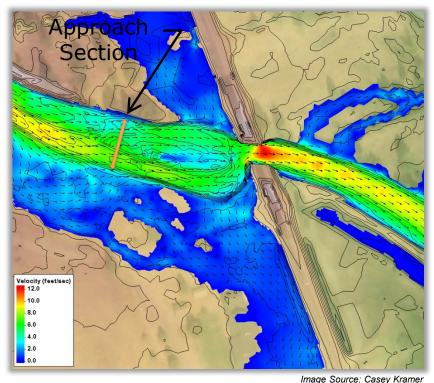
- Compare the average velocity (V₁) and the critical velocity (V_c) for bed material transport at the approach section
 - If V₁ > V₂ → live-bed is most likely
 - If V₁ < V_c → clear-water is most likely
- Compute clear-water scour using the bed material gradation (D_{50}) at the water crossing
- Always compute contraction scour for live-bed and clear-water scour conditions



How to Compute Contraction Scour

Approach Section location:

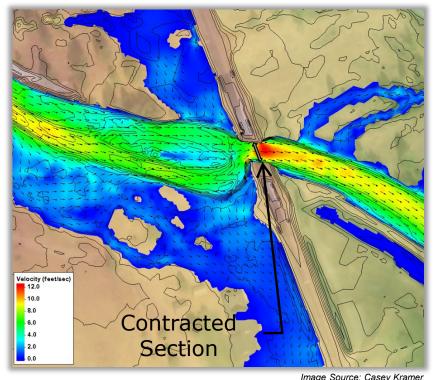
- Upstream of where overbank flow is diverted into the main channel
- Width represents the flow that would likely transport sediment:
 - Toe to toe (of slopes) or between tops of banks
 - Field observations (e.g., sediment being mobilized, vegetation, etc.)
 - Critical velocity or shear for incipient motion
- Avoid locally higher velocity locations, as they result in underestimating contraction scour



How to Compute Contraction Scour

Contracted Section location:

- Where flow is most contracted through the bridge
- Width should be consistent with the approach section reference (i.e., toe to toe (of slopes) or between tops of banks)
- Width should exclude effective width of piers
- Width needs to be adjusted for skew



Main Channel vs Overbank Contraction Scour

- If the main channel can migrate to either abutment, the main channel computed contraction scour is applied to the entire opening width
- Main channel contraction scour is likely live-bed scour, but clear-water scour may occur
- Overbank contraction scour is not common for most WSDOT fish passage projects. When it does occur, it is predominantly clear-water scour when channel is assessed to be stable and the overbanks are vegetated (i.e., secondary and relief structures)

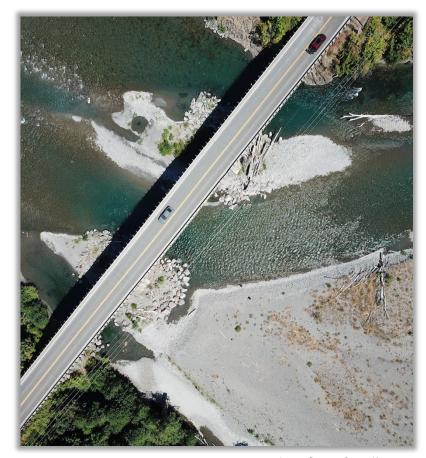
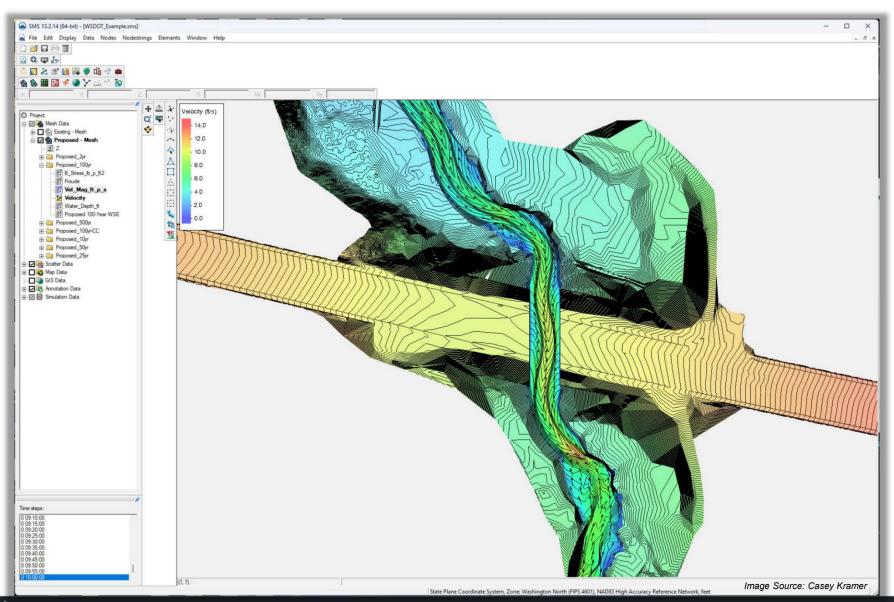


Image Source: Casey Kramer

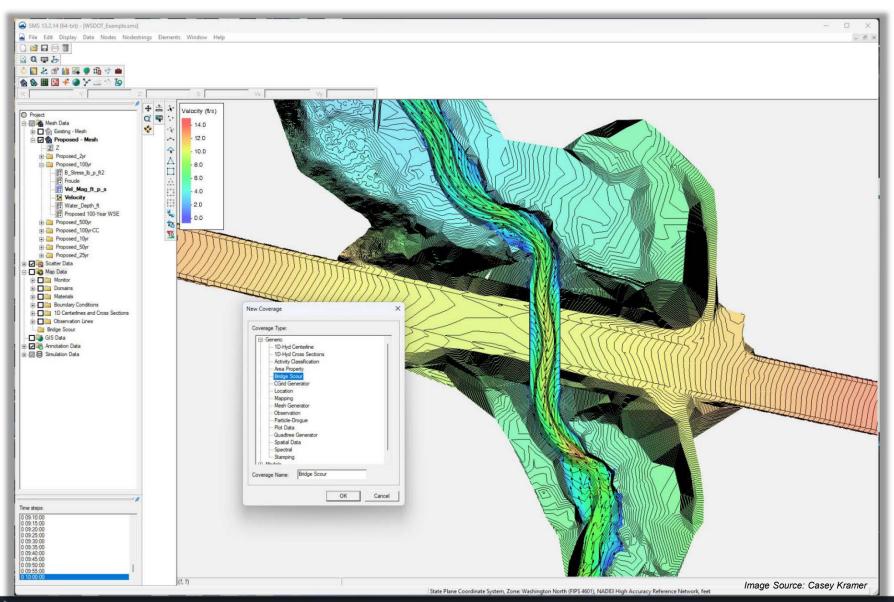


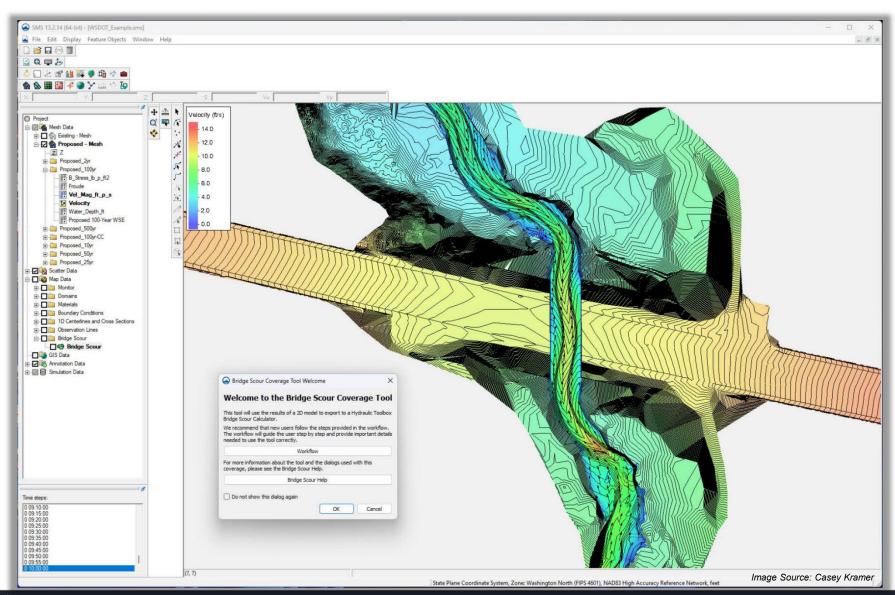
- The following provides an example for computing contraction scour for a PHD
- Key assumptions:
 - The structure type, size and location has not been determined
 - Contraction scour is determined at the selected contracted section location.
 - Per the WSDOT H_HD template other total scour tables may be necessary to perform scour analysis at appropriate locations
 - Channel profile and geometry is typical through the crossing, therefore flow depths are assumed to be uniform.
 - Applying depths of scour at locations other than the location of the selected contraction scour arc needs to be assessed carefully

- The hydraulic engineer of record determines appropriate total scour elevations that are commensurate with the site and acceptable level of risk in coordination with WSDOT
- Ultimate goal is to determine scour elevations at each infrastructure component being designed. Coordination MUST happen with the project, geotechnical, and bridge and structures offices to determine appropriate scour elevations to be used for design as the design progresses
- The intent of the next series of slides is to show an example for how contraction scour should be calculated but does not go into detail of all scenarios and analyses required to develop total scour. Completion of NHI Course 135046 and the FHWA scour workshop is required for more details on computing total scour

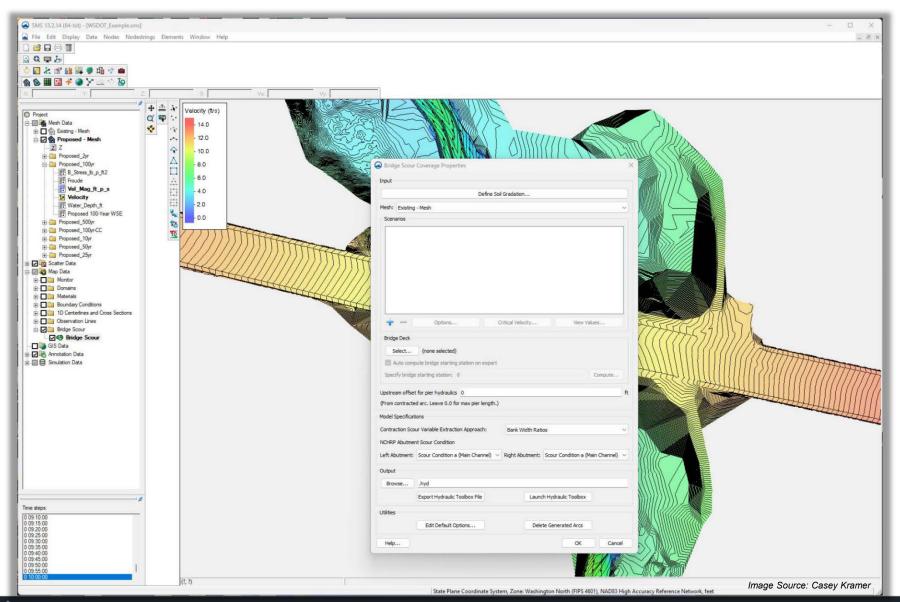




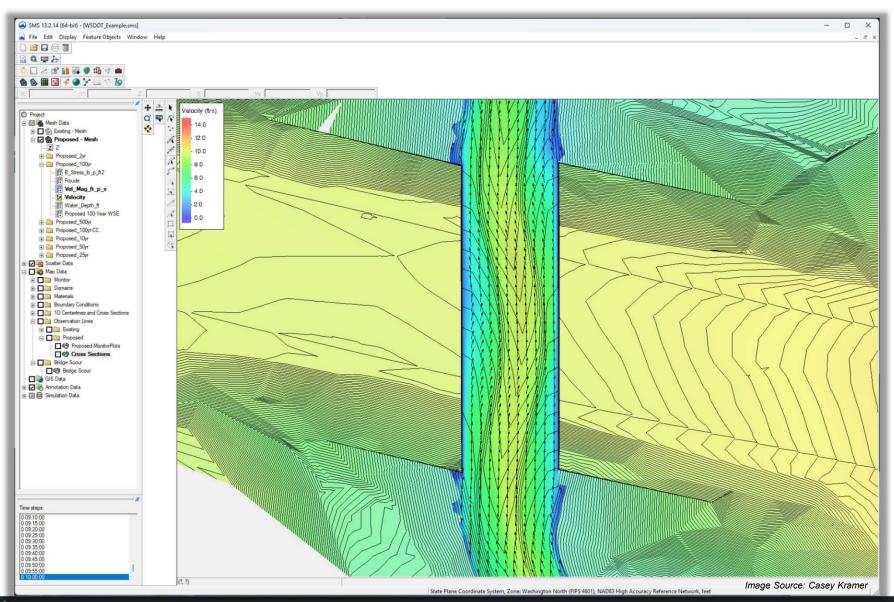




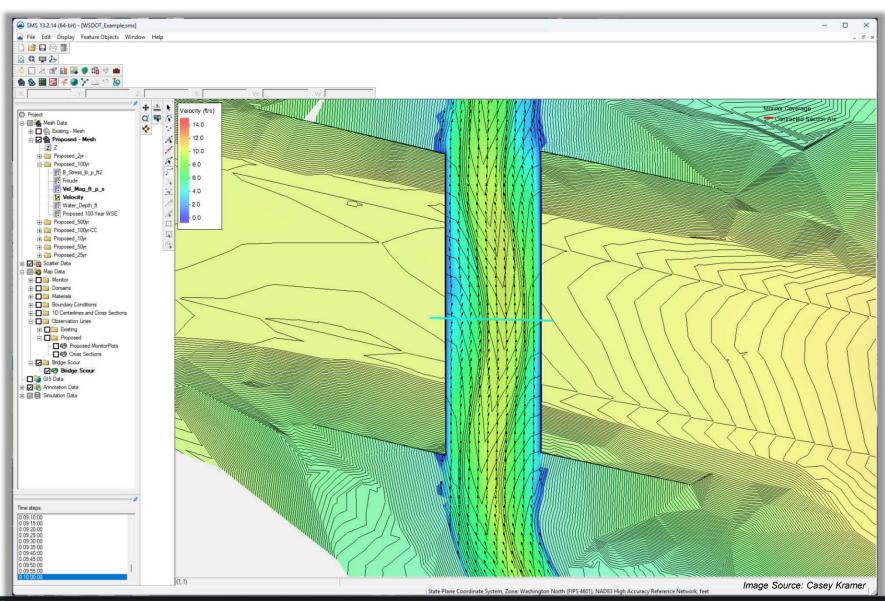




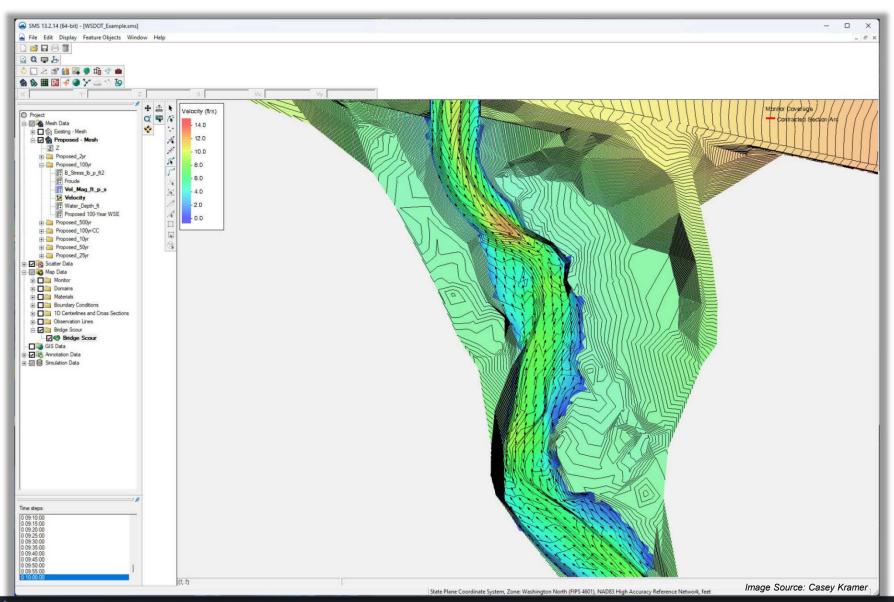




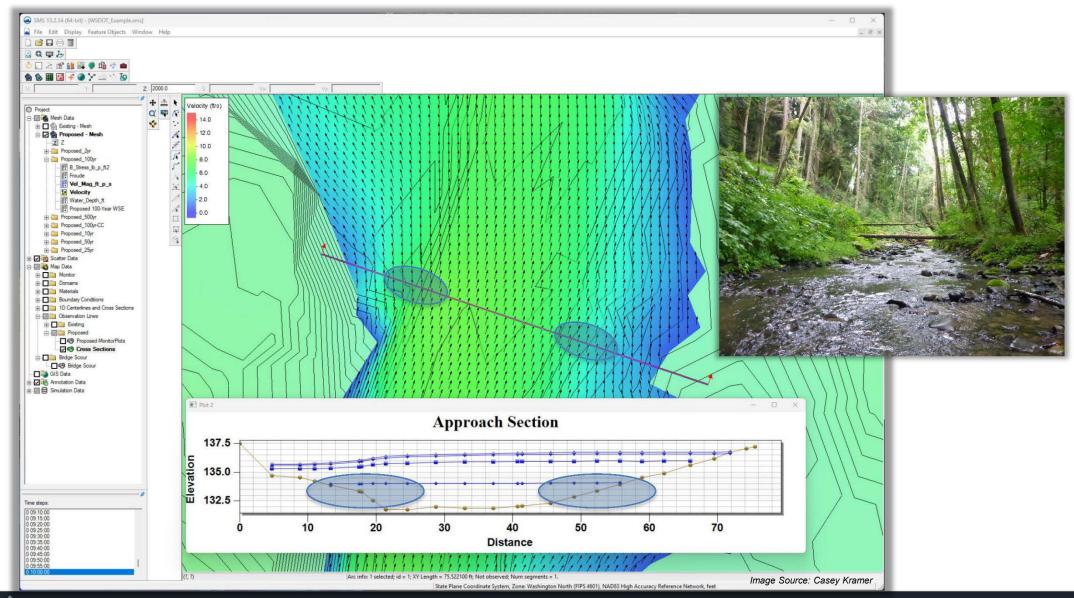




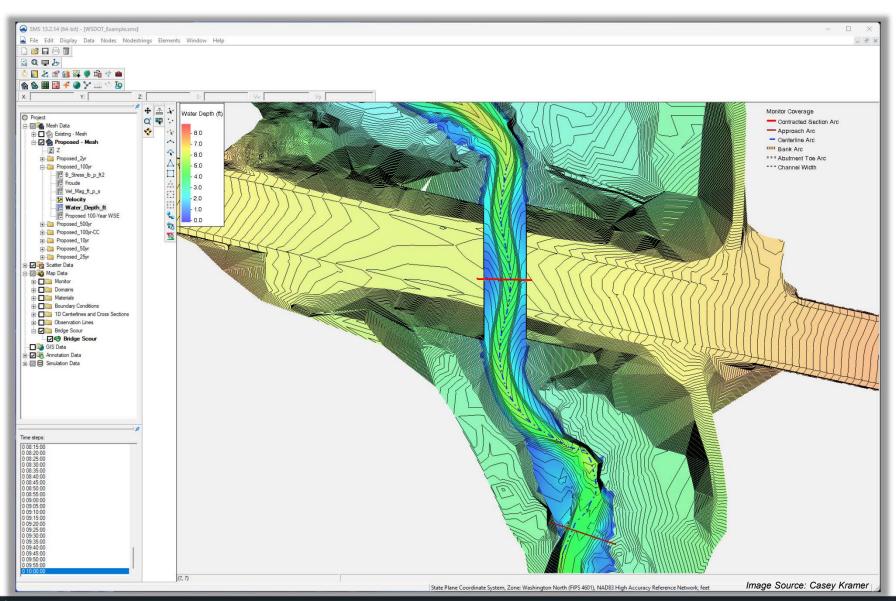




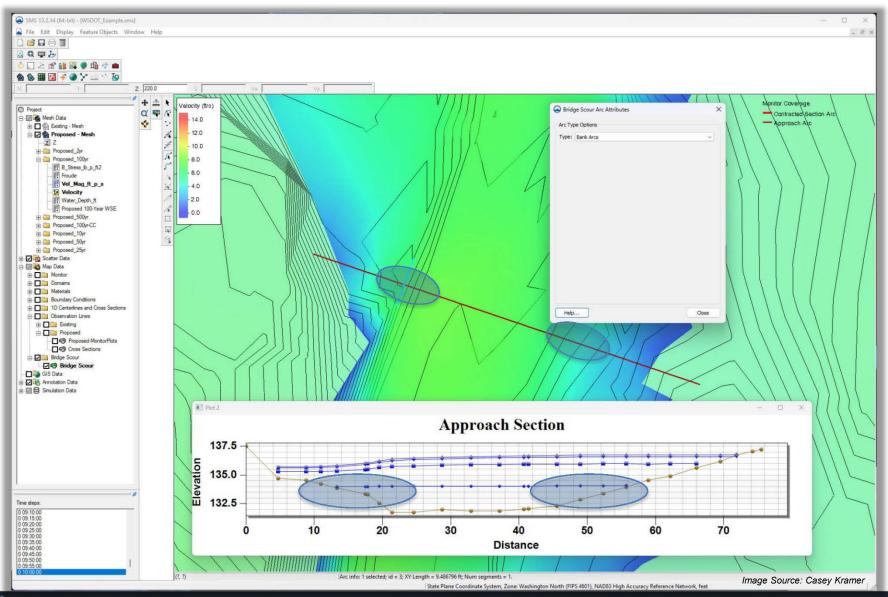




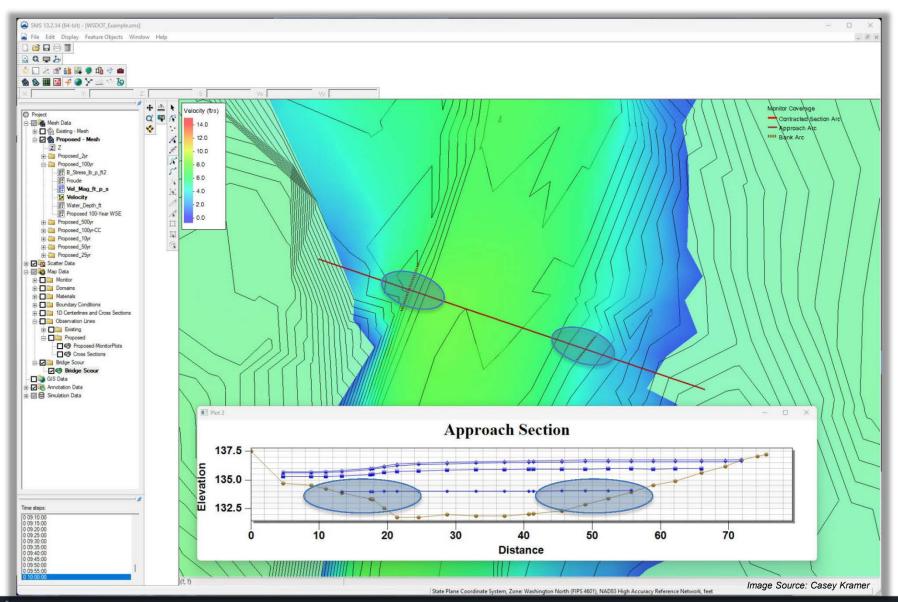




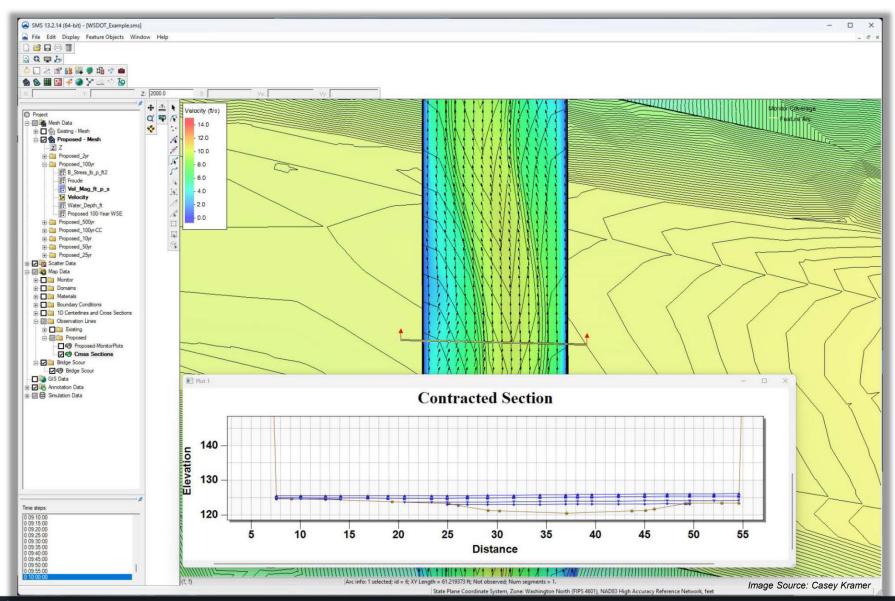




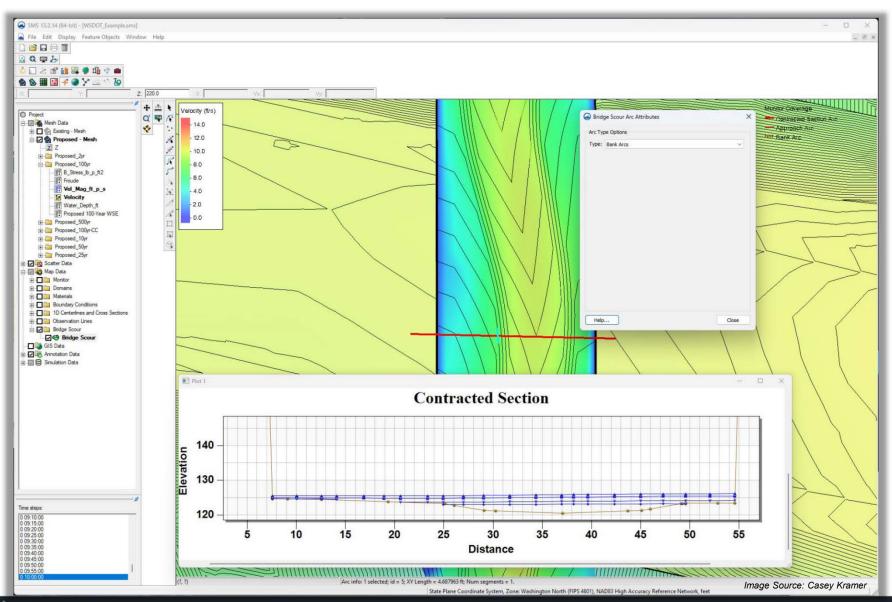




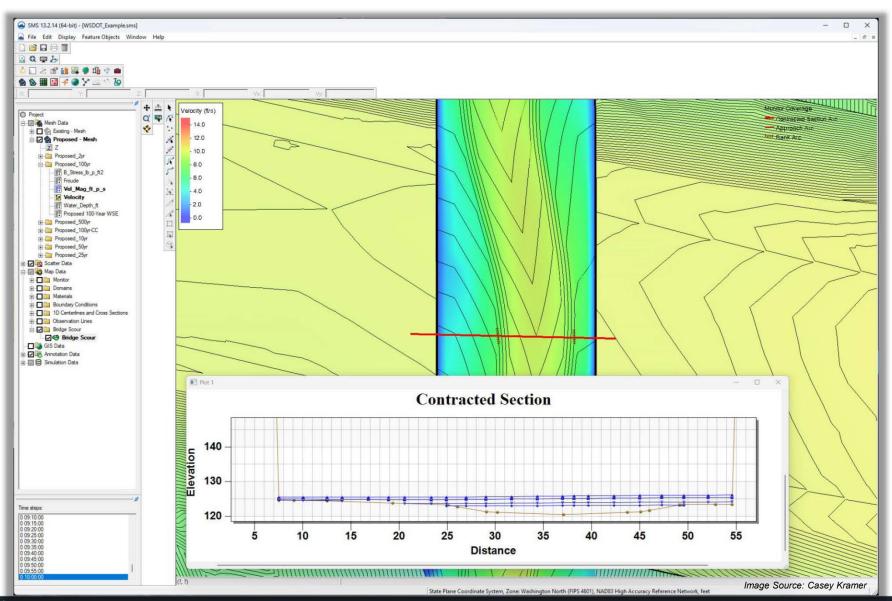




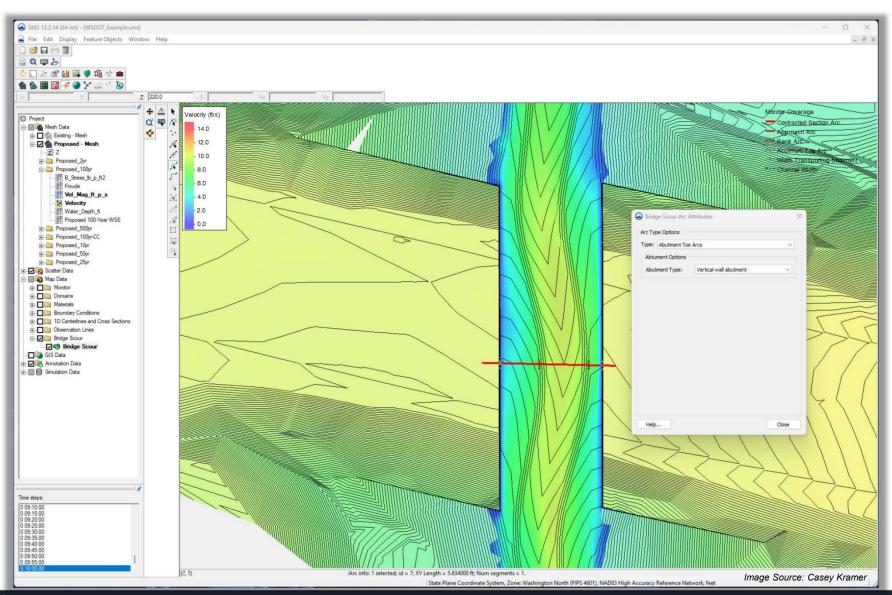




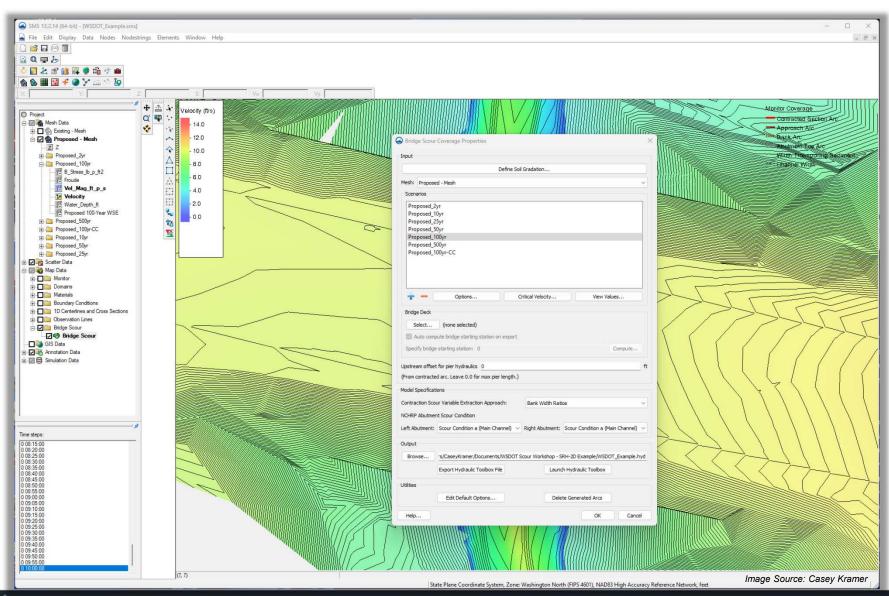




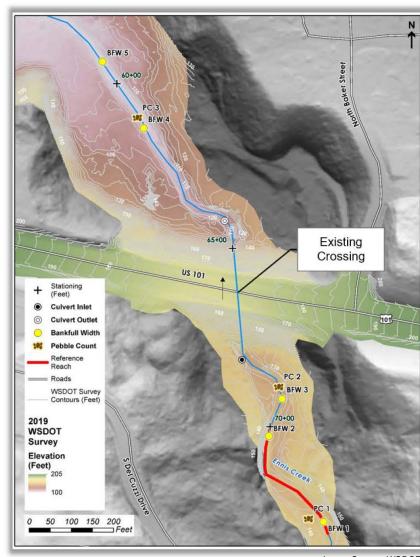






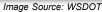






Particle Percent Smaller Than	Pebble Count 1, Particle Diameter (Inches) ^a	Pebble Count 2, Particle Diameter (Inches) ^b	Pebble Count 3, Particle Diameter (Inches) ^c 1.6 4.3 12.8 39.4		
D ₁₆	1.5	1.3			
D ₅₀	3.0	3.1			
D ₈₄	6.9	5.9			
D ₁₀₀	19.7	17.7			

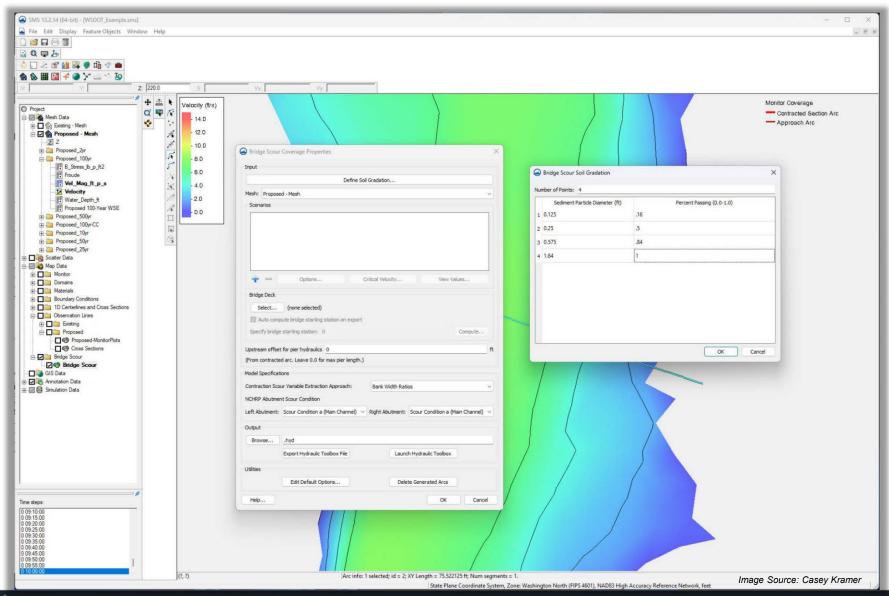
b. Pebble Count 2 token about 110 feet upstream of culvert at cobble bar.



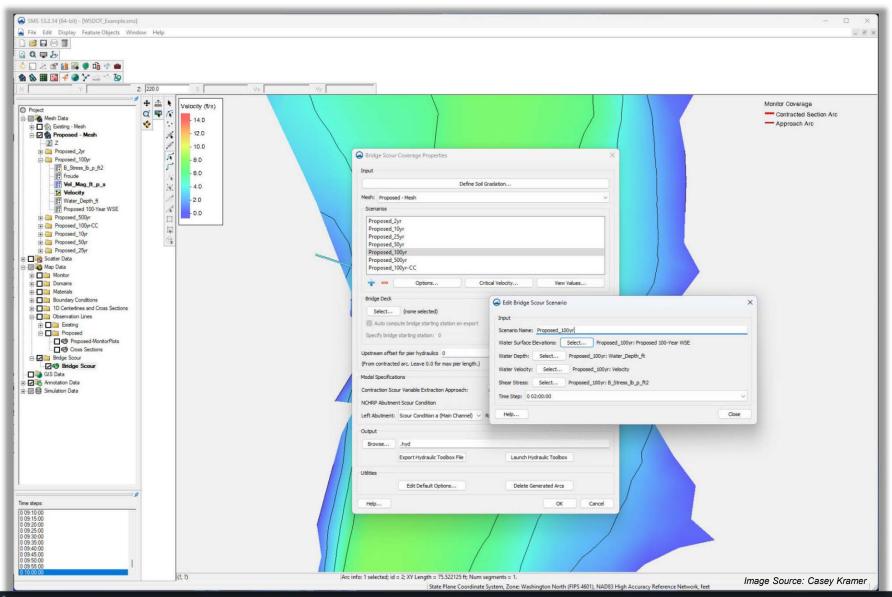




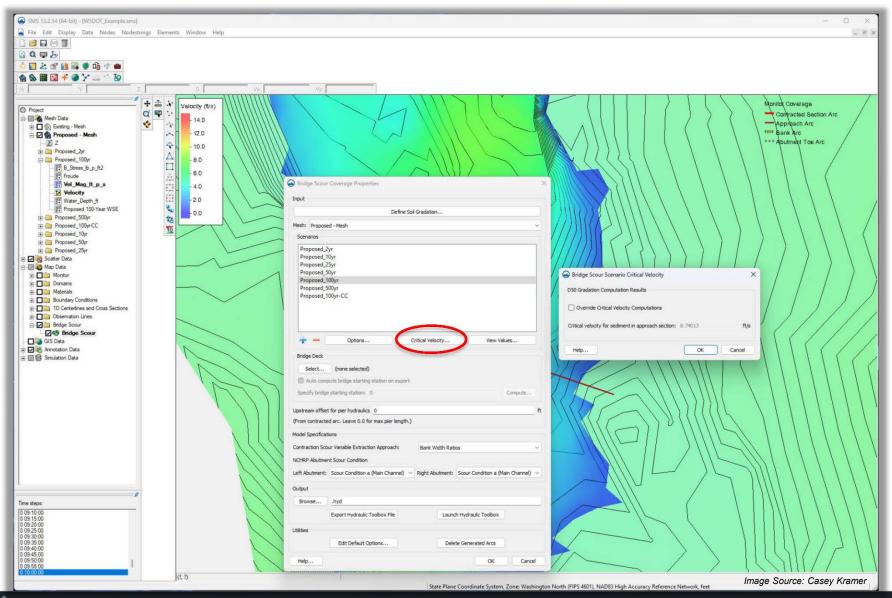
c. Pebble Count 3 taken about 300 feet downstream of culvert in active channel where alluvial cover was present.



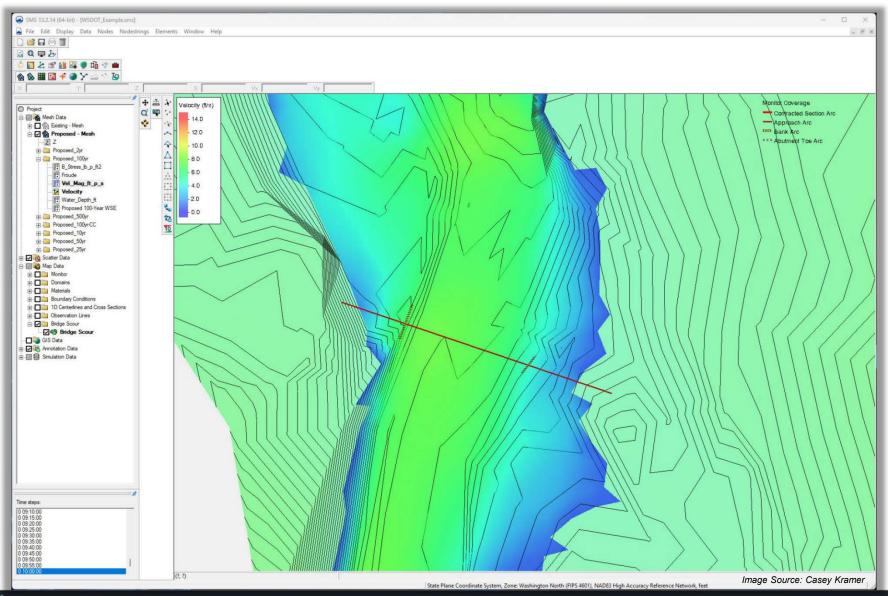




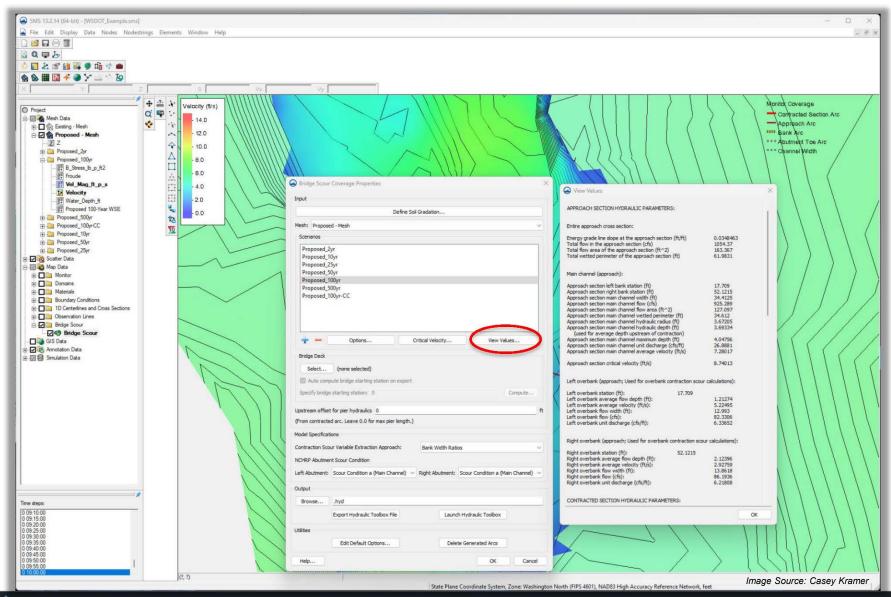




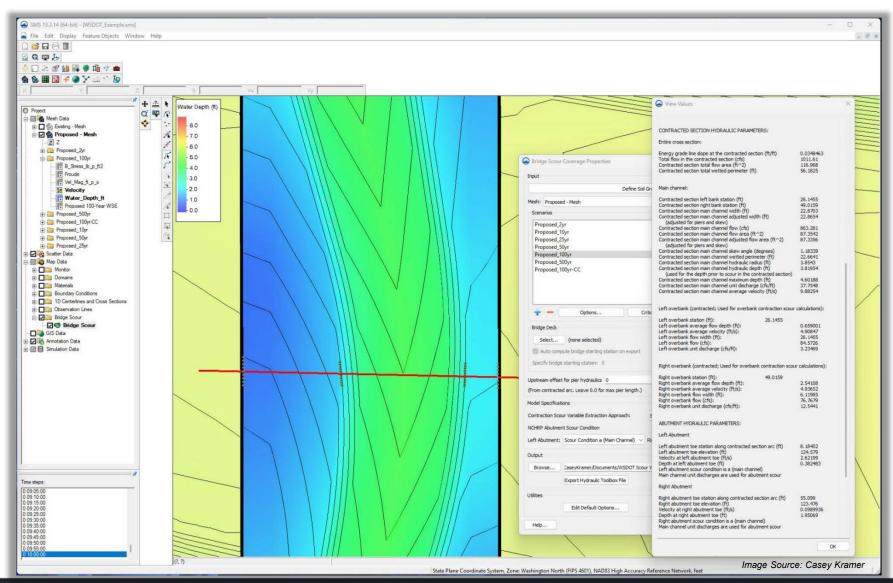




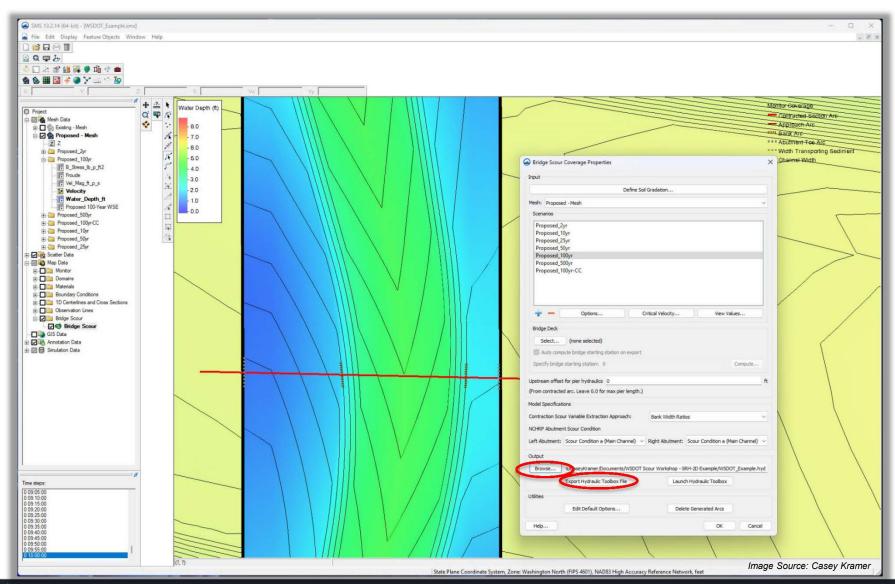




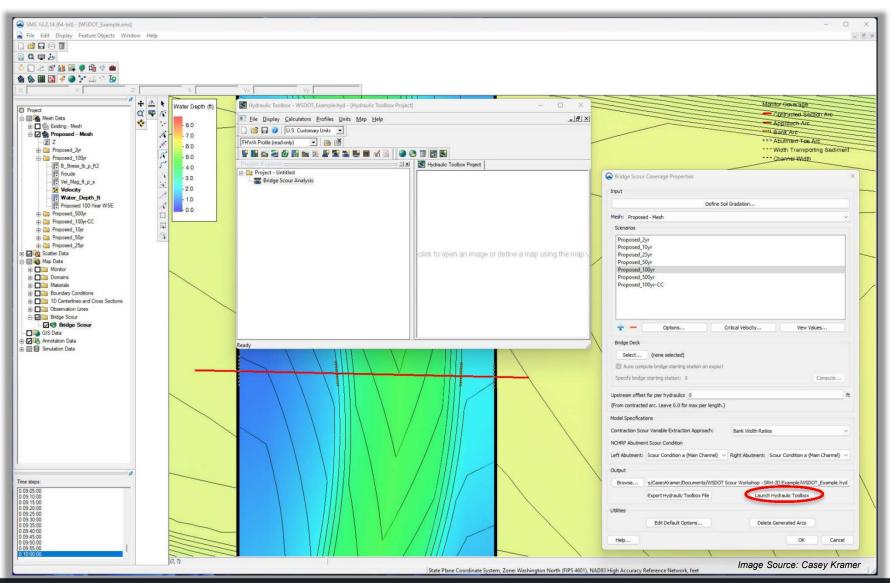




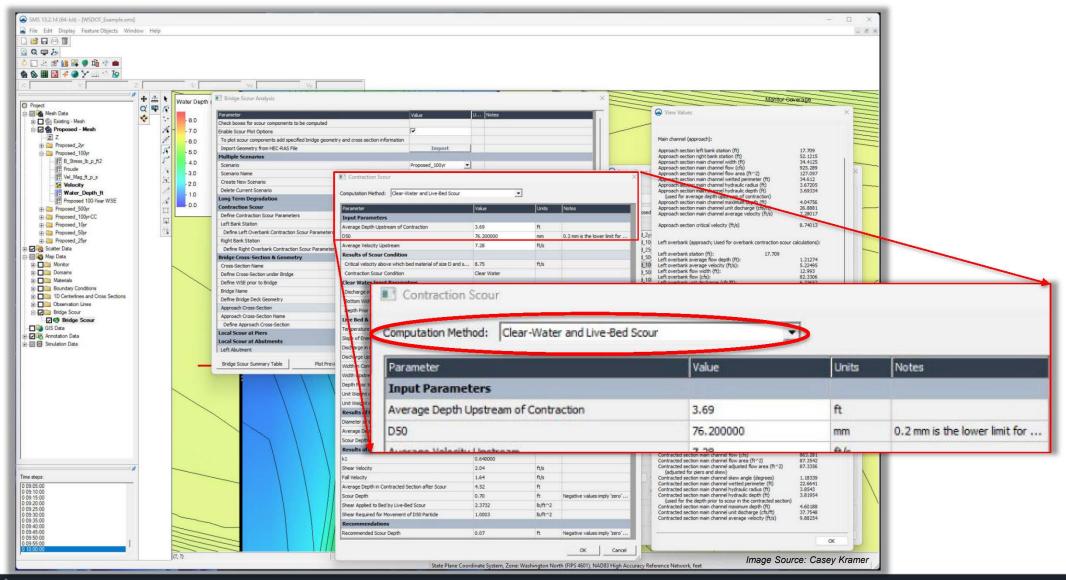


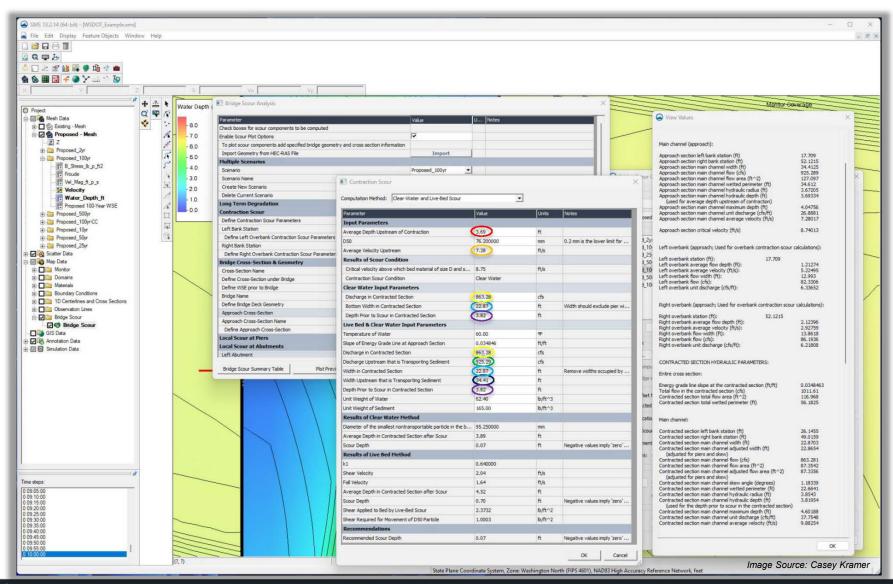




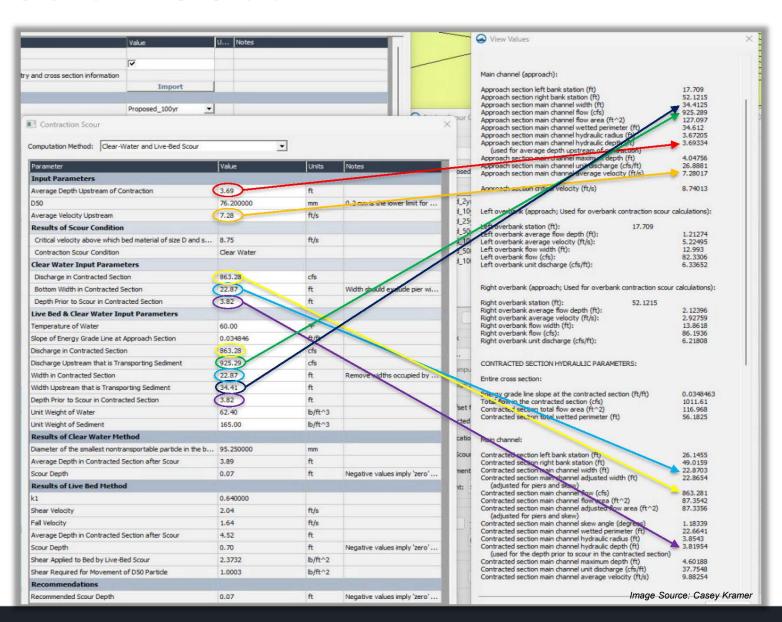






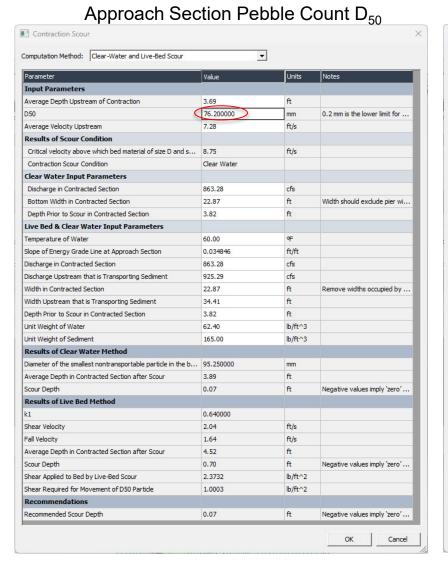


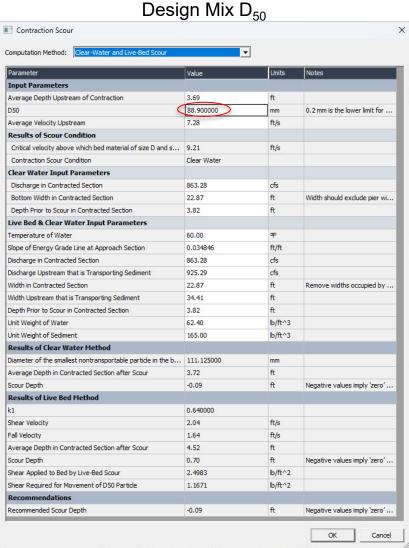






Particle Percent Smaller Than	Bathurst Calculated 100- Year Particle Diameter (Inches)	Proposed Streambed Gradation (Inches)			
D ₁₆	1.1	0.5			
D ₅₀	3.6	3.5			
D ₈₄	9.1	11.8			
D ₁₀₀	22.7	18.0			







To Demonstrate Requirement of All Flows Up To Scour Design Flood and Scour Check Flood ONLY

■ Bridge Scour Summary Table									×
Parameter	Proposed_2yr	Proposed_10yr	Proposed_25yr	Proposed_50yr	Proposed_100yr	Proposed_500yr	Proposed_100yr-CC	Units	Notes
Scenario						To the second	10010		
Contraction Scour									
Selected Contraction Computation Method	Clear-Water	Clear-Water	Clear-Water	Clear-Water	Clear-Water a	Clear-Water an	Clear-Water and Li		Clear-Water and Live-Bed Scour
Applied Contraction Scour Depth	0.00	0.00	0.00	0.00	0.00	0.16	0.10	ft	
Clear Water Contraction Scour Depth	-0.37	-0.30	-0.24	-0.17	-0.09	0.16	0.10	ft	Item bolded is the governing contraction scour for scenario
Live Bed Contraction Scour Depth	0.61	0.77	0.73	0.71	0.70	0.71	0.71	ft	Item bolded is the governing contraction scour for scenario

<u>Depths of Scour are Determined at PHD as Structure Type, Size and Location has not Been Determined</u>

Total Scour Elevation at Each Infrastructure Component is Determined by Interdisciplinary Team as Design Progresses